TRANSPORTATION IMPACT STUDY

for the proposed

RETAIL DEVELOPMENT

South Fayette Township, Allegheny County, Pennsylvania

October 17, 2023

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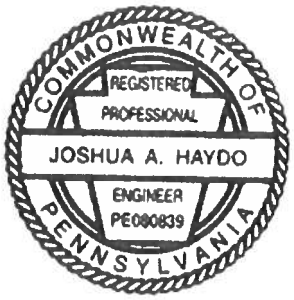

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Transportation Impact Study
Proposed Retail Development
South Fayette Township, Allegheny County, Pennsylvania

EXECUTIVE SUMMARY

Project Description

The proposed project is located on the northwestern corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. The development is proposed to consist of ~45,126-square foot of retail space.

This report analyzes the impact of this project on the traffic operations of the adjacent roadway network under:

- Existing Year 2023 Condition
- Opening Year 2024 Without and With Development Conditions
- Design Year 2029 Without and With Development Conditions

Existing and Future Without Development Conditions

A linear growth rate of 1.00% was obtained from a representative of the Southwestern Pennsylvania Commission (SPC) for South Fayette Township. This rate was applied to the Existing Year 2023 Condition peak hour traffic volumes to develop the Opening Year 2024 and Design Year 2029 Base Condition traffic volumes.

Trip Generation and Distribution

Trip Generation

The Institute of Transportation Engineers' (ITE's) *Trip Generation Manual*, 11th Edition, was used to determine the trip generation rates associated with the proposed development and background development trips (specifically, Land Use Codes #821 – *Shopping Plaza 40k-150k without Supermarket,* #931 – *Fine Dining Restaurant,* #932 – *High-Turnover Sit-Down Restaurant,* and #934 – *Fast Food Restaurant with Drive-Through Window*). The rates for LUC #821 were utilized to determine the additional trips anticipated to be generated by the proposed development on a typical weekday and during the AM, PM, and SAT peak hours, which are summarized as follows:

- 3,048 additional vehicles during a typical weekday (1,524 entering and 1,524 existing)

- 78 additional vehicles during the AM peak hour (48 entering and 30 exiting)
- 234 additional vehicles during the PM peak hour (115 entering and 119 exiting)
- 251 additional vehicles during the SAT peak hour (131 entering and 120 exiting)

Trip Distribution

The trip generation data, the distribution of traffic at the existing study intersections, and the location of the proposed development were all used to assign the site-generated traffic to the study area roadway network.

List of Study Intersections

The study area for this project includes two (2) existing intersections and two (2) proposed intersections:

- Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- Newbury Drive with Plaza Access / Site Drive C
- Millers Run Road (SR 0050) with Site Drive A
- Millers Run Road (SR 0050) with Site Drive B

Conclusions and Recommendations

The proposed retail development in South Fayette Township, Allegheny County, Pennsylvania is not anticipated to impact traffic operations in the study area. The following is a summary of the results and recommended improvements at each of the study intersections:

Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 2.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- No roadway improvements are recommended.

Newbury Drive with Plaza Access / Site Drive C

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 10.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- Construct a full access site drive (Site Drive C) along the western side of Newbury Drive, as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive A

- Construct a right-in / right-out access site drive (Site Drive A) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive B

- Construct a right-in / right-out access site drive (Site Drive B) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Transportation Impact Study
Proposed Retail Development
South Fayette Township, Allegheny County, Pennsylvania

1.0 INTRODUCTION / PROJECT SUMMARY

David E. Wooster and Associates (Wooster) has completed a Transportation Impact Study (TIS) to determine the impacts to traffic operations resulting from the construction of a proposed retail development located on the northwestern corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. The development is proposed to consist of ~45,126-square foot of retail space.

A site location graphic is shown on **Figure 1** and a preliminary Site Plan can be seen on **Figure 2** in the Figures section at the end of this report. The project is anticipated to open by the end of 2024. As such, this report analyzes the impact of this project on the traffic operations of the adjacent roadway network under:

- Existing Year 2023 Condition
- Opening Year 2024 Without and With Development Conditions
- Design Year 2029 Without and With Development Conditions

Traffic volumes have been developed for each scenario, capacity analyses have been performed, and the results have been reported in terms of both Level-of-Service (LOS) and average delay per vehicle. The capacity analyses contained in this report were performed using Synchro Software Version 11 (Synchro).

A virtual TIS Scoping Meeting was held on Thursday, September 14, 2023 and was attended by representatives of PennDOT Engineering District 11-0 (PennDOT), South Fayette Township, the applicant, and Wooster. A copy of the TIS Scoping Checklist has been included in **Appendix A** at the end of this report.

2.0 DATA COLLECTION

2.1 Turning Movement Counts

Turning movement counts were performed at the existing study intersections on a typical weekday (Tuesday through Thursday) between the hours of 7:00 a.m. and 9:00 a.m. and between the hours of 4:00 p.m. and 6:00 p.m. Counts were also performed on a typical Saturday between the hours of 11:00 a.m. and 2:00 p.m. These times were chosen because they typically reflect the morning (AM), evening (PM), and Saturday midday (SAT) peak hours for vehicular traffic. These counts were performed in September 2023.

Summaries of the turning movement counts can be found in **Appendix B** at the end of this report. This information was then utilized to determine the Existing Year 2023 Condition peak hour traffic volumes, which can be seen graphically on **Figure 3a**.

2.2 Crash Data

Copies of crash data summaries for the five (5) most recent calendar years were obtained from the Pennsylvania Department of Transportation (PennDOT) Crash Information Tool (PCIT) for the existing study intersections. As the crash data is property of PennDOT, a summary of the identified crashes and the corresponding crash reports are included in a separately-bound appendix to this report.

3.0 STUDY AREA CONDITIONS

3.1 Study Area

The study area for this project includes two (2) existing intersections and two (2) proposed intersections:

- Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- Newbury Drive with Plaza Access / Site Drive C
- Millers Run Road (SR 0050) with Site Drive A
- Millers Run Road (SR 0050) with Site Drive B

3.2 Existing Study Intersection Descriptions

3.2.1 Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

The intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive is a signalized intersection with four (4) approaches. The eastbound approach (Millers Run Road) consists of an exclusive left turn lane, an exclusive through lane, and a shared through / right turn lane. The westbound approach (Millers Run Road) consists of an exclusive left turn lane, two (2) exclusive through lanes, and an exclusive, channelized right turn lane, which is yield controlled. The posted speed limit on Millers Run Road (SR 0050) is 40 mph. The northbound approach (Todd A. Miller Drive) consists of an exclusive left turn lane, an exclusive through lane, and an exclusive, channelized right turn lane, which is yield controlled. There is no posted speed limit on Todd A. Miller Drive.[1] The southbound approach (Newbury Drive) consists of two (2) exclusive left turn lanes and a shared through / right turn lane. There is no posted speed limit on Newbury Drive.

[1] HCM 6th Edition supports only speed limits within the range of 25-55 mph. As such, a speed limit of 25 mph was assumed for all roadways without a posted speed limit or a speed limit less than 25 mph.

3.2.2 Newbury Drive with Plaza Access / Site Drive C

The intersection of Newbury Drive with Plaza Access / Existing Site Access[2] is an unsignalized intersection with four (4) approaches. The eastbound approach (Existing Site Access) consists of a single lane that is used to perform all possible movements and is stop-controlled. There is no posted speed limit on the Existing Site Access. The westbound approach (Plaza Access) consists of a single lane that is used to perform all possible movements and is stop-controlled. There is no posted speed limit on the Plaza Access. The northbound approach (Newbury Drive) consists of a shared left turn / through lane and a shared through / right turn lane, which operate under free-flow conditions. The southbound approach (Newbury Drive) consists of a shared left turn / through lane and a shared through / right turn lane, which operate under free-flow conditions. There is no posted speed limit on Newbury Drive.

A complete photo log of the existing study intersections can be found in **Appendix C** at the end of this report.

3.3 Signal Permit Plans

Traffic signal permit plans were requested for the signalized study intersection from a representative of PennDOT. The signal permit plans were utilized to model the Existing Year 2023 Condition in Synchro.

Copies of the signal permit plans have been included in **Appendix D** at the end of this report.

3.4 Existing Condition Capacity Analysis

The capacity analyses contained in this report were performed using Synchro Software Version 11 (Synchro) HCM 6th Edition reports. Using the traffic volumes developed for each scenario, Synchro assigns a Level of Service (LOS) for each approach to each study intersection. These LOS range from "A" to "F", similar to a school's grading system, with LOS A being the best possible traffic operation conditions and LOS F being the worst. A summary of these guidelines has been included in **Appendix E** at the end of this report.

The Existing Year 2023 Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the existing study intersections during the AM, PM, and SAT peak hours.

[2] Existing Site Access will become the proposed Site Drive C.

Tables 1A and **1B** in the Tables section at the end of this report show the Existing Year 2023 Condition AM, PM, and SAT peak hour LOS for the existing study intersection. The Existing Year 2023 Condition peak hour LOS can be seen graphically on **Figure 3b**.

Synchro and HCM printouts for the Existing Year 2023 Condition can be found in **Appendix F** and **Appendix G**, respectively, at the end of this report.

4.0 OPENING YEAR 2024 WITHOUT DEVELOPMENT CONDITION

4.1 Base Condition Traffic Volume Development

A linear growth rate of 1.00% was applied to the Existing Year 2023 Condition peak hour traffic volumes (Figure 3a) to develop the Opening Year 2024 and Design Year 2029 Base Condition traffic volumes, which can be seen graphically on **Figure 4** and **Figure 5**, respectively.

4.2 Background Developments (Piazza Retail, South Fayette Commons, and Newbury: Cigar Bar & Restaurant Developments)

Trips associated with several developments that have not yet been constructed (or fully constructed) were added to the Opening Year 2024 Base Condition to develop the Opening Year 2024 Without Development Condition. These developments are discussed in the following sections. Source data for these developments (i.e. email correspondence, development descriptions, site plans, TIA/TIS figures, etc.) can be found in **Appendix H** at the end of this report.

The background developments include the construction of the Piazza Retail, South Fayette Commons, and Newbury (specifically, the Cigar Lounge, Bar & Restaurant) developments.

- The Piazza Retail development consists of a 8,250-SF Fast Food Restaurant with a Drive-Through and a 6,800-SF High-Turnover (Sit-Down) Restaurant.
- The South Fayette Commons development consists of a 10,500-SF Specialty Retail Center and a 3,000-SF High-Turnover (Sit-Down) Restaurant.
- The Newbury Cigar Lounge, Bar, & Restaurant development consists of a 9,377-SF Fine Dining Restaurant.

Trips associated with these developments that anticipated to travel through the study area during the AM, PM, and SAT peak hours can be seen graphically on **Figure 6a** through **Figure 10**.

4.3 Without Development Condition Traffic Volume Development

The background development trips (Figure 10) were added to the Opening Year 2024 Base Condition traffic volumes (Figure 4) to develop the Opening Year 2024 Without Development Condition traffic volumes, which can be seen graphically on **Figure 11a.**

4.4 Capacity Analysis

The analysis performed for the Opening Year 2024 Without Development Condition assumed the same intersection geometry and traffic control used in the Existing Year 2023 Condition analysis.

The Opening Year 2024 Without Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A and **1B** in the Tables section at the end of this report show the Opening Year 2024 Without Development Condition AM, PM, and SAT peak hour LOS for the existing study intersection. The Opening Year 2024 Without Development Condition peak hour LOS can be seen graphically on **Figure 11b**.

Synchro and HCM printouts for the Opening Year 2024 Without Development Condition can be found in **Appendix I** and **Appendix J**, respectively, at the end of this report.

5.0 DESIGN YEAR 2029 WITHOUT DEVELOPMENT CONDITION

5.1 Traffic Volume Development

Once again, background development trips (Figure 10) were added to the Design Year 2029 Base Condition traffic volumes (Figure 5) to develop the Design Year 2029 Without Development Condition traffic volumes, which can be seen graphically on **Figure 12a.**

5.3 Capacity Analysis

The analysis performed for the Design Year 2029 Without Development Condition assumed the same intersection geometry and traffic control used in the Existing Year 2023 Condition analysis.

The Design Year 2029 Without Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A and **1B** in the Tables section at the end of this report show the Design Year 2029 Without Development Condition AM, PM, and SAT peak hour LOS for the existing study intersection. The Design Year 2029 Without Development Condition peak hour LOS can be seen graphically on **Figure 12b**.

Synchro and HCM printouts for the Design Year 2029 Without Development Condition can be found in **Appendix K** and **Appendix L**, respectively, at the end of this report.

6.0 DEVELOPMENT DESCRIPTION

6.1 Proposed Development

The development is proposed to consist of ~45,126-square foot of retail space.

6.2 Proposed Access

Access to the site is proposed via three (3) site drives:

- Two (2) right-in / right-out access site drives (Site Drives A and B) along the northern side of Millers Run Road (SR 0050).
- One (1) full access site drive (Site Drive C) along the western side of Newbury Drive.

6.3 Trip Generation

The Institute of Transportation Engineers' (ITE's) *Trip Generation Manual*, 11th Edition, was used to determine the trip generation rates associated with the proposed development and background development trips (specifically, Land Use Codes #821 – *Shopping Plaza 40k-150k without Supermarket,* #931 – *Fine Dining Restaurant,* #932 – *High-Turnover Sit-Down Restaurant,* and #934 – *Fast Food Restaurant with Drive-Through Window*). The rates for LUC #821 were utilized to determine the additional trips anticipated to be generated by the proposed development on a typical weekday and during the AM, PM, and SAT peak hours, which are summarized as follows:

- 3,048 additional vehicles during a typical weekday (1,524 entering and 1,524 existing)
- 78 additional vehicles during the AM peak hour (48 entering and 30 exiting)
- 234 additional vehicles during the PM peak hour (115 entering and 119 exiting)
- 251 additional vehicles during the SAT peak hour (131 entering and 120 exiting)

The site-generated trips were then split into primary trips and pass-by trips. The pass-by trip percentages were determined using the information contained in ITE's *Trip Gen Web-Based App* and are summarized as follows:

ITE Land Use Code #821 – *Shopping Plaza*

- AM peak hour – 30% reduction (PM-10%)
- PM peak hour – 40% reduction (2021 Pass-By Rates)
- SAT peak hour – 31% reduction (2021 Pass-By Rates)

Table 2 in the Tables section at the end of this report summarizes the traffic anticipated to be generated by the proposed development. Copies of the trip generation calculations can be found in **Appendix M** at the end of this report.

6.4 Trip Distribution

6.4.1 Primary Trips

The trip generation data, the distribution of traffic at the existing study intersections, and the location of the proposed development were all used to assign the site-generated traffic to the study area roadway network. Additionally, site drive selection was governed generally[3] by the following assumptions:

- 30% of inbound trips originating from the east were assumed to utilize Site Drive A; 60% were assumed to utilize Site Drive B; the remaining 10% were assumed to utilize Site Drive C.
- 45% of outbound trips destined to the west were assumed to utilize Site Drive A; 45% were assumed to utilize to utilize Site Drive B; the remaining 10% were assumed to utilize to utilize Site Drive C.

The projected primary trip distribution and primary trips associated with the proposed GetGo can be seen graphically on **Figure 13a** and **Figure 13b**, respectively.

6.4.2 Pass-By Trips

In order to project the pass-by trip distribution for the site-generated traffic, the turning movement count data at the existing intersections were utilized. Additionally, site drive selection was once again governed generally by the same assumptions utilized for the primary trip distribution (Section 6.4.1).

[3] Engineering judgment was also utilized in estimating the distribution of both primary and pass-by trips. As such, the percentages listed in Section 6.4.1 may not be reflected exactly in the report figures, and may differ somewhat between the primary and pass-by distributions based on motorists' origins and destinations, which differ depending on whether their trip is primary or pass-by in nature.

The existing intersection distribution percentages can be seen graphically on **Figure 14**. The pass-by trip distribution percentage for each approach to the site (from the east, west, north, and south) can be seen graphically on **Figures 15a** through **15d**. Each movement on each approach was evaluated separately.

The individual (i.e. directional) pass-by trip distribution percentages were then combined to develop the overall pass-by trip distribution, which can be seen graphically on **Figure 16a**. The projected site-generated pass-by trips associated with the proposed development can be seen graphically on **Figure 16b**.

The site-generated primary trips (Figure 13b) and pass-by trips (Figure 16b) were then combined onto **Figure 17**, which depicts the total site-generated traffic associated with the proposed development.

7.0 OPENING YEAR 2024 WITH DEVELOPMENT CONDITION

7.1 Traffic Volume Development

To develop the Opening Year 2024 With Development Condition traffic volumes, the proposed site-generated trips (Figure 17) were added to the Opening Year 2024 Without Development Condition traffic volumes (Figure 11a). The resulting Opening Year 2024 With Development Condition traffic volumes can be seen graphically on **Figure 18a**.

7.2 Capacity Analysis

The analysis performed for the Opening Year 2024 With Development Condition assumed the same intersection geometry and traffic control used in the Opening Year 2024 Without Development Condition analysis.

The Opening Year 2024 With Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the study intersections during the AM, PM, and SAT peak hours.

Additionally, the capacity analyses show minimal increases in average delay (less than 10.0 seconds per vehicle) under the Opening Year 2024 With Development Condition when compared to the Opening Year 2024 Without Development Condition at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A through **1D** in the Tables section at the end of this report show the Opening Year 2024 With Development Condition AM, PM, and SAT peak hour LOS for the study

intersections. The Opening Year 2024 With Development Condition peak hour LOS can be seen graphically on **Figure 18b**.

Synchro and HCM printouts for the Opening Year 2024 With Development Condition can be found in **Appendix N** and **Appendix O**, respectively, at the end of this report.

8.0 DESIGN YEAR 2029 WITH DEVELOPMENT CONDITION

8.1 Traffic Volume Development

Similar to the Opening Year 2024 With Development Condition, the proposed site-generated trips (Figure 17) were added to the Design Year 2029 Without Development Condition traffic volumes (Figure 12a) to determine the Design Year 2029 With Development Condition traffic volumes, which can be seen graphically on **Figure 19a**.

8.2 Capacity Analysis

The analysis performed for the Design Year 2029 With Development Condition assumed the same intersection geometry and traffic control used in the Design Year 2029 Without Development Condition analysis.

The Design Year 2029 With Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the study intersections during the AM, PM, and SAT peak hours.

Additionally, the capacity analyses show minimal increases in average delay (less than 10.0 seconds per vehicle) under the Design Year 2029 With Development Condition when compared to the Design Year 2029 Without Development Condition at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A through **1D** in the Tables section at the end of this report show the Design Year 2029 With Development Condition AM, PM, and SAT peak hour LOS for the study intersections. The Design Year 2029 With Development Condition peak hour LOS can be seen graphically on **Figure 19b**.

Synchro and HCM printouts for the Design Year 2029 With Development Condition can be found in **Appendix P** and **Appendix Q**, respectively, at the end of this report.

9.0 SIGHT DISTANCE EVALUATION

Sight distance requirements were evaluated at the proposed site drives in accordance with PennDOT Title 67, Chapter 441, *Access To And Occupancy Of Highways by Driveways and Local Roads*.

Sight distance requirements were based on observed 85th percentile speeds or a design speed (posted speed limit plus 5 mph) on the abutting roadway(s), whichever is greater. The observed 85th percentile speeds along Newbury Drive were obtained via a radar speed study. The radar speed data can be found in **Appendix R** at the end of this report. Appropriate friction factors from Table B of PennDOT Publication 212 were also utilized, where appropriate, to determine the required minimum sight distances.

A complete photo log of the existing sight distance at the proposed site drives can be found in **Appendix S** at the end of this report. The results of the sight distance analyses are summarized in the table below:

Sight Line	Approach Grade (%)	Required Sight Distance (feet)	Available Sight Distance (feet)
Millers Run Road (SR 0050) with Site Drive A			
Speed Limit = 40 mph			
Corner Sight Distance Looking Left	N/A[4]	383'	>1,000'
Millers Run Road (SR 0050) with Site Drive B			
Speed Limit = 40 mph			
Corner Sight Distance Looking Left	N/A	383'	>1,000'
Newbury Drive with Site Drive C			
Speed Limit = Not Posted; 85th Percentile Speed (NB/SB) = 24 mph / 28 mph			
Corner Sight Distance Looking Left	-2.2%	176'	~270'
Corner Sight Distance Looking Right	-1.8%	201'	~270'
Stopping Sight Distance Looking Ahead	-2.8%	177'	~290'
Stopping Sight Distance from Behind	-1.8%	201'	~250'

[4] Because available sight line is greater than 1,000 feet, approach grade is inconsequential. Assumed 0.0% for calculations/table.

As demonstrated in the table, adequate sight distance is available at all of the proposed site drives.

***** UNLESS OTHERWISE NOTED, THE AVAILABLE SIGHT DISTANCES REPORTED IN THIS STUDY ASSUME THAT ON-SITE CLEARING AND/OR GRADING WILL BE PERFORMED, AS NECESSARY, IN ORDER TO MAXIMIZE SIGHT LINES TO THE EXTENT POSSIBLE *****

10.0 QUEUING ANALYSIS

Queuing analyses were performed to compare pre-development and post-development queuing at the study intersections. The Design Year 2029 Without and With Development Conditions were modeled in Synchro and transferred to SimTraffic. Five (5) separate 60-minute simulations (utilizing a ten-minute seeding interval) were performed for each individual peak hour.

Queue reports from SimTraffic for the Design Year 2029 Without and With Development Conditions can be found in **Appendix T** and **Appendix U**, respectively, at the end of this report. Spreadsheets summarizing the 95th percentile queue averages are also included. The results for the Design Year 2029 Without and With Development Condition queues are summarized in **Tables 3A** through **3D** for the AM, PM, and SAT peak hours.

As can be seen in the Tables, queue lengths are generally not anticipated to increase significantly (no more than 1-2 car lengths) under the Design Year 2029 With Development Condition when compared to the Design Year 2029 Without Development Condition.

In addition, queue lengths are not projected to exceed their respective storage capacities under the Design Year 2029 Without or With Development Conditions during the AM, PM, or SAT peak hours.

11.0 SUMMARY AND CONCLUSIONS

In conclusion, the proposed development in South Fayette Township, Allegheny County, Pennsylvania is not anticipated to impact traffic operations in the study area. The following is a summary of the results and recommended improvements at each of the study intersections:

Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 10.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- No roadway improvements are recommended.

Newbury Drive with Plaza Access / Site Drive C

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 10.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- Construct a full access site drive (Site Drive C) along the western side of Newbury Drive, as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive A

- Construct a right-in / right-out access site drive (Site Drive A) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive B

- Construct a right-in / right-out access site drive (Site Drive B) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

REPORT TABLES

Table 1A - Level-of-Service Summary

Intersection		AM Peak Hour Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive						
Millers Run Road (SR 0050)		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn	C (24.8)	C (26.9)	C (26.2)	NM	C (27.4)	C (26.9)	NM
	Through Right Turn	B (14.4)	B (17.0)	B (17.1)	NM	B (17.2)	B (17.4)	NM
	Approach	**B (14.8)**	**B (17.3)**	**B (17.7)**	**NM**	**B (17.6)**	**B (17.9)**	**NM**
Westbound	Left Turn	C (23.5)	C (25.2)	C (25.3)	NM	C (25.8)	C (25.9)	NM
	Through	B (11.6)	B (11.8)	B (12.7)	NM	B (11.9)	B (12.8)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	**B (13.2)**	**B (15.1)**	**B (15.7)**	**NM**	**B (15.2)**	**B (15.9)**	**NM**
Newbury Drive / Todd A. Miller Drive		North/South Roadway						
Northbound	Left Turn	C (33.9)	C (25.1)	C (25.2)	NM	C (25.8)	C (25.9)	NM
	Through	C (28.3)	C (26.3)	C (26.5)	NM	C (27.0)	C (27.2)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	**C (29.8)**	**C (25.5)**	**C (25.6)**	**NM**	**C (26.2)**	**C (26.3)**	**NM**
Southbound	Left Turn	C (21.0)	C (22.9)	C (22.8)	NM	C (23.4)	C (23.3)	NM
	Through Right Turn	B (19.2)	C (23.2)	C (22.9)	NM	C (23.6)	C (23.3)	NM
	Approach	**C (20.8)**	**C (22.9)**	**C (22.8)**	**NM**	**C (23.4)**	**C (23.3)**	**NM**
	Overall Intersection	***B (15.4)***	***B (18.0)***	***B (18.3)***	***NM***	***B (18.2)***	***B (18.6)***	***NM***

NM = No Mitigation

Intersection		PM Peak Hour Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive						
Millers Run Road (SR 0050)		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn	C (24.5)	C (25.8)	C (26.4)	NM	C (26.0)	C (27.1)	NM
	Through Right Turn	B (14.0)	B (16.0)	B (16.5)	NM	B (16.4)	B (16.7)	NM
	Approach	**B (14.8)**	**B (16.8)**	**B (17.7)**	**NM**	**B (17.1)**	**B (18.0)**	**NM**
Westbound	Left Turn	C (24.7)	C (26.7)	C (27.3)	NM	C (26.9)	C (27.9)	NM
	Through	B (14.9)	B (15.3)	B (17.0)	NM	B (15.8)	B (17.3)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	**B (15.4)**	**B (16.8)**	**B (18.2)**	**NM**	**B (17.2)**	**B (18.6)**	**NM**
Newbury Drive / Todd A. Miller Drive		North/South Roadway						
Northbound	Left Turn	C (27.2)	C (25.4)	C (25.9)	NM	C (25.5)	C (26.4)	NM
	Through	C (25.6)	C (25.7)	C (26.2)	NM	C (25.9)	C (26.7)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	**C (26.2)**	**C (25.5)**	**C (26.0)**	**NM**	**C (25.7)**	**C (26.5)**	**NM**
Southbound	Left Turn	C (20.8)	C (22.1)	C (22.2)	NM	C (22.1)	C (22.6)	NM
	Through Right Turn	B (19.1)	C (22.0)	C (21.5)	NM	C (21.9)	C (21.8)	NM
	Approach	**C (20.5)**	**C (22.1)**	**C (22.1)**	**NM**	**C (22.1)**	**C (22.4)**	**NM**
	Overall Intersection	***B (16.5)***	***B (18.3)***	***B (19.3)***	***NM***	***B (18.6)***	***B (19.6)***	***NM***

NM = No Mitigation

Intersection		SAT Peak Hour Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive						
Millers Run Road (SR 0050)		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn	C (24.7)	C (26.6)	C (27.9)	NM	C (26.7)	C (27.8)	NM
	Through Right Turn	B (13.9)	B (17.4)	B (17.9)	NM	B (17.8)	B (18.4)	NM
	Approach	**B (14.7)**	**B (18.1)**	**B (19.3)**	**NM**	**B (18.6)**	**B (19.7)**	**NM**
Westbound	Left Turn	C (25.5)	C (27.1)	C (27.7)	NM	C (27.3)	C (27.8)	NM
	Through	B (14.3)	B (15.3)	B (17.0)	NM	B (15.7)	B (17.6)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	**B (14.8)**	**B (17.3)**	**B (18.7)**	**NM**	**B (17.6)**	**B (19.1)**	**NM**
Newbury Drive / Todd A. Miller Drive		North/South Roadway						
Northbound	Left Turn	C (31.0)	C (26.1)	C (26.7)	NM	C (26.3)	C (26.9)	NM
	Through	C (27.7)	C (26.4)	C (26.9)	NM	C (26.6)	C (27.1)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	**C (28.9)**	**C (26.2)**	**C (26.8)**	**NM**	**C (26.4)**	**C (27.0)**	**NM**
Southbound	Left Turn	C (20.6)	C (22.4)	C (22.5)	NM	C (22.4)	C (22.6)	NM
	Through Right Turn	B (18.4)	C (22.1)	C (21.7)	NM	C (22.0)	C (21.6)	NM
	Approach	**C (20.3)**	**C (22.3)**	**C (22.3)**	**NM**	**C (22.3)**	**C (22.4)**	**NM**
	Overall Intersection	***B (16.3)***	***B (19.4)***	***C (20.3)***	***NM***	***B (19.6)***	***C (20.6)***	***NM***

NM = No Mitigation

Table 1B - Level-of-Service Summary

		AM Peak Hour						
Intersection		Newbury Drive with Plaza Access / Site Drive C						
Plaza Access / Site Drive C		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn			B (10.1)	NM		B (10.2)	NM
	Through							
	Right Turn							
	Approach			**B (10.1)**	**NM**		**B (10.2)**	**NM**
Westbound	Left Turn	B (11.6)	B (11.9)	B (13.4)	NM	B (12.0)	B (13.7)	NM
	Through							
	Right Turn							
	Approach	**B (11.6)**	**B (11.9)**	**B (13.4)**	**NM**	**B (12.0)**	**B (13.7)**	**NM**
Newbury Drive		North/South Roadway						
Northbound	Left Turn			A (7.8)	NM		A (7.9)	NM
	Through	FREE	FREE	A (0.1)	NM	FREE	A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	**FREE**	**FREE**	**A (0.7)**	**NM**	**FREE**	**A (0.7)**	**NM**
Southbound	Left Turn	A (7.9)	A (7.9)	A (7.9)	NM	A (7.9)	A (7.9)	NM
	Through			A (0.1)	NM		A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	**A (0.7)**	**A (0.7)**	**A (0.7)**	**NM**	**A (0.7)**	**A (0.6)**	**NM**
	Overall Intersection	***A (1.3)***	***A (1.3)***	***A (2.0)***	***NM***	***A (1.3)***	***A (2.0)***	***NM***

NM = No Mitigation

		PM Peak Hour						
Intersection		Newbury Drive with Plaza Access / Site Drive C						
Plaza Access / Site Drive C		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn			B (13.3)	NM		B (13.6)	NM
	Through							
	Right Turn							
	Approach			**B (13.3)**	**NM**		**B (13.6)**	**NM**
Westbound	Left Turn	B (13.7)	B (14.5)	C (19.6)	NM	B (15.0)	C (20.6)	NM
	Through							
	Right Turn							
	Approach	**B (13.7)**	**B (14.5)**	**C (19.6)**	**NM**	**B (15.0)**	**C (20.6)**	**NM**
Newbury Drive		North/South Roadway						
Northbound	Left Turn			A (8.3)	NM		A (8.4)	NM
	Through	FREE	FREE	A (0.2)	NM	FREE	A (0.2)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	**FREE**	**FREE**	**A (1.0)**	**NM**	**FREE**	**A (0.9)**	**NM**
Southbound	Left Turn	A (8.2)	A (8.3)	A (8.3)	NM	A (8.3)	A (8.3)	NM
	Through			A (0.1)	NM		A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	**A (0.4)**	**A (0.4)**	**A (0.4)**	**NM**	**A (0.4)**	**A (0.4)**	**NM**
	Overall Intersection	***A (1.6)***	***A (1.5)***	***A (3.3)***	***NM***	***A (1.5)***	***A (3.2)***	***NM***

NM = No Mitigation

		SAT Peak Hour						
Intersection		Newbury Drive with Plaza Access / Site Drive C						
Plaza Access / Site Drive C		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn			C (15.9)	NM		C (16.4)	NM
	Through							
	Right Turn							
	Approach			**C (15.9)**	**NM**		**C (16.4)**	**NM**
Westbound	Left Turn	C (20.3)	C (24.5)	F (59.1)	NM	D (26.0)	F (67.5)	NM
	Through							
	Right Turn							
	Approach	**C (20.3)**	**C (24.5)**	**F (59.1)**	**NM**	**D (26.0)**	**F (67.5)**	**NM**
Newbury Drive		North/South Roadway						
Northbound	Left Turn			A (8.5)	NM		A (8.5)	NM
	Through	FREE	FREE	A (0.3)	NM	FREE	A (0.3)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	**FREE**	**FREE**	**A (1.0)**	**NM**	**FREE**	**A (1.0)**	**NM**
Southbound	Left Turn	A (8.6)	A (8.8)	A (8.8)	NM	A (8.9)	A (8.8)	NM
	Through			A (0.1)	NM		A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	**A (0.6)**	**A (0.5)**	**A (0.5)**	**NM**	**A (0.5)**	**A (0.5)**	**NM**
	Overall Intersection	***A (3.1)***	***A (3.3)***	***A (8.3)***	***NM***	***A (3.3)***	***A (9.0)***	***NM***

NM = No Mitigation

Table 1C - Level-of-Service Summary

		AM Peak Hour						
Intersection		Millers Run Road (SR 0050) with Site Drive A						
Millers Run Road (SR 0050)		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through							
	Right Turn			A (9.8)	NM		A (9.9)	NM
	Approach			**A (9.8)**	**NM**		**A (9.9)**	**NM**
	Overall Intersection			***A (0.0)***	***NM***		***A (0.0)***	***NM***

NM = No Mitigation

		PM Peak Hour						
Intersection		Millers Run Road (SR 0050) with Site Drive A						
Millers Run Road (SR 0050)		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through							
	Right Turn			B (11.7)	NM		B (11.9)	NM
	Approach			**B (11.7)**	**NM**		**B (11.9)**	**NM**
	Overall Intersection			***A (0.1)***	***NM***		***A (0.1)***	***NM***

NM = No Mitigation

		SAT Peak Hour						
Intersection		Millers Run Road (SR 0050) with Site Drive A						
Millers Run Road (SR 0050)		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through							
	Right Turn			B (10.8)	NM		B (11.0)	NM
	Approach			**B (10.8)**	**NM**		**B (11.0)**	**NM**
	Overall Intersection			***A (0.1)***	***NM***		***A (0.1)***	***NM***

NM = No Mitigation

Table 1D - Level-of-Service Summary

Intersection		AM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive B						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through							
	Right Turn			A (9.8)	NM		A (9.9)	NM
	Approach			**A (9.8)**	**NM**		**A (9.9)**	**NM**
	Overall Intersection			***A (0.0)***	***NM***		***A (0.0)***	***NM***

NM = No Mitigation

Intersection		PM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive B						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through							
	Right Turn			B (11.8)	NM		B (12.0)	NM
	Approach			**B (11.8)**	**NM**		**B (12.0)**	**NM**
	Overall Intersection			***A (0.2)***	***NM***		***A (0.2)***	***NM***

NM = No Mitigation

Intersection		SAT Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive B						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			**FREE**	**NM**		**FREE**	**NM**
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through							
	Right Turn			B (11.0)	NM		B (11.1)	NM
	Approach			**B (11.0)**	**NM**		**B (11.1)**	**NM**
	Overall Intersection			***A (0.1)***	***NM***		***A (0.1)***	***NM***

Table 2
Trip Generation Summary

TIME PERIOD	ANTICIPATED TRIP GENERATION		
	IN	OUT	TOTAL
LUC #821 – *Shopping Plaza (40-150k) without Supermarket* – 45,126 SF			
ADT	**1,524**	**1,524**	**3,048**
AM Peak Hour	**48**	**30**	**59**
Primary Trips	*34*	*21*	*55*
Pass-By Trips (26%)	*14*	*9*	*13*
PM Peak Hour	**115**	**119**	**234**
Primary Trips	*69*	*71*	*140*
Pass-By Trips (36%)	*46*	*48*	*94*
SAT Peak Hour	**131**	**120**	**251**
Primary Trips	*90*	*83*	*173*
Pass-By Trips (26%)	*41*	*37*	*78*

Table 3A - Queue Summary

Intersection		AM Peak Hour Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn	225'	50'	81'
	Through		194'	195'
	Through		171'	172'
	Right Turn			
Westbound	Left Turn	200'	113'	126'
	Through		125'	133'
	Through		74'	95'
	Right Turn	350'	11'	25'
Newbury Drive / Todd A. Miller Drive		North/South Roadway		
Northbound	Left Turn		74'	72'
	Through		64'	69'
	Right Turn		37'	29'
Southbound	Left Turn	375'	115'	110'
	Left Turn		142'	134'
	Through		91'	81'
	Right Turn			

Intersection		PM Peak Hour Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn	225'	73'	99'
	Through		170'	163'
	Through		135'	148'
	Right Turn			
Westbound	Left Turn	200'	121'	112'
	Through		202'	217'
	Through		159'	178'
	Right Turn	350'	32'	52'
Newbury Drive / Todd A. Miller Drive		North/South Roadway		
Northbound	Left Turn		87'	86'
	Through		69'	67'
	Right Turn		48'	43'
Southbound	Left Turn	375'	154'	171'
	Left Turn		181'	198'
	Through		116'	124'
	Right Turn			

Intersection		SAT Peak Hour Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn	225'	79'	109'
	Through		169'	163'
	Through		147'	142'
	Right Turn			
Westbound	Left Turn	200'	104'	103'
	Through		178'	188'
	Through		124'	146'
	Right Turn	350'	49'	58'
Newbury Drive / Todd A. Miller Drive		North/South Roadway		
Northbound	Left Turn		91'	91'
	Through		72'	70'
	Right Turn		34'	44'
Southbound	Left Turn	375'	155'	176'
	Left Turn		180'	202'
	Through		116'	118'
	Right Turn			

Table 3B - Queue Summary

Intersection		AM Peak Hour Newbury Drive with Plaza Access / Site Drive C		
Plaza Access / Site Drive C		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			17'
	Through			
	Right Turn			
Westbound	Left Turn		39'	49'
	Through			
	Right Turn			
Newbury Drive		North/South Roadway		
Northbound	Left Turn			24'
	Through		0'	
	Through		3'	0'
	Right Turn			
Southbound	Left Turn		30'	20'
	Through			
	Through		0'	0'
	Right Turn			

Intersection		PM Peak Hour Newbury Drive with Plaza Access / Site Drive C		
Plaza Access / Site Drive C		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			39'
	Through			
	Right Turn			
Westbound	Left Turn		60'	68'
	Through			
	Right Turn			
Newbury Drive		North/South Roadway		
Northbound	Left Turn			44'
	Through		0'	
	Through		0'	3'
	Right Turn			
Southbound	Left Turn		37'	23'
	Through			
	Through		0'	3'
	Right Turn			

Intersection		SAT Peak Hour Newbury Drive with Plaza Access / Site Drive C		
Plaza Access / Site Drive C		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			47'
	Through			
	Right Turn			
Westbound	Left Turn		92'	143'
	Through			
	Right Turn			
Newbury Drive		North/South Roadway		
Northbound	Left Turn			51'
	Through		0'	
	Through		9'	5'
	Right Turn			
Southbound	Left Turn		51'	37'
	Through			
	Through		0'	6'
	Right Turn			

Table 3C - Queue Summary

Intersection		AM Peak Hour Millers Run Road (SR 0050) with Site Drive A		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Site Drive A		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			0'

Intersection		PM Peak Hour Millers Run Road (SR 0050) with Site Drive A		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Site Drive A		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			7'

Intersection		SAT Peak Hour Millers Run Road (SR 0050) with Site Drive A		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Site Drive A		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			5'

Table 3D - Queue Summary

Intersection		AM Peak Hour Millers Run Road (SR 0050) with Site Drive B		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Site Drive B		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			0'

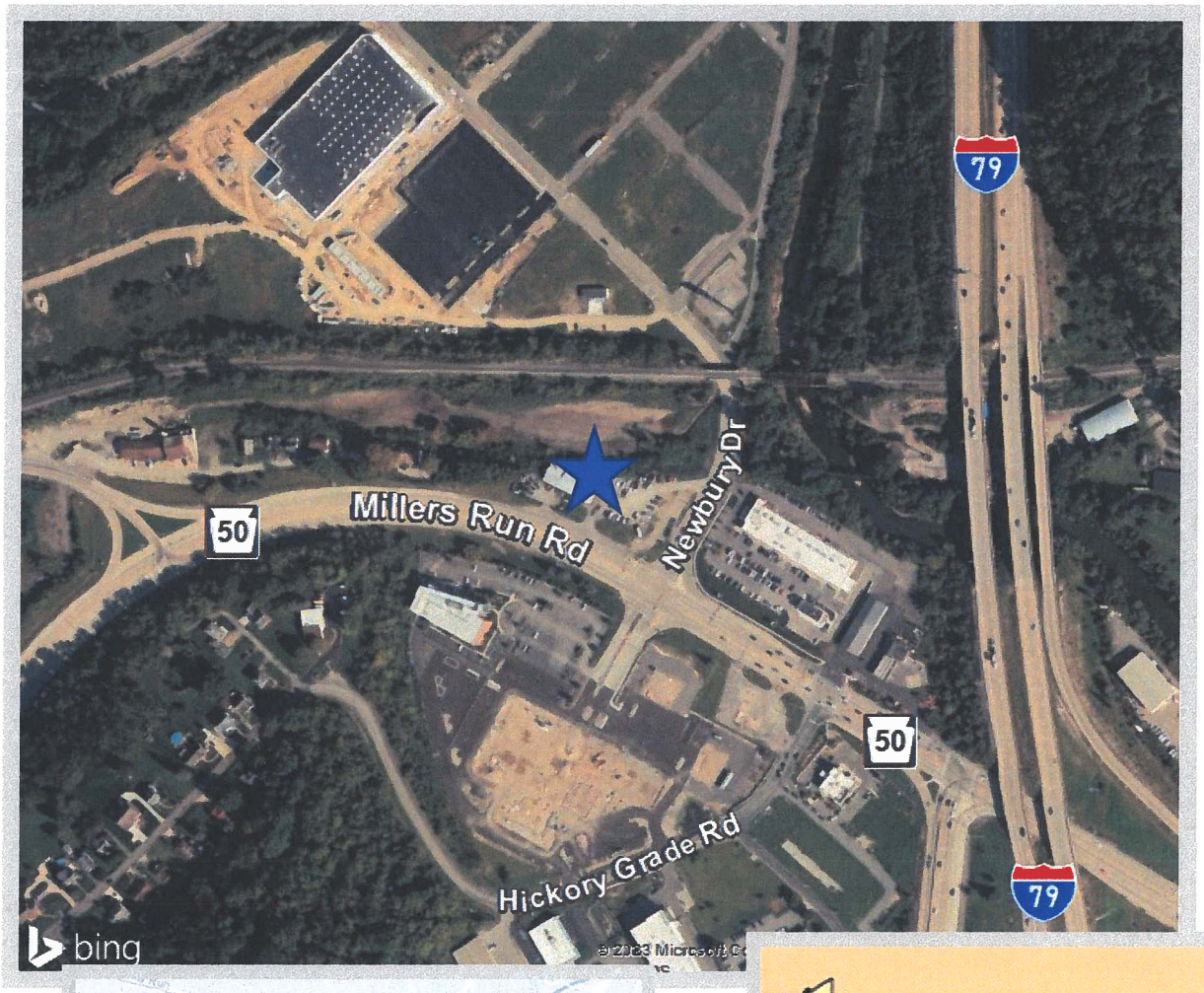
Intersection		PM Peak Hour Millers Run Road (SR 0050) with Site Drive B		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Site Drive B		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			8'

Intersection		SAT Peak Hour Millers Run Road (SR 0050) with Site Drive B		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development *95th Percentile Queue Length*	Design Year 2029 With Development *95th Percentile Queue Length*
Eastbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			
	Through			0'
	Through			0'
	Right Turn			
Site Drive B		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			9'

REPORT FIGURES

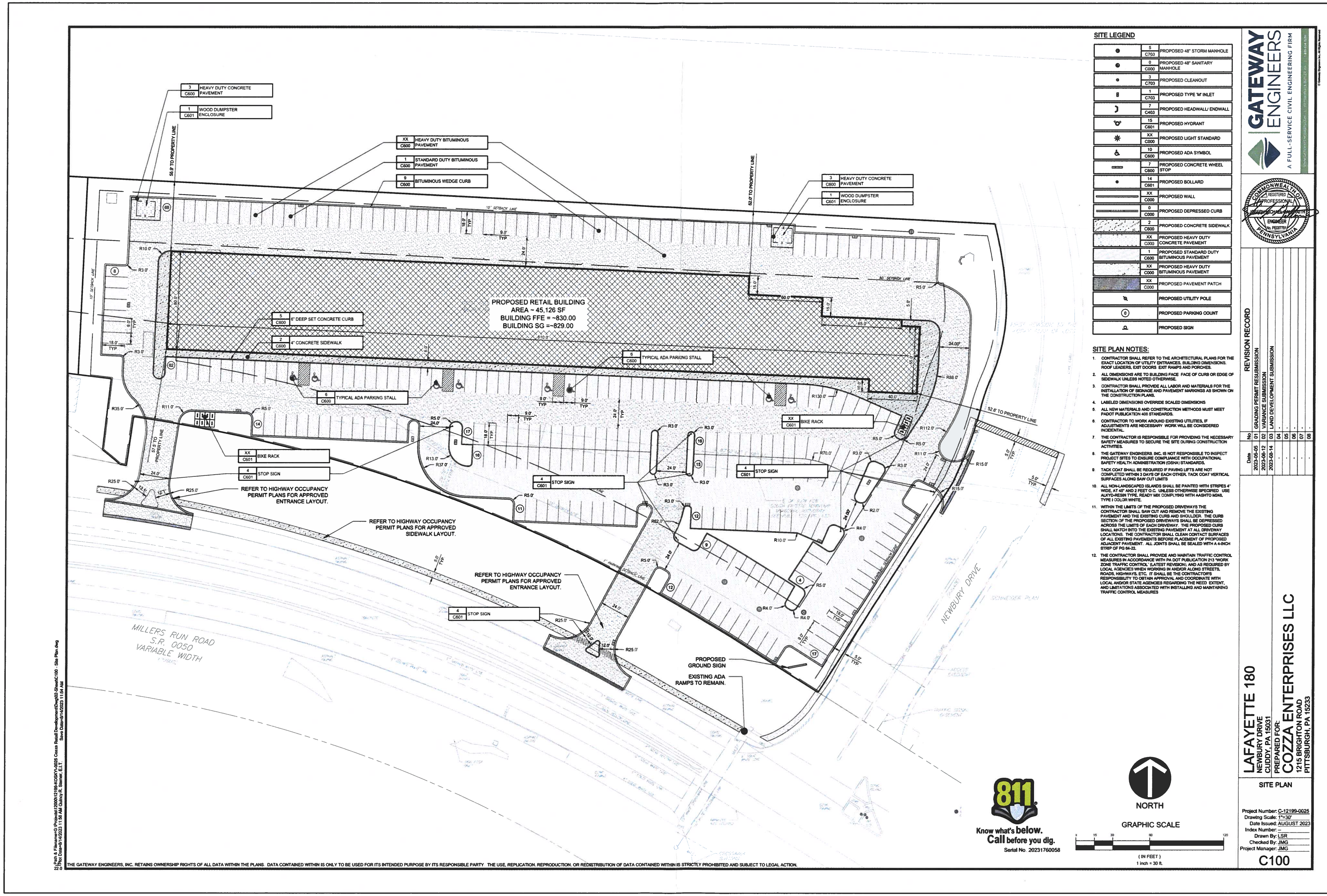
Proposed Development

South Fayette Township, Allegheny County, Pennsylvania



WOOSTER
TRAFFIC ▲ PERMITS ▲ SIGNALS

Figure 1

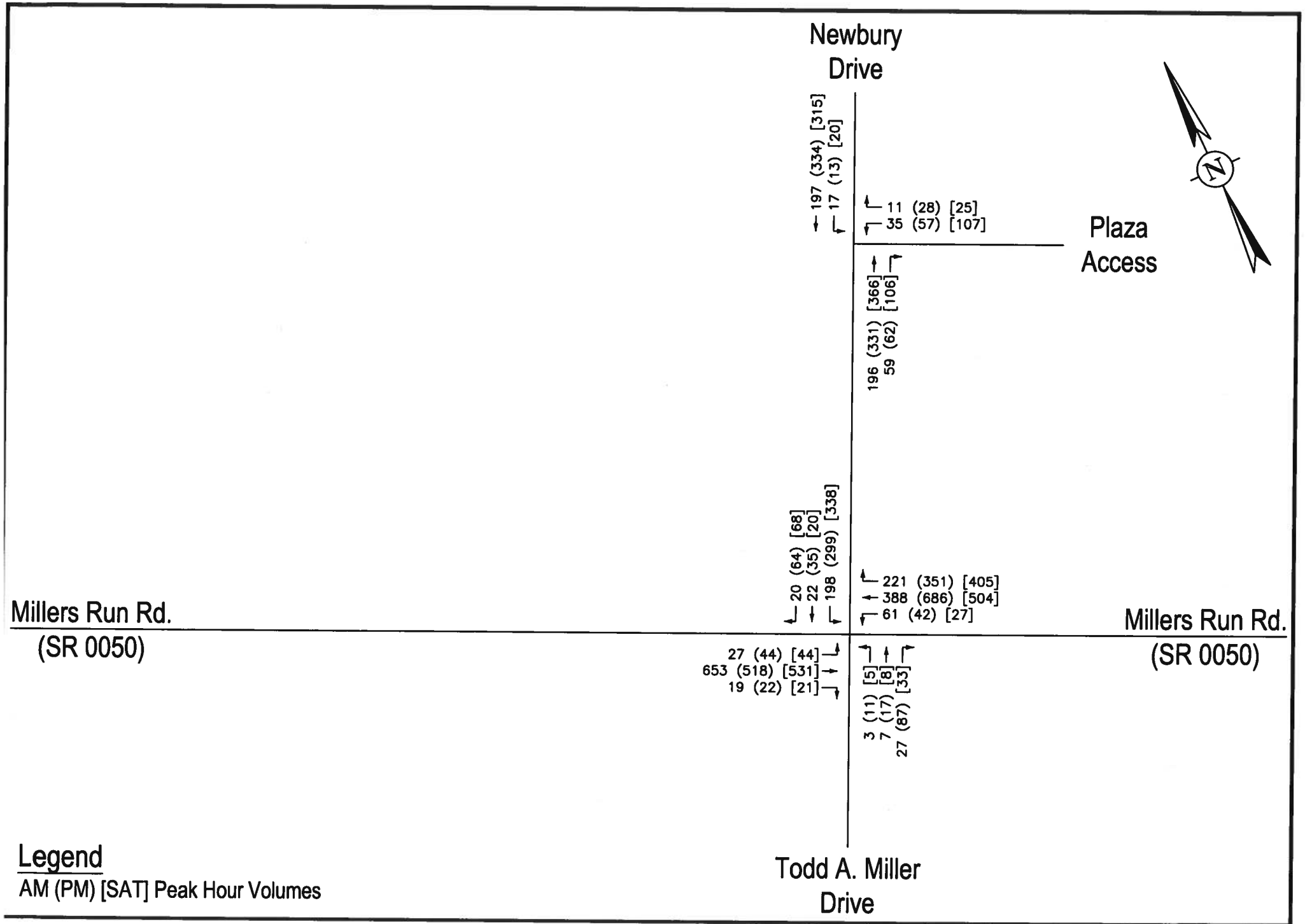


NOTE:
NOT TO SCALE

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Preliminary Site Plan

DAVID E. WOOSTER & ASSOC., LLC

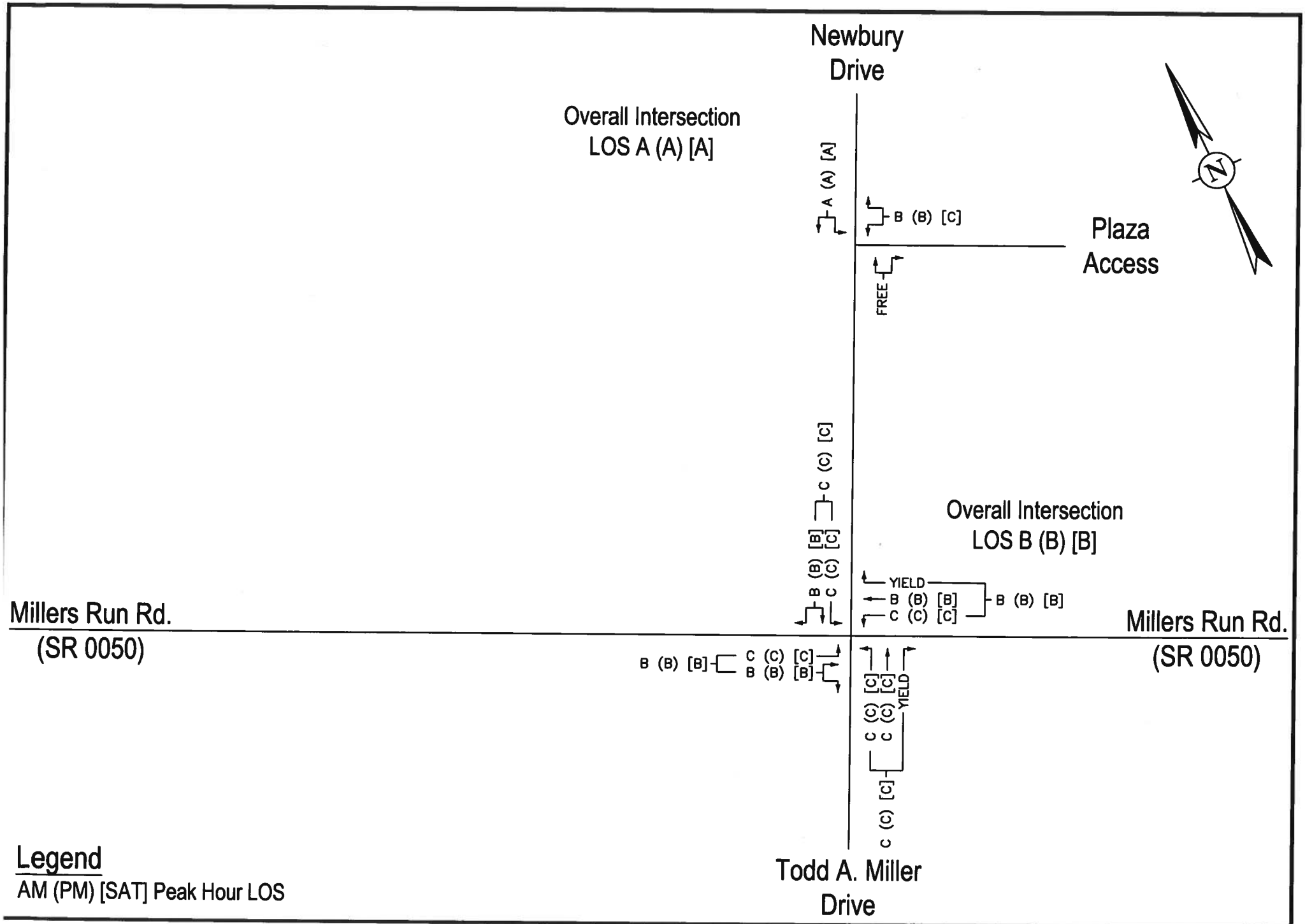
FIGURE 2
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Existing Year 2023 Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

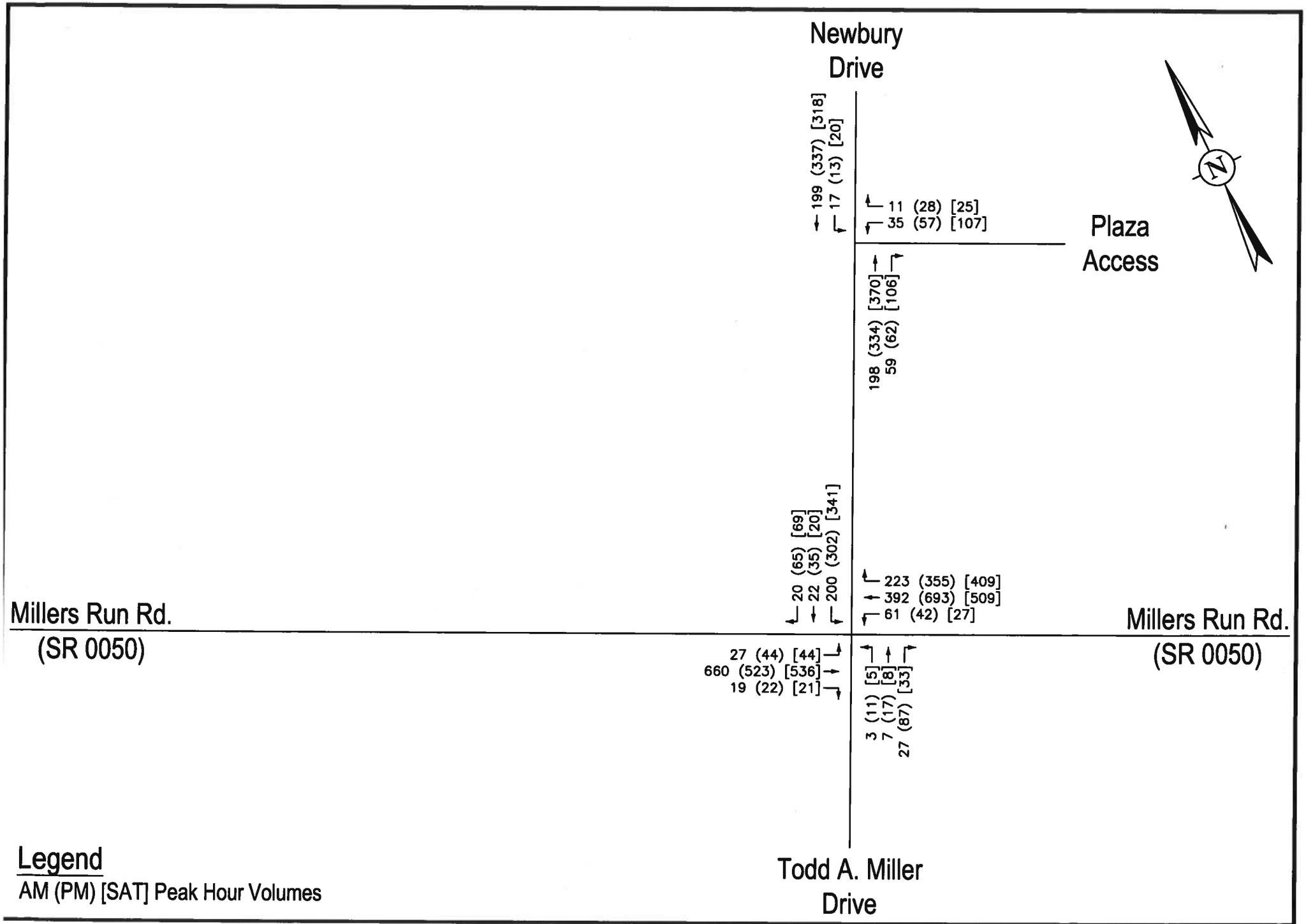
FIGURE 3a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Existing Year 2023 Condition Peak Hour LOS

DAVID E. WOOSTER & ASSOC., LLC

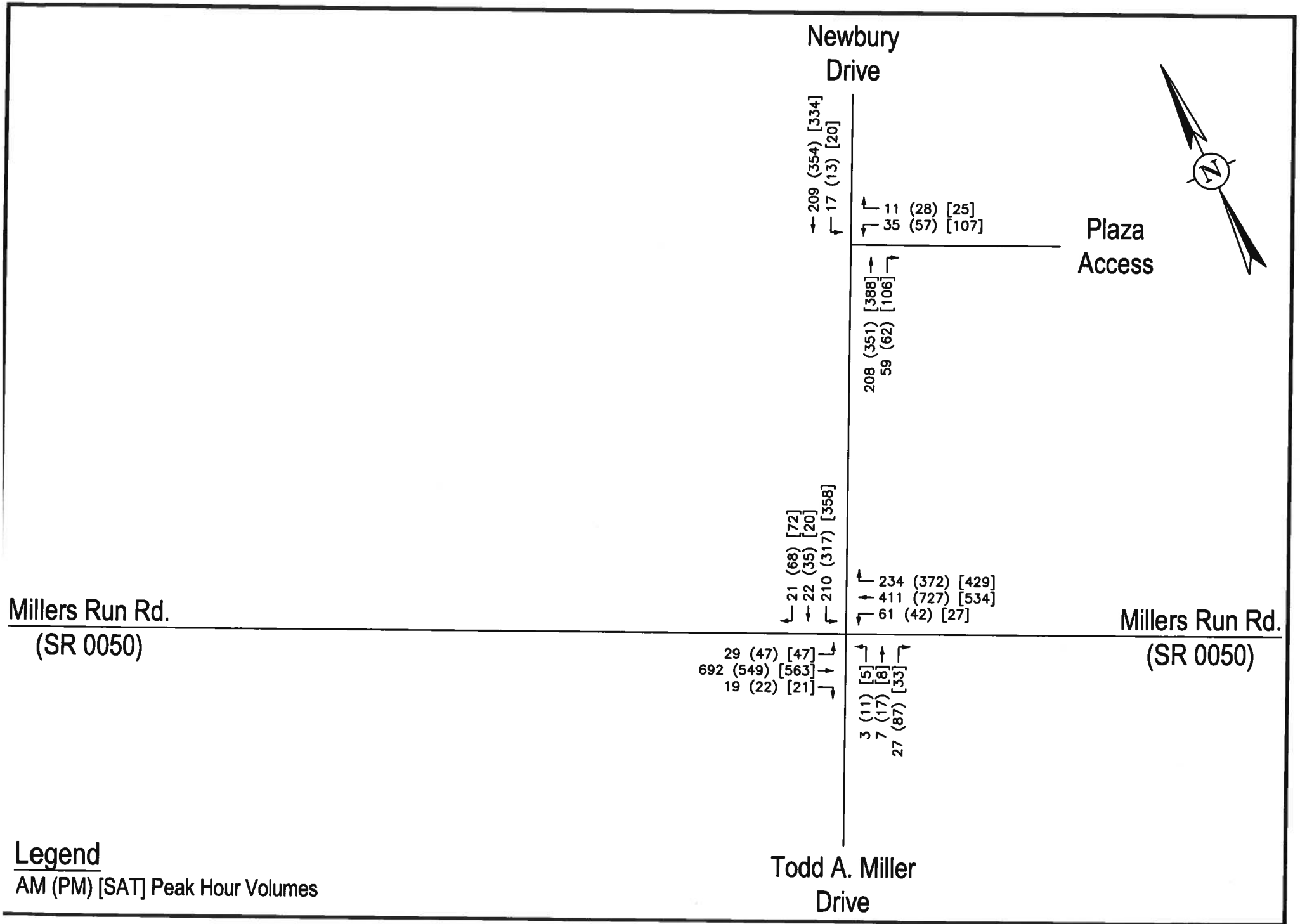
FIGURE 3b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 Base Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

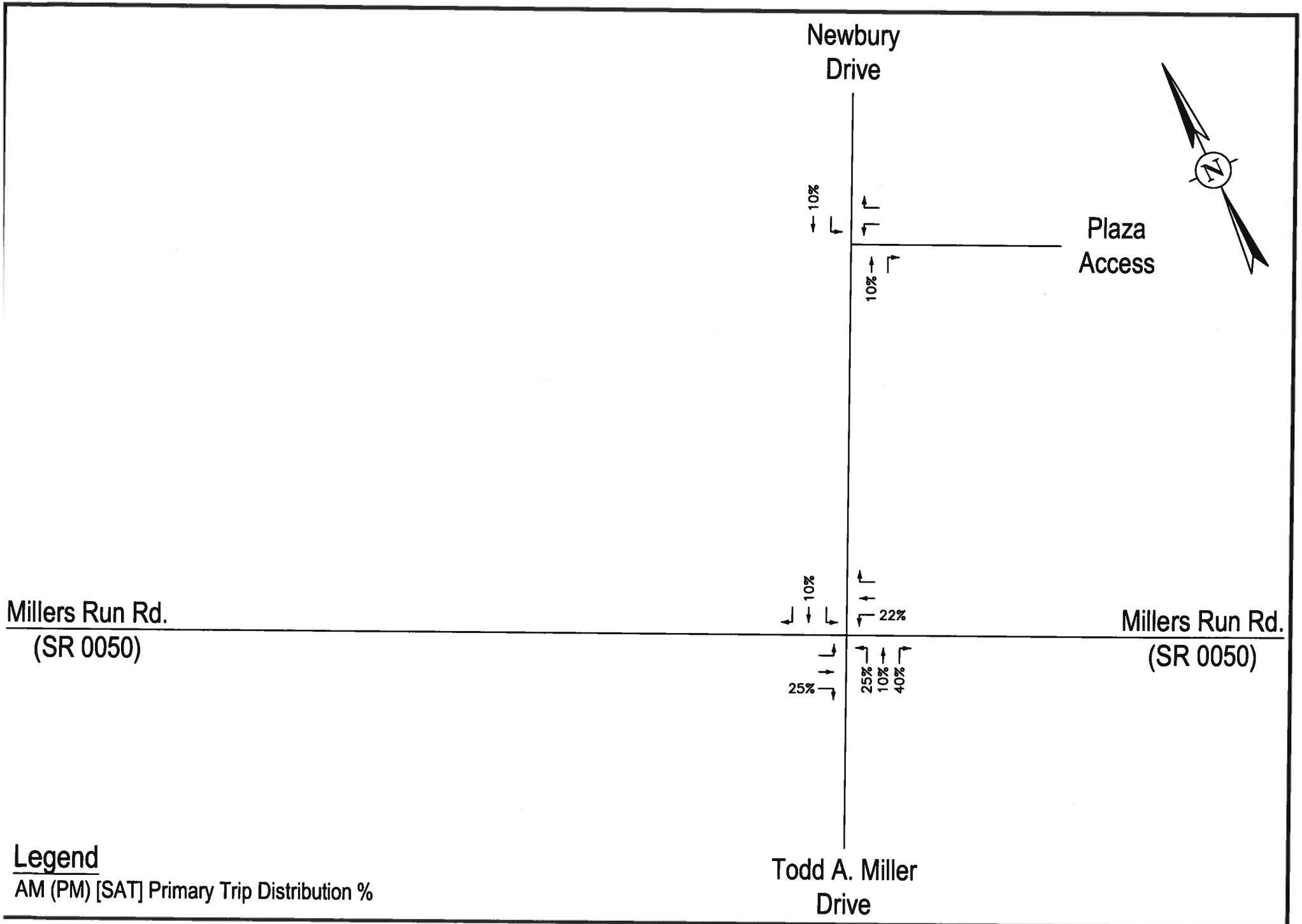
FIGURE 4
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 Base Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 5
4392-092723



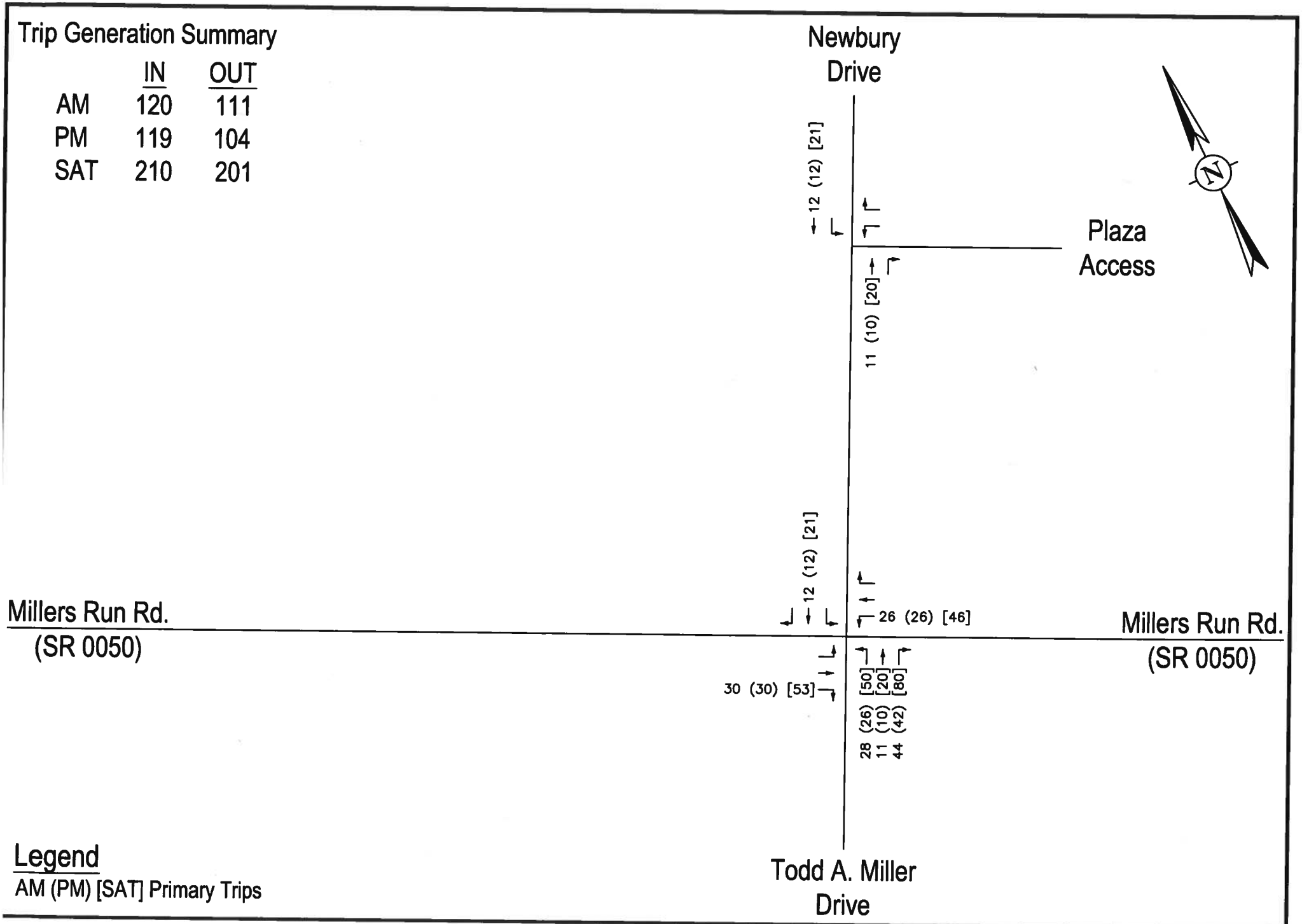
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trip Distribution Percentages (The Piazza Retail Development)

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 6a
4392-092723

Trip Generation Summary

	IN	OUT
AM	120	111
PM	119	104
SAT	210	201



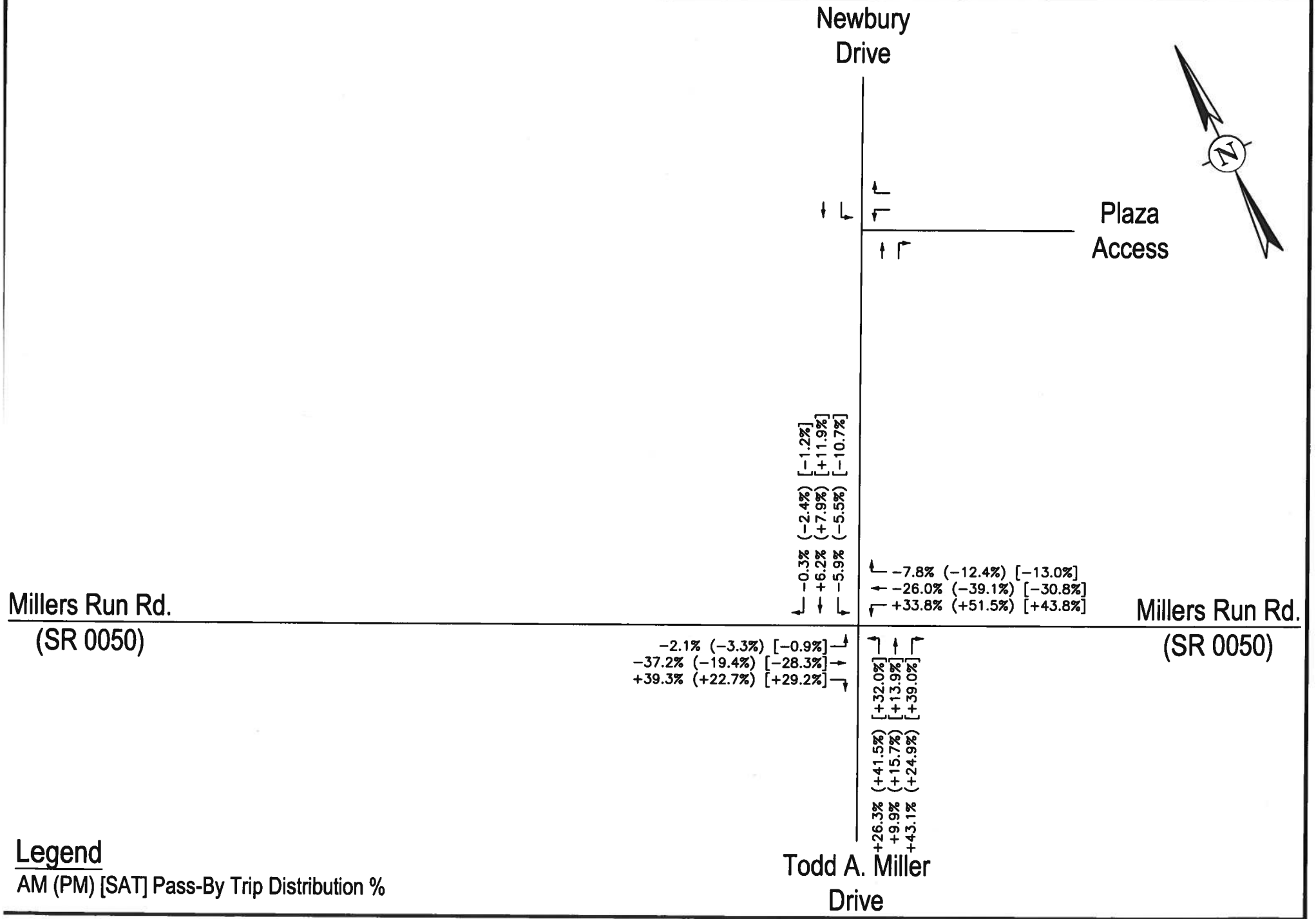
Legend

AM (PM) [SAT] Primary Trips

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trips (The Piazza Retail Development)

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 6b
4392-092723



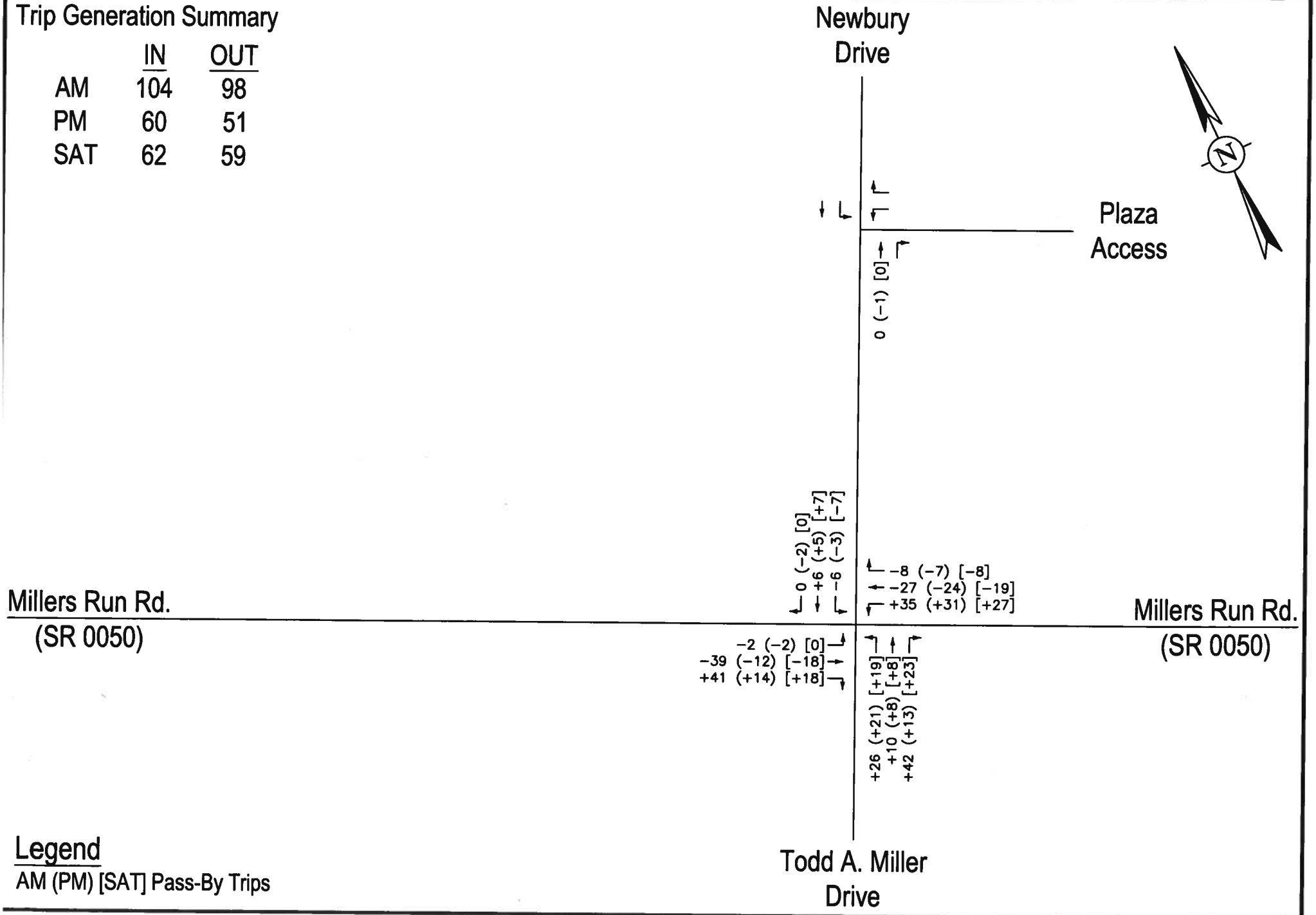
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass–By Trip Distribution Percentages (The Piazza Retail Development)

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 7a
4392-092723

Trip Generation Summary

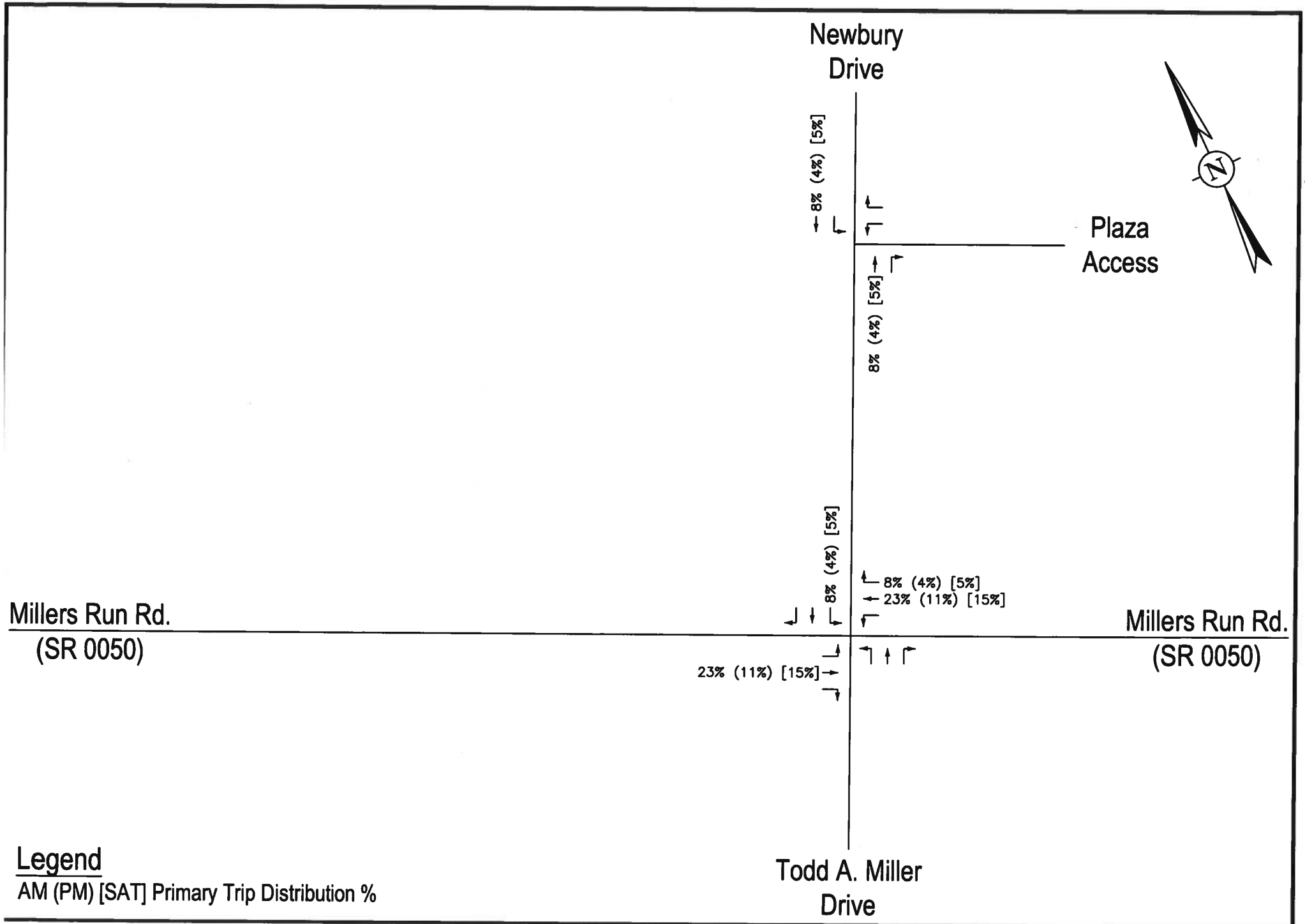
	IN	OUT
AM	104	98
PM	60	51
SAT	62	59



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass–By Trips (The Piazza Retail Development)

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 7b
4392-092723



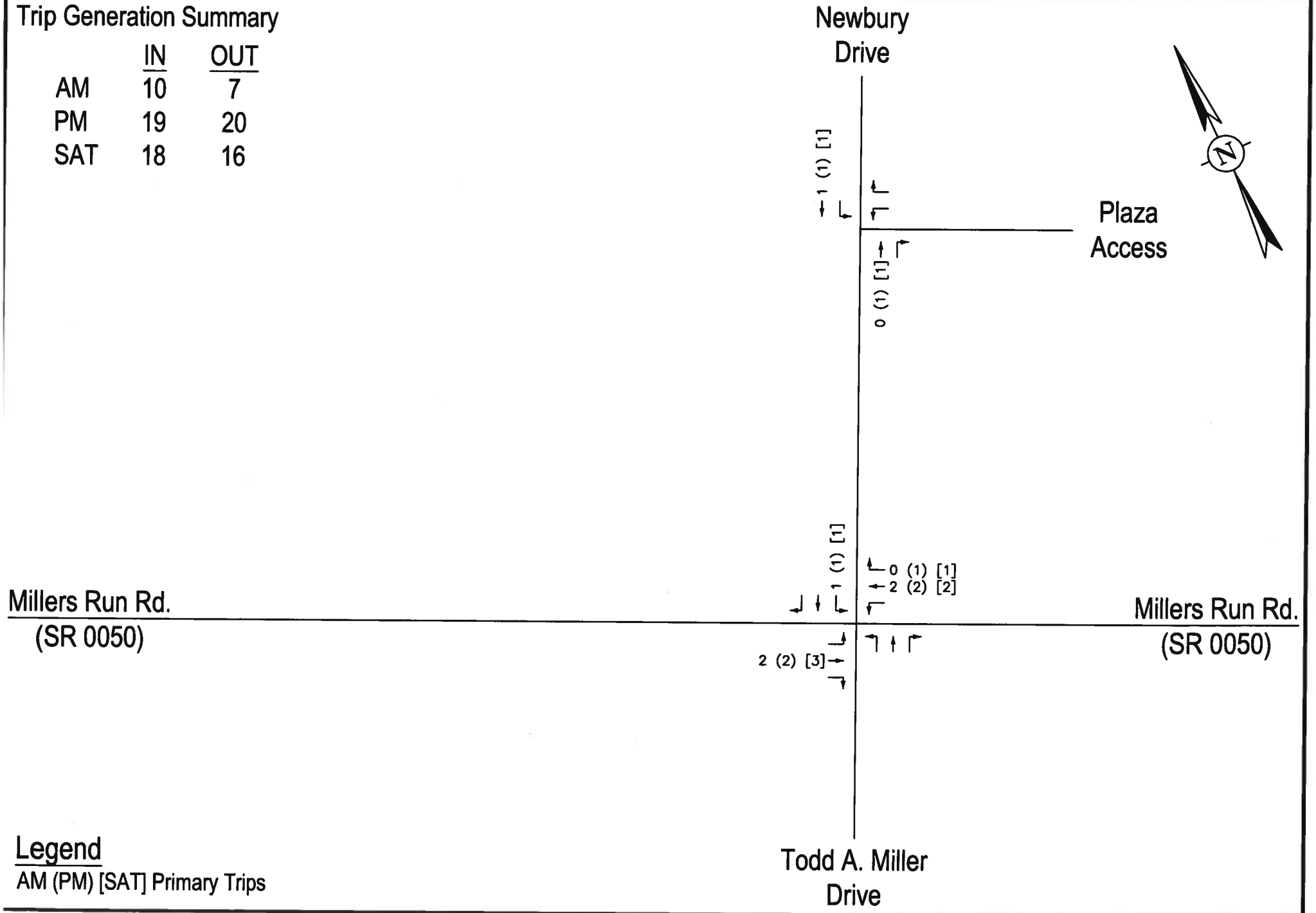
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trip Distribution Percentages (South Fayette Commons Development)

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 8a
4392-092723

Trip Generation Summary

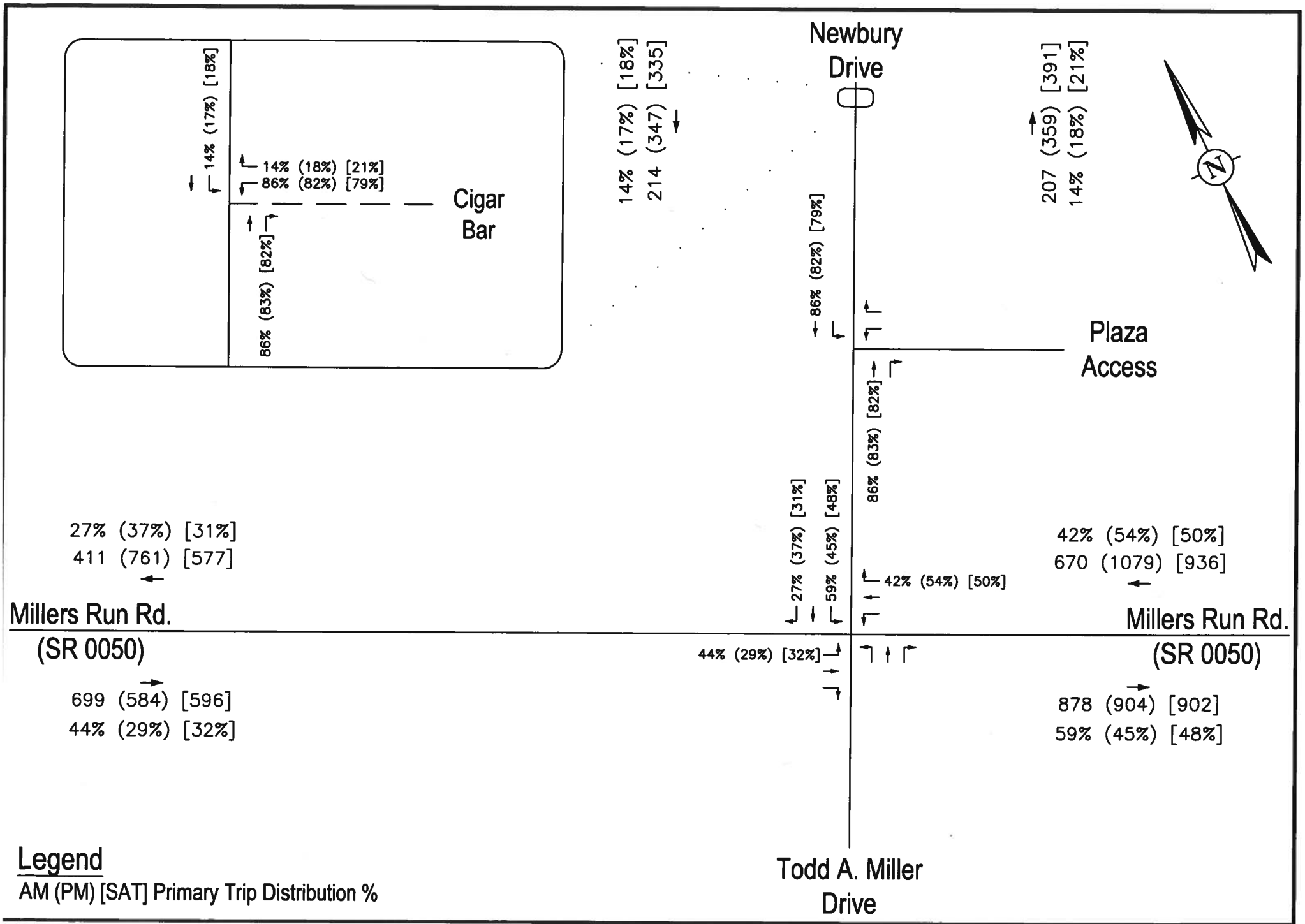
	IN	OUT
AM	10	7
PM	19	20
SAT	18	16



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trips (South Fayette Commons Development)

DAVID E. WOOSTER & ASSOC., LLC

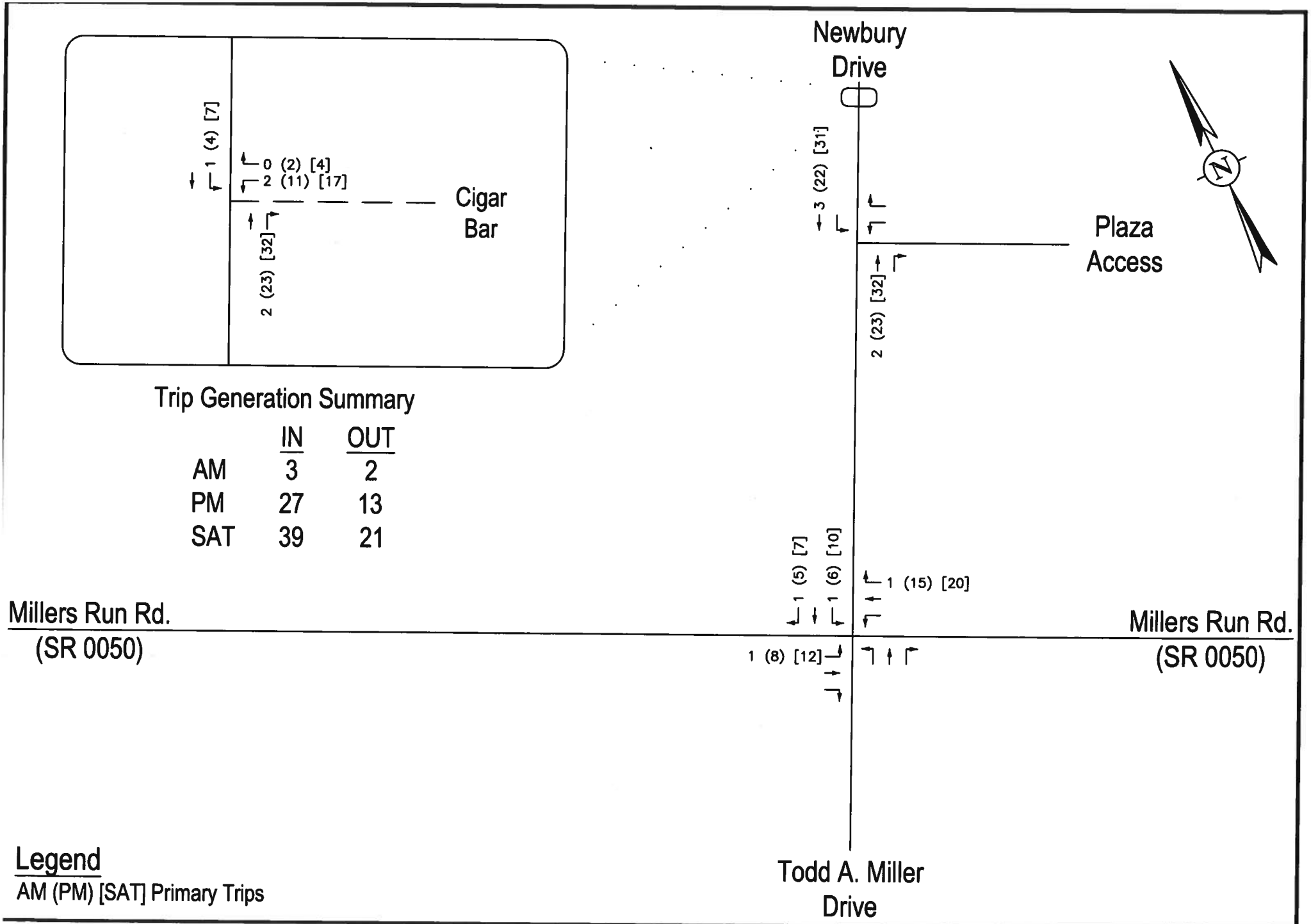
FIGURE 8b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trip Distribution Percentages (Cigar Lounge, Bar & Restaurant Development)

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 9a
4392-092723



Trip Generation Summary

	IN	OUT
AM	3	2
PM	27	13
SAT	39	21

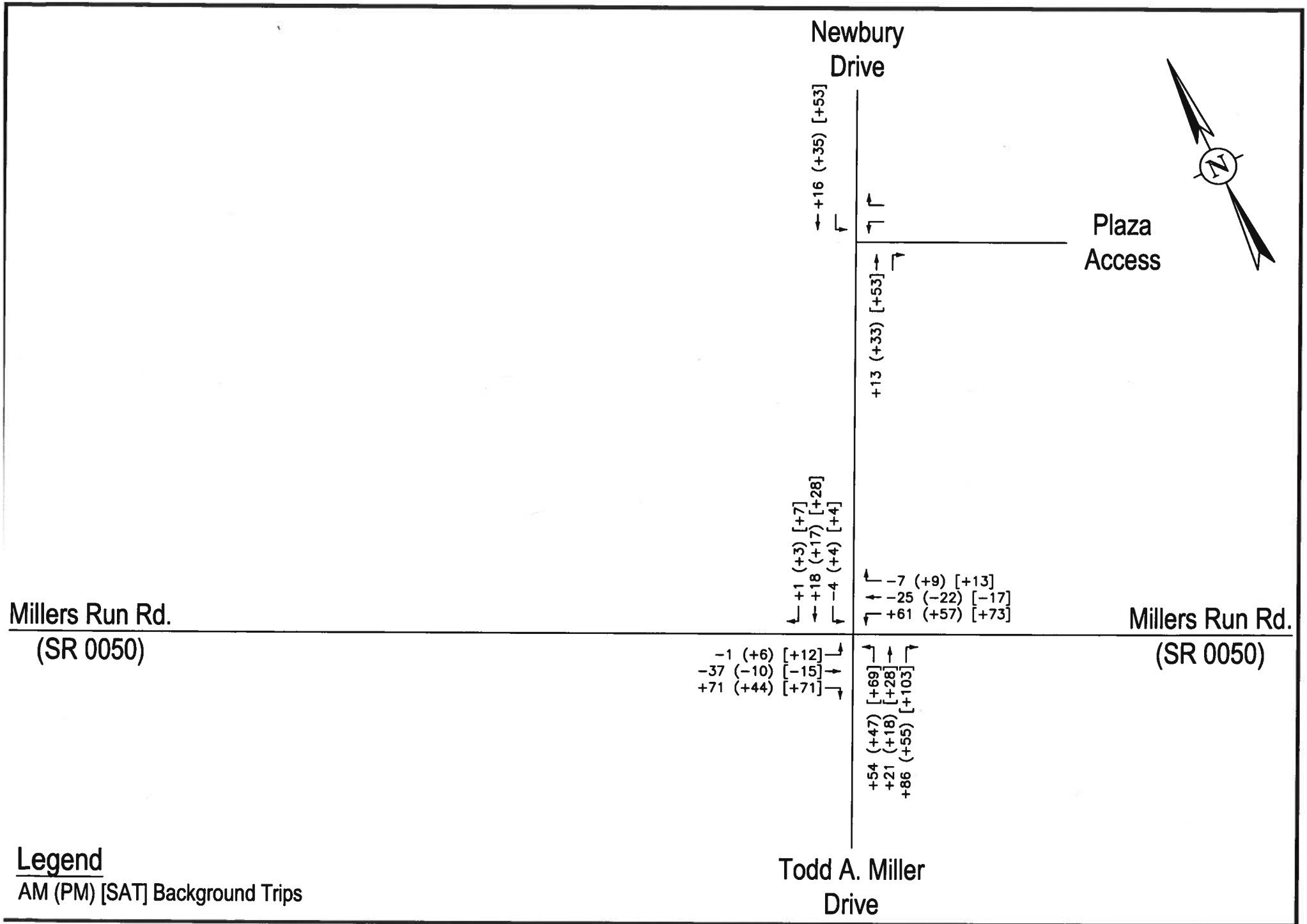
Legend

AM (PM) [SAT] Primary Trips

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trips (Cigar Lounge, Bar & Restaurant Development)

DAVID E. WOOSTER & ASSOC., LLC

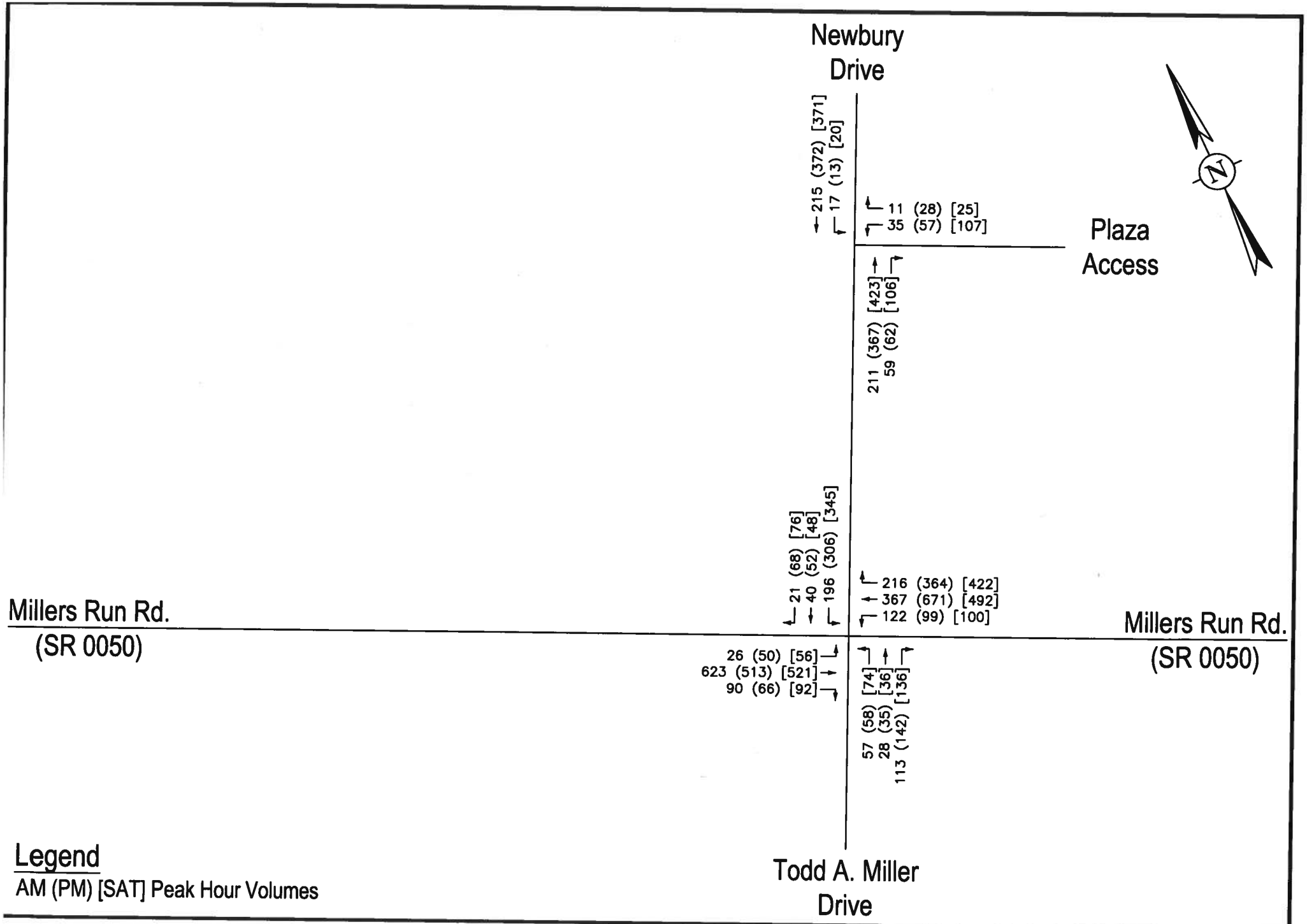
FIGURE 9b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Total Background Trips

DAVID E. WOOSTER & ASSOC., LLC

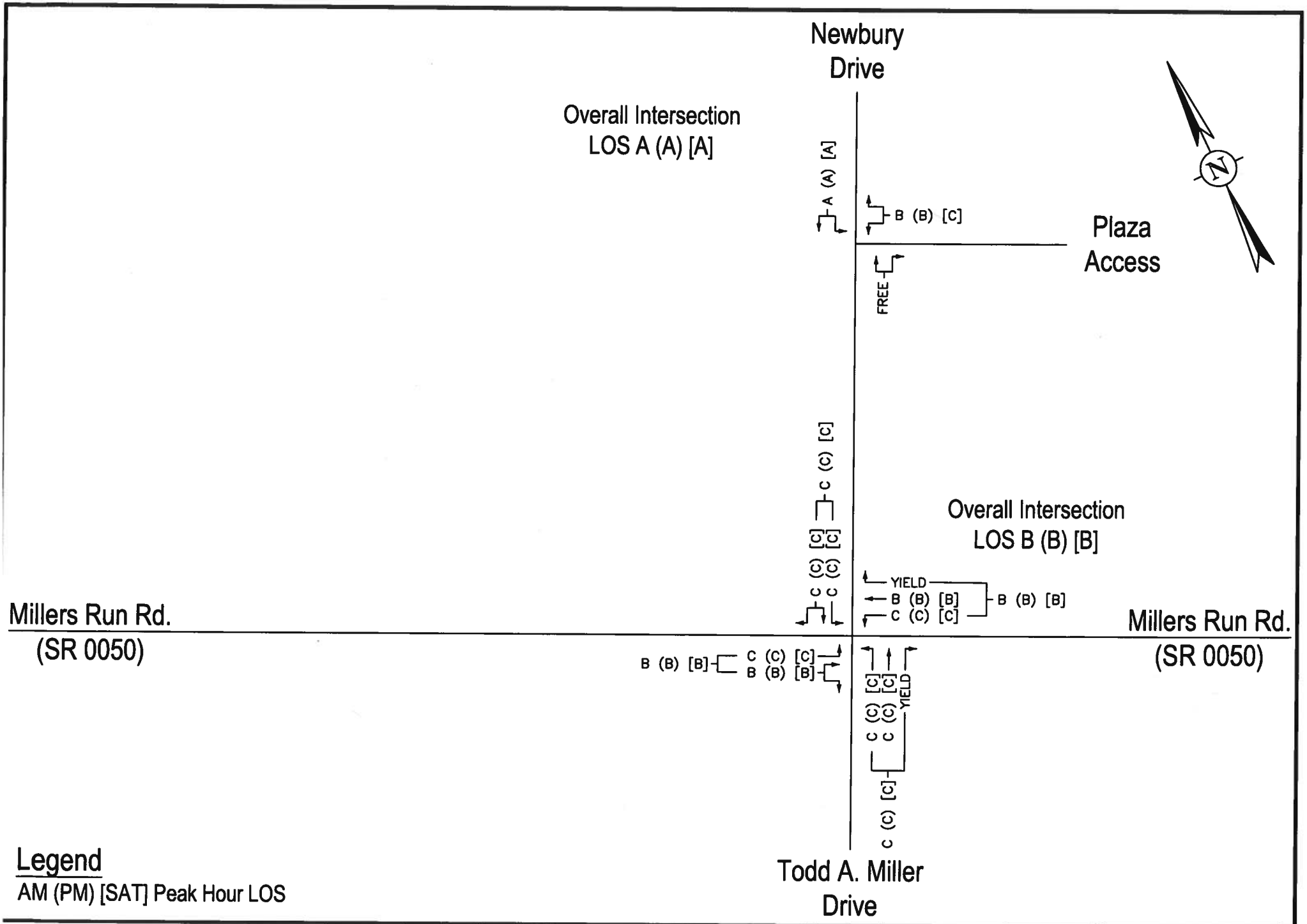
FIGURE 10
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 Without Development Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

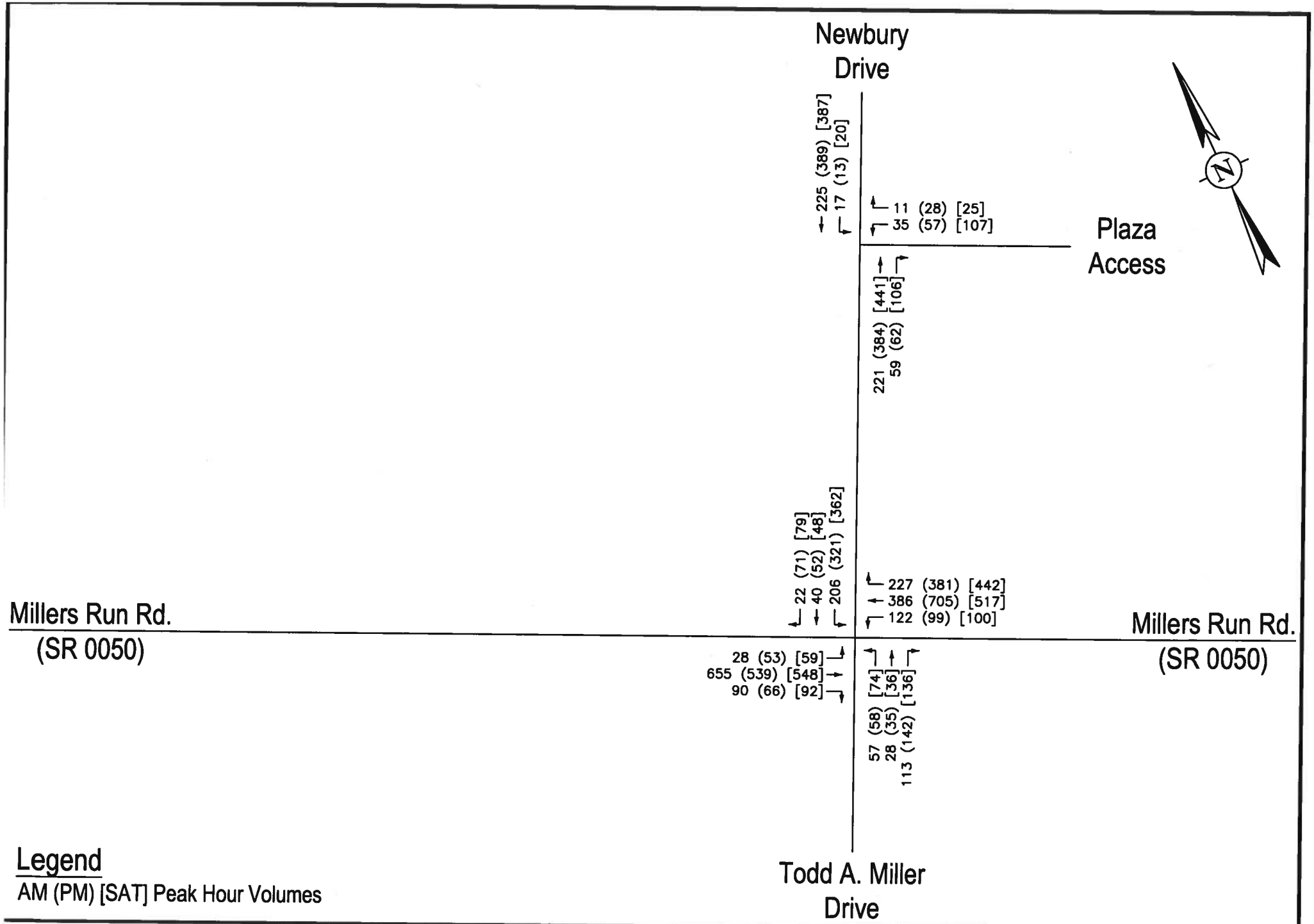
FIGURE 11a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 Without Development Condition Peak Hour LOS

DAVID E. WOOSTER & ASSOC., LLC

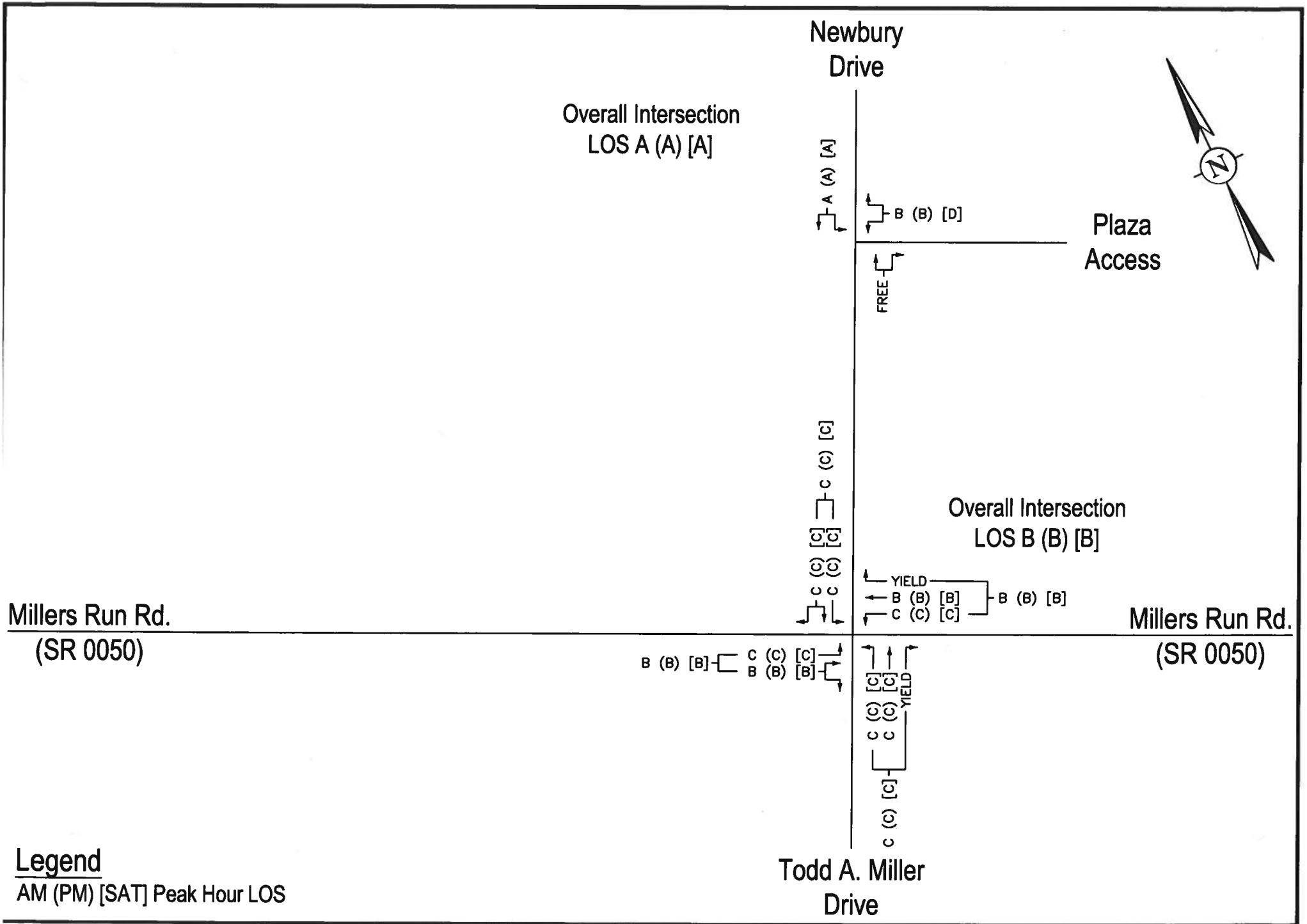
FIGURE 11b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 Without Development Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

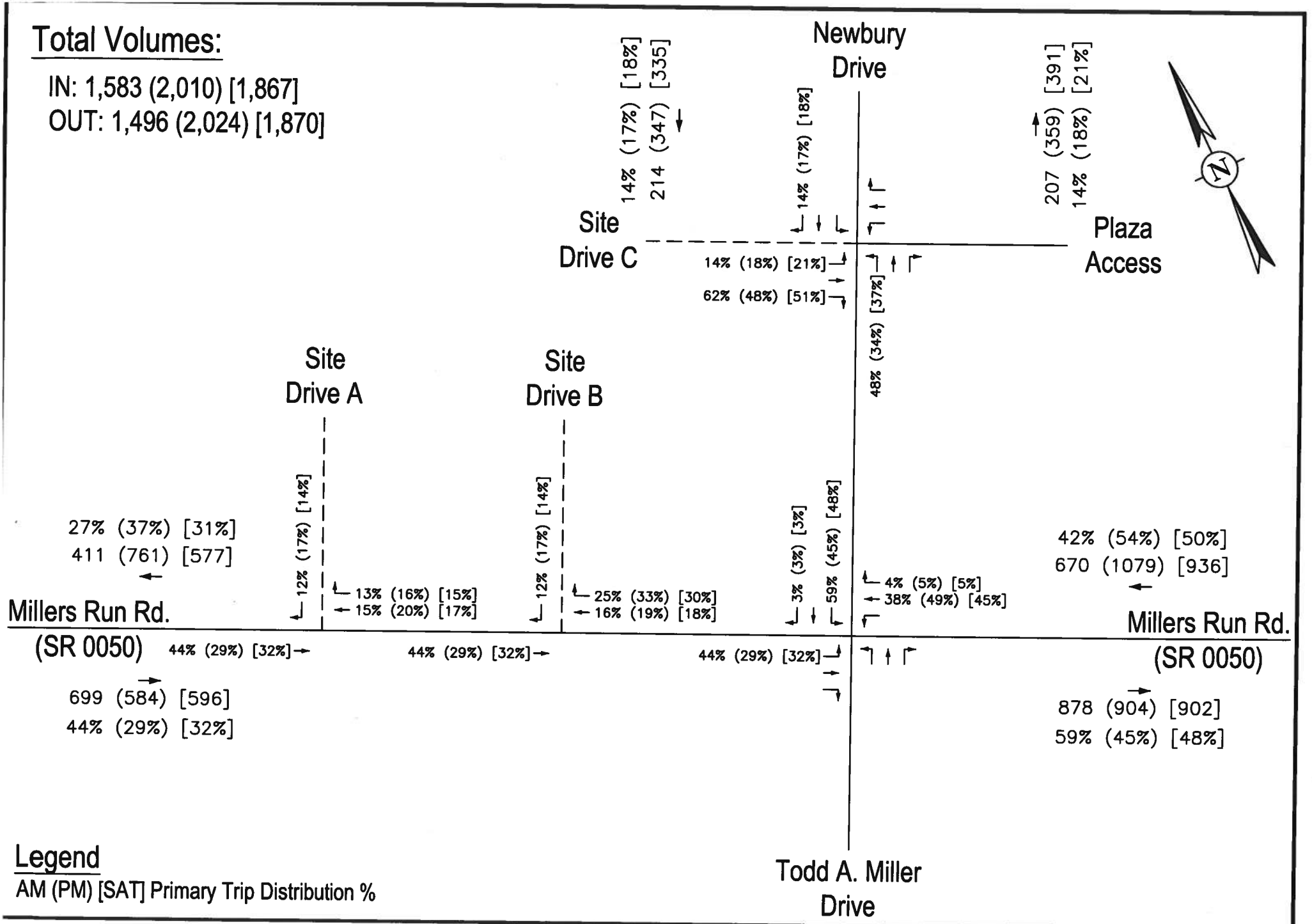
FIGURE 12a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 Without Development Condition Peak Hour LOS

DAVID E. WOOSTER & ASSOC., LLC

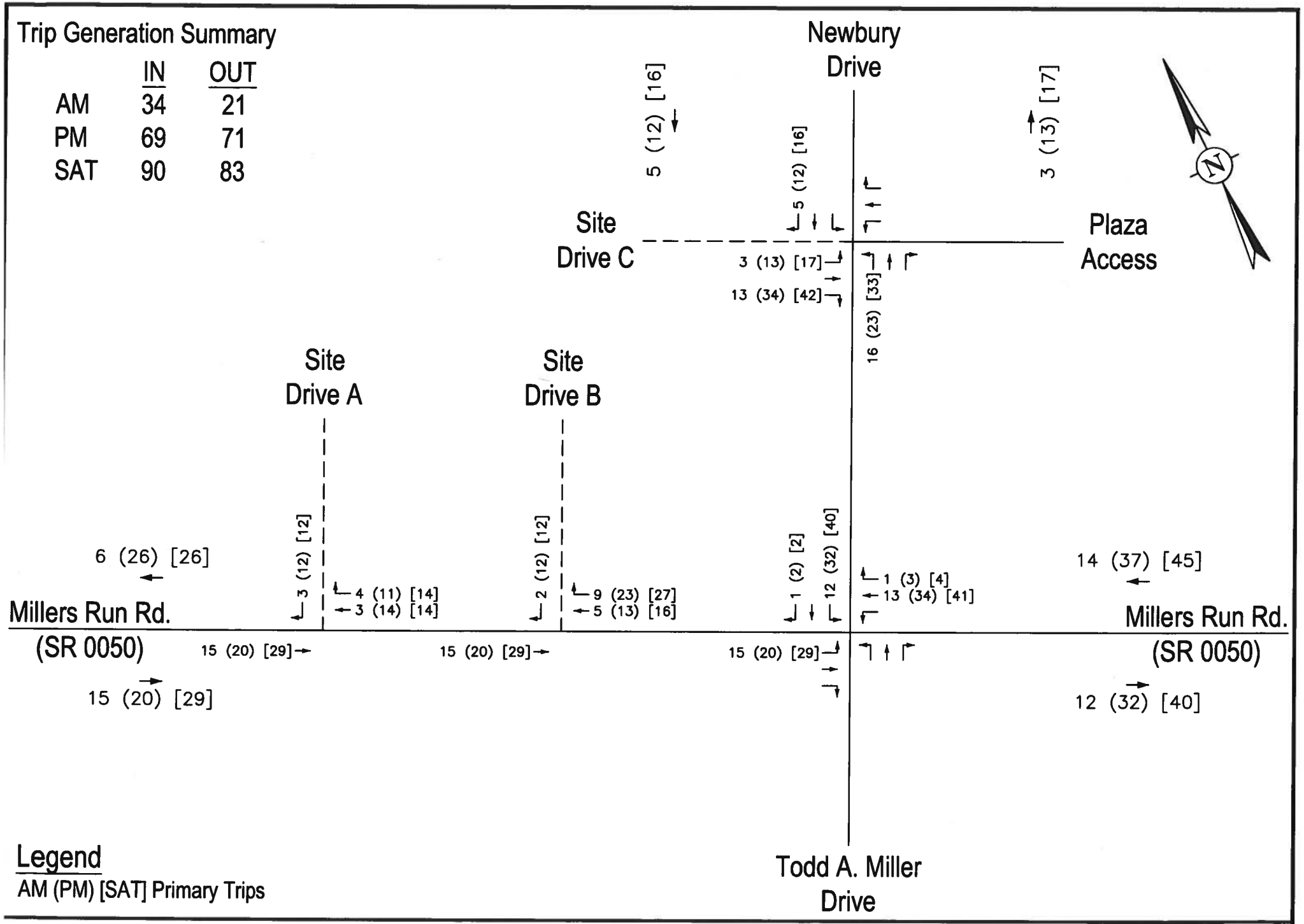
FIGURE 12b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trip Distribution Percentages

DAVID E. WOOSTER & ASSOC., LLC

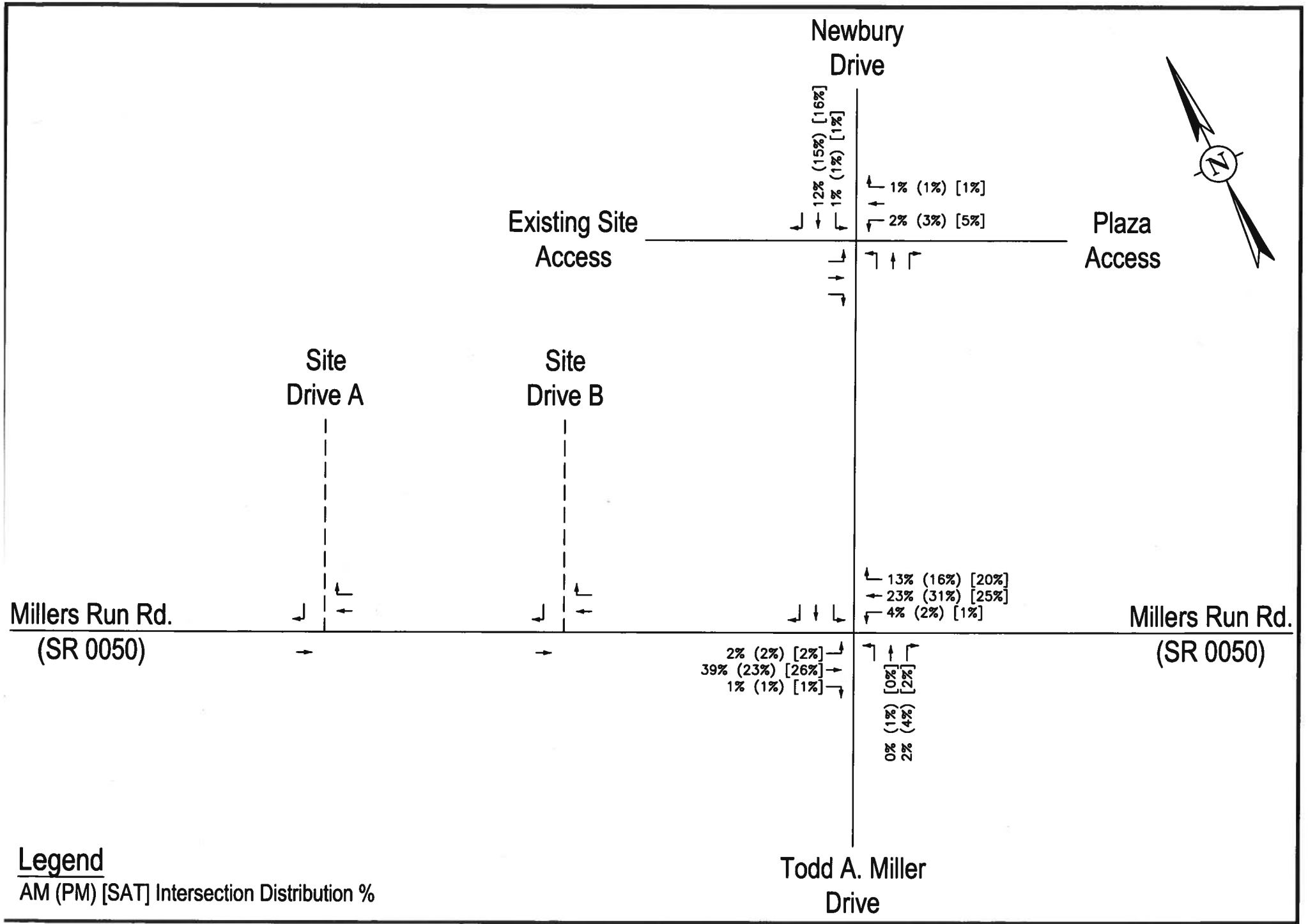
FIGURE 13a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Site-Generated Primary Trips

DAVID E. WOOSTER & ASSOC., LLC

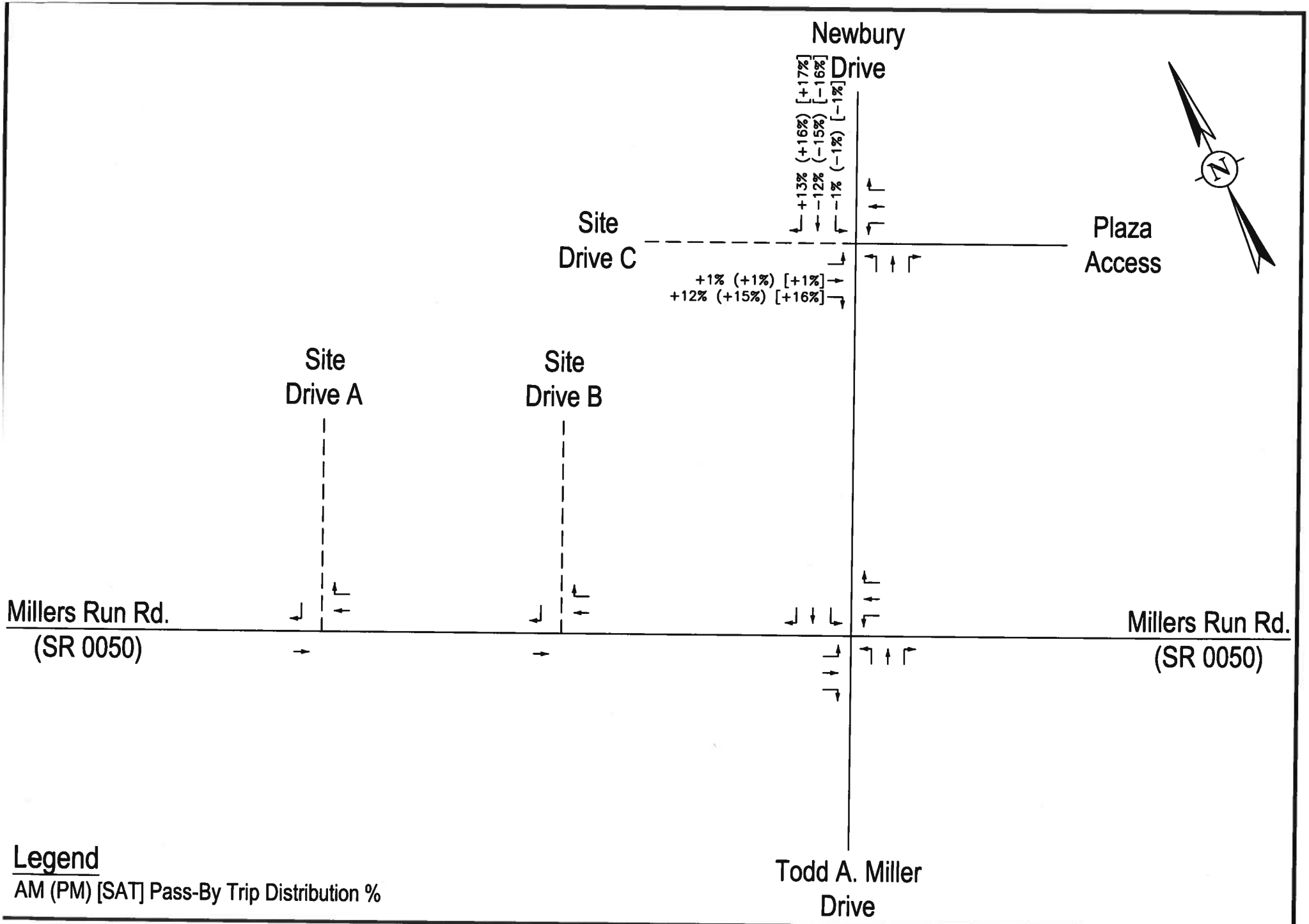
FIGURE 13b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Intersection Distribution Percentages

DAVID E. WOOSTER & ASSOC., LLC

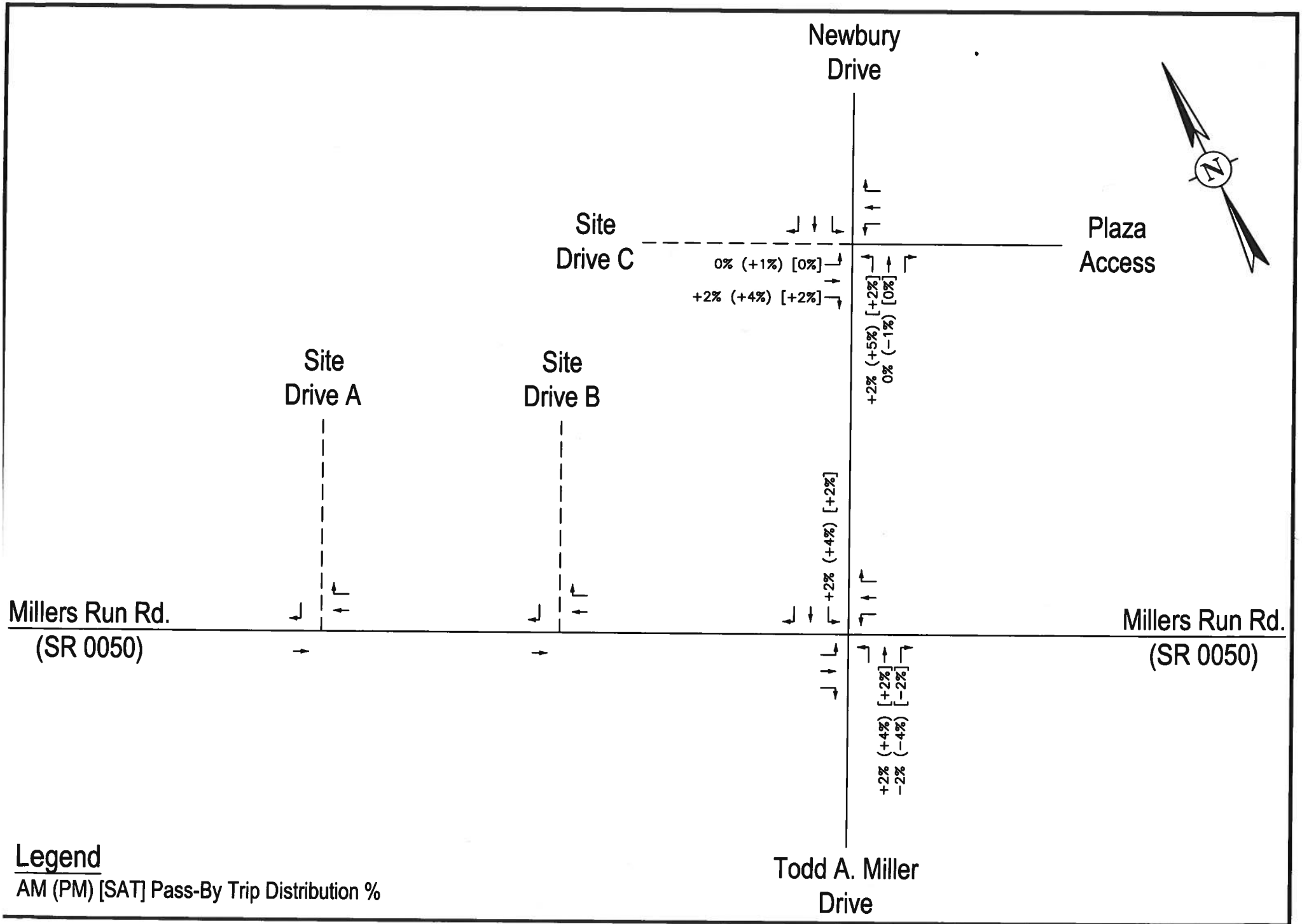
FIGURE 14
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trip Distribution Percentages (From North)

DAVID E. WOOSTER & ASSOC., LLC

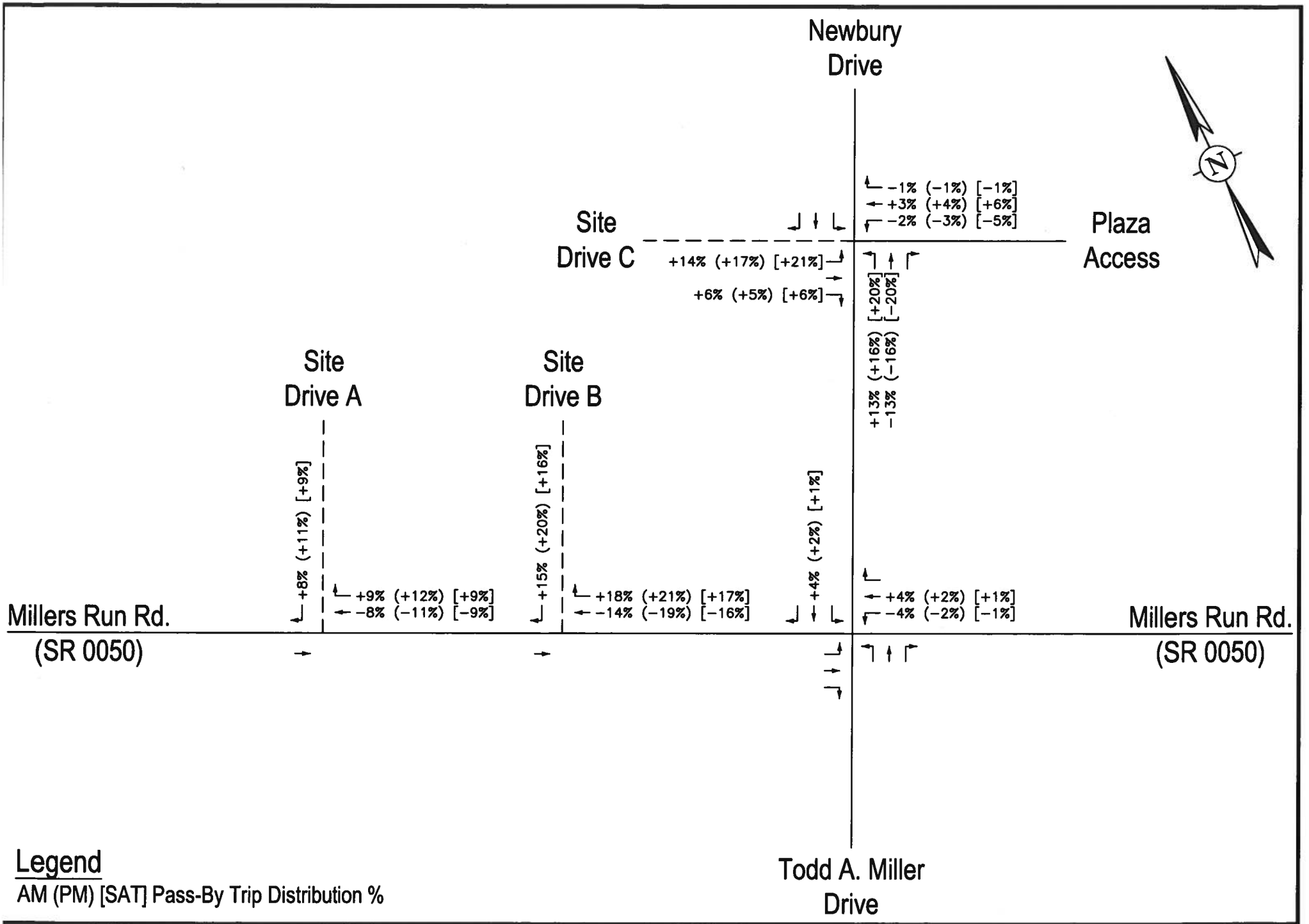
FIGURE 15a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass–By Trip Distribution Percentages (From South)

DAVID E. WOOSTER & ASSOC., LLC

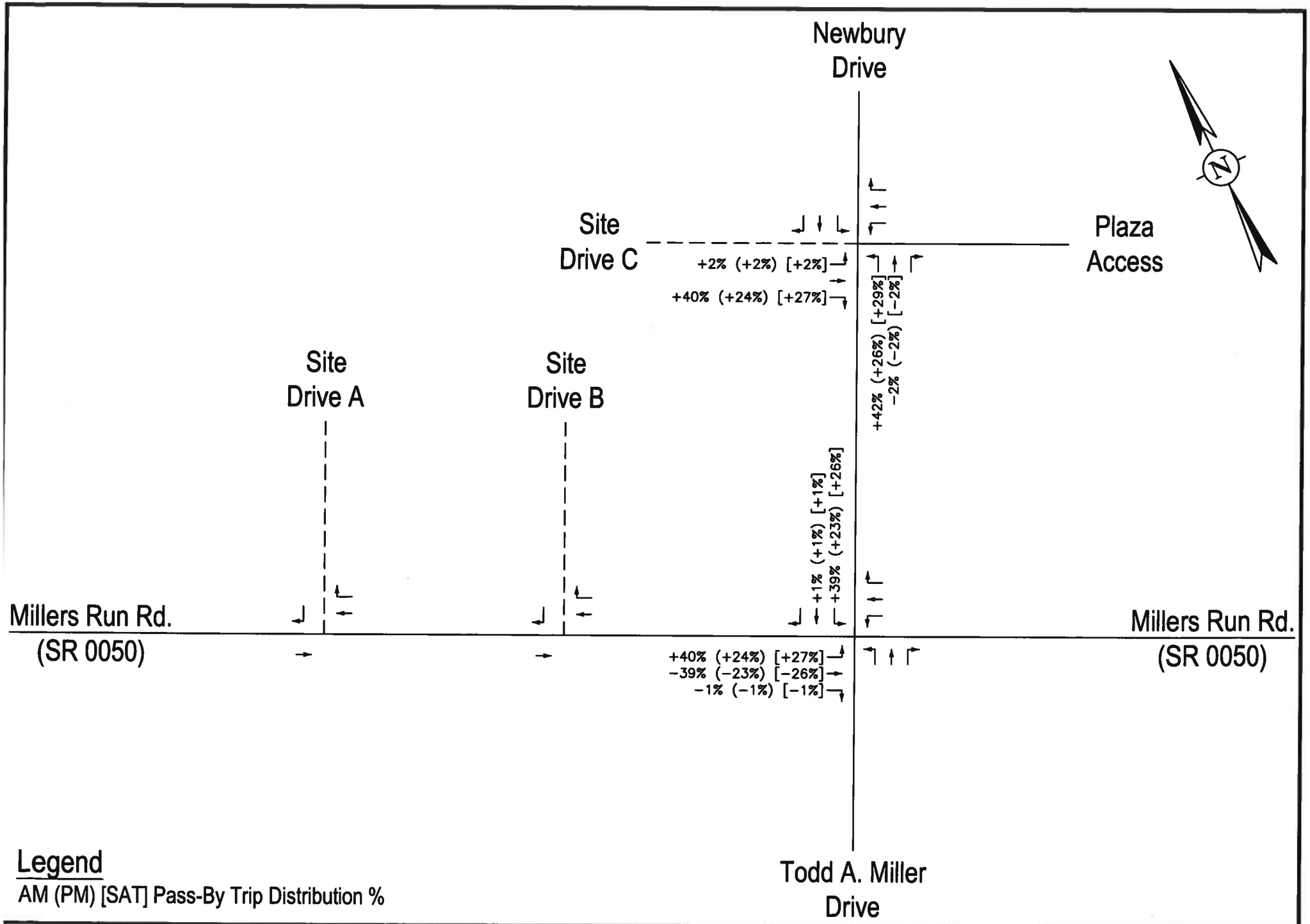
FIGURE 15b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trip Distribution Percentages (From East)

DAVID E. WOOSTER & ASSOC., LLC

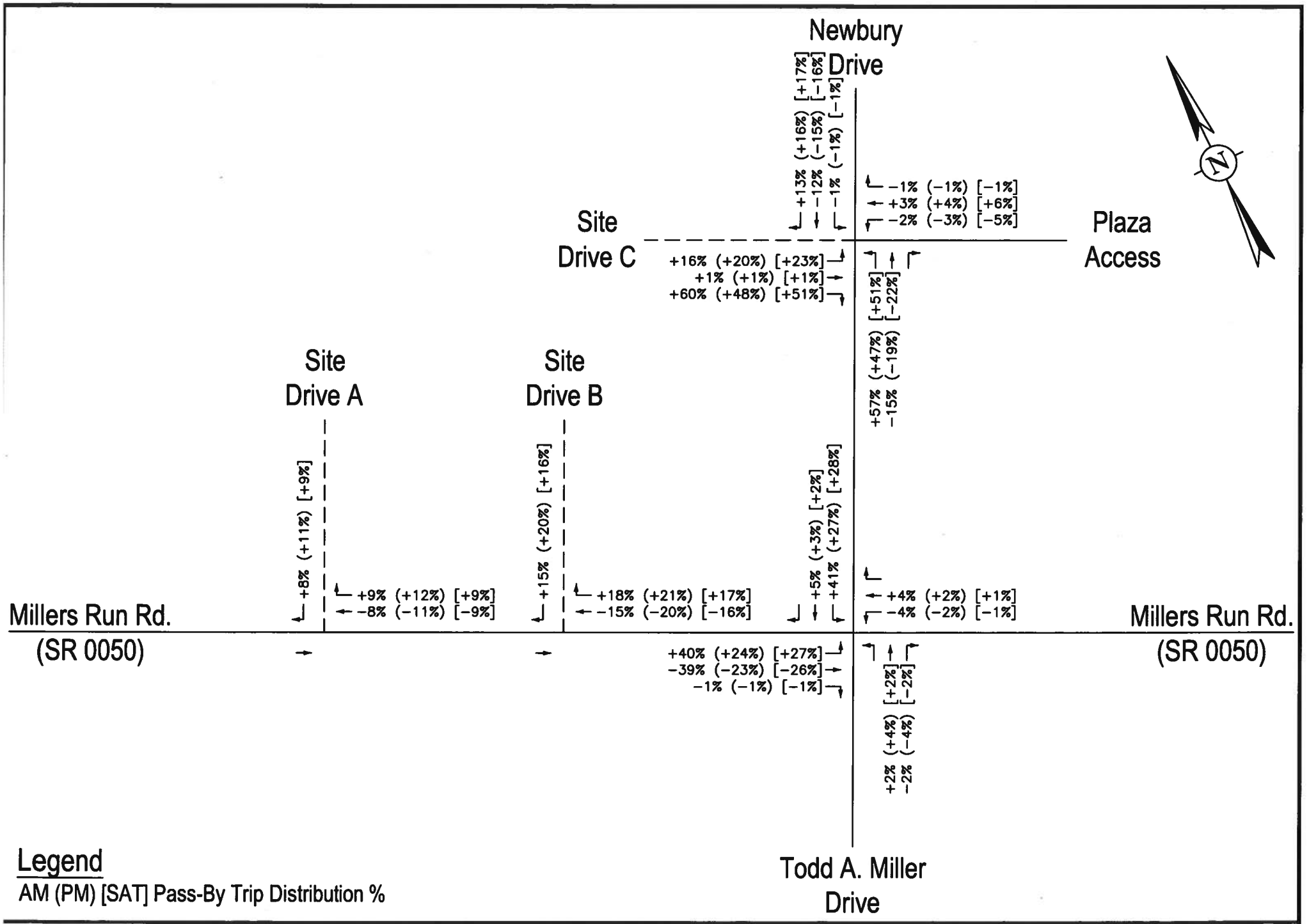
FIGURE 15c
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trip Distribution Percentages (From West)

DAVID E. WOOSTER & ASSOC., LLC

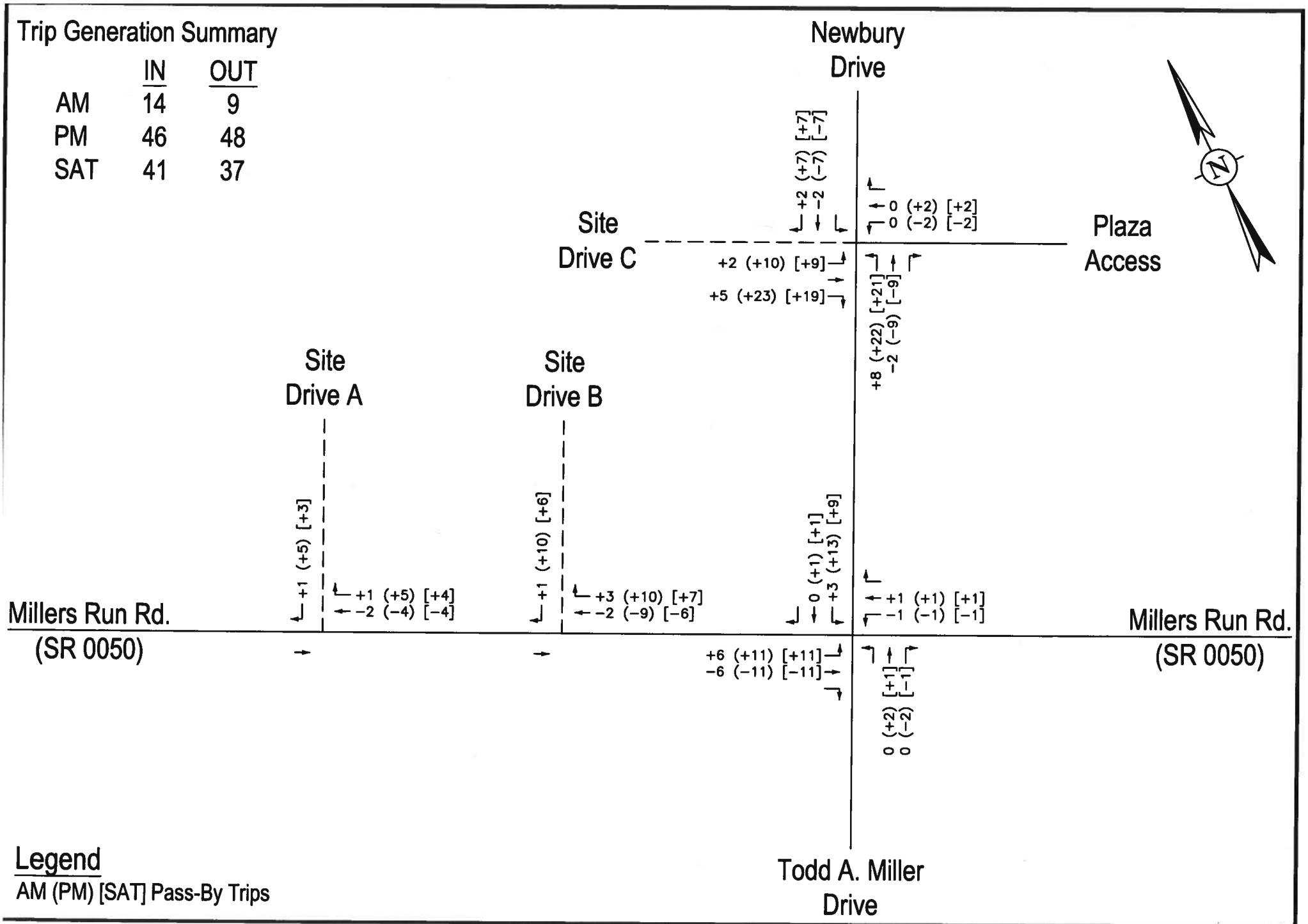
FIGURE 15d
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass–By Trip Distribution Percentages (Total)

DAVID E. WOOSTER & ASSOC., LLC

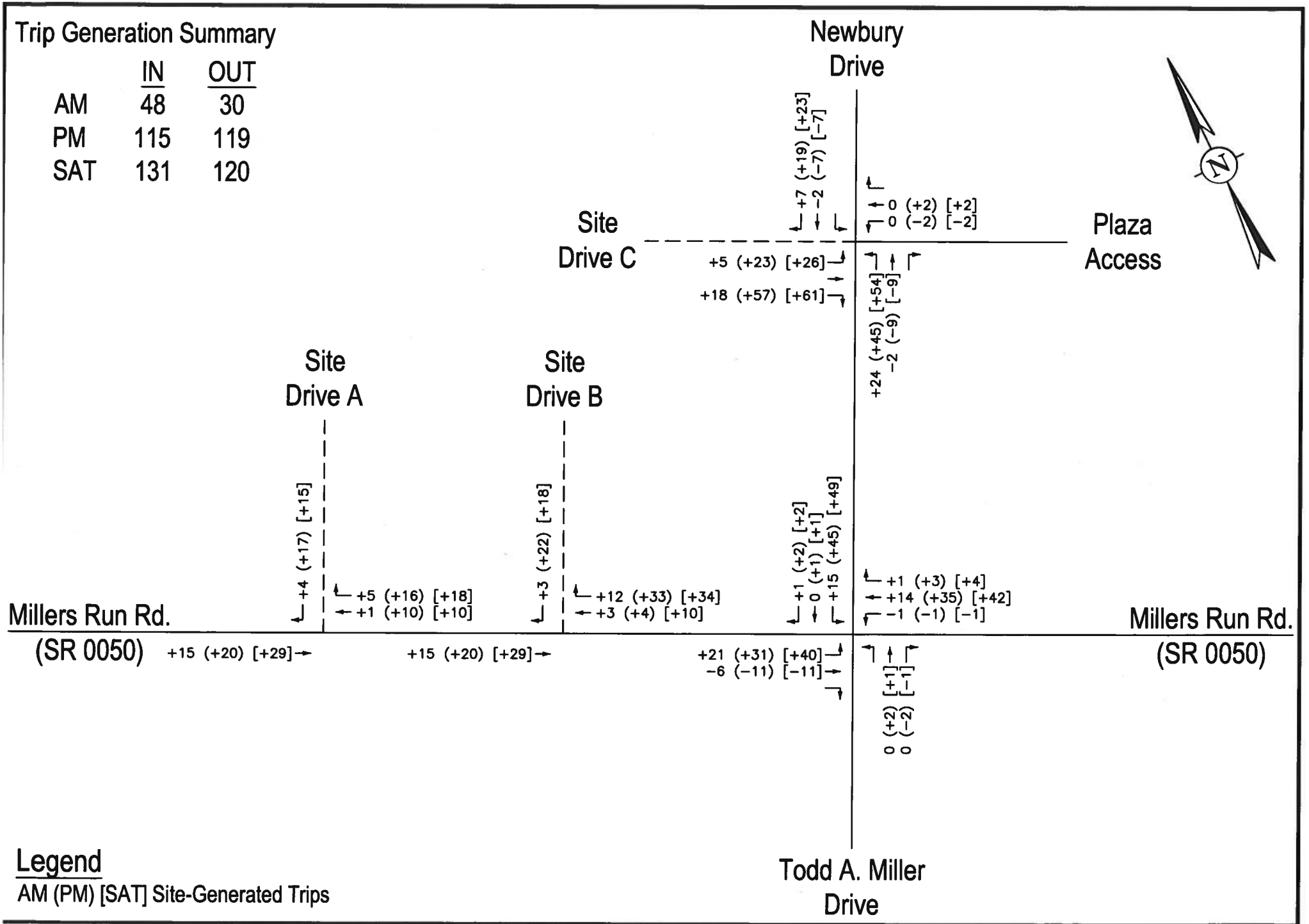
FIGURE 16a
4392–092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Site-Generated Pass-By Trips

DAVID E. WOOSTER & ASSOC., LLC

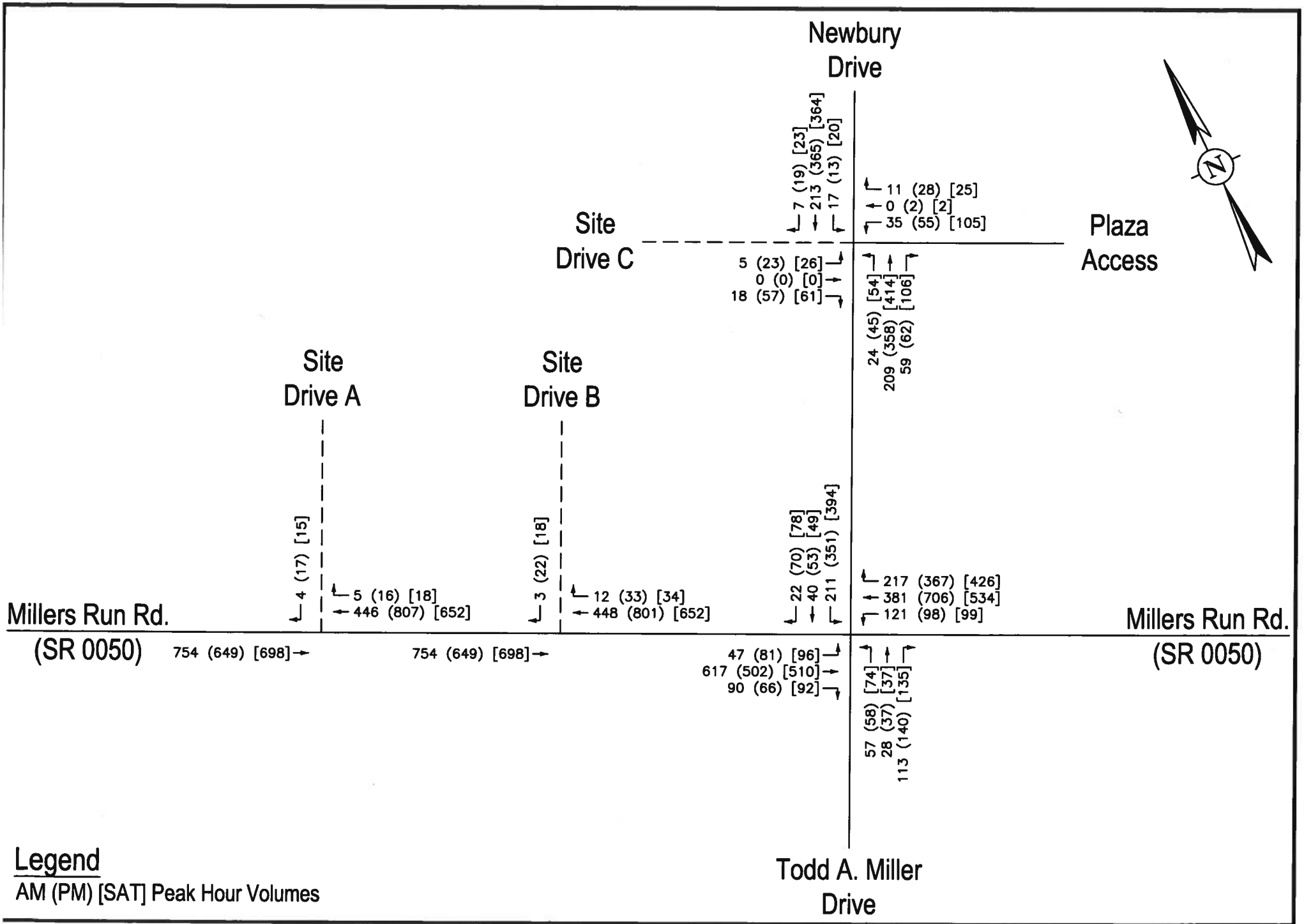
FIGURE 16b
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Total Site-Generated Trips

FIGURE 17
4392-092723

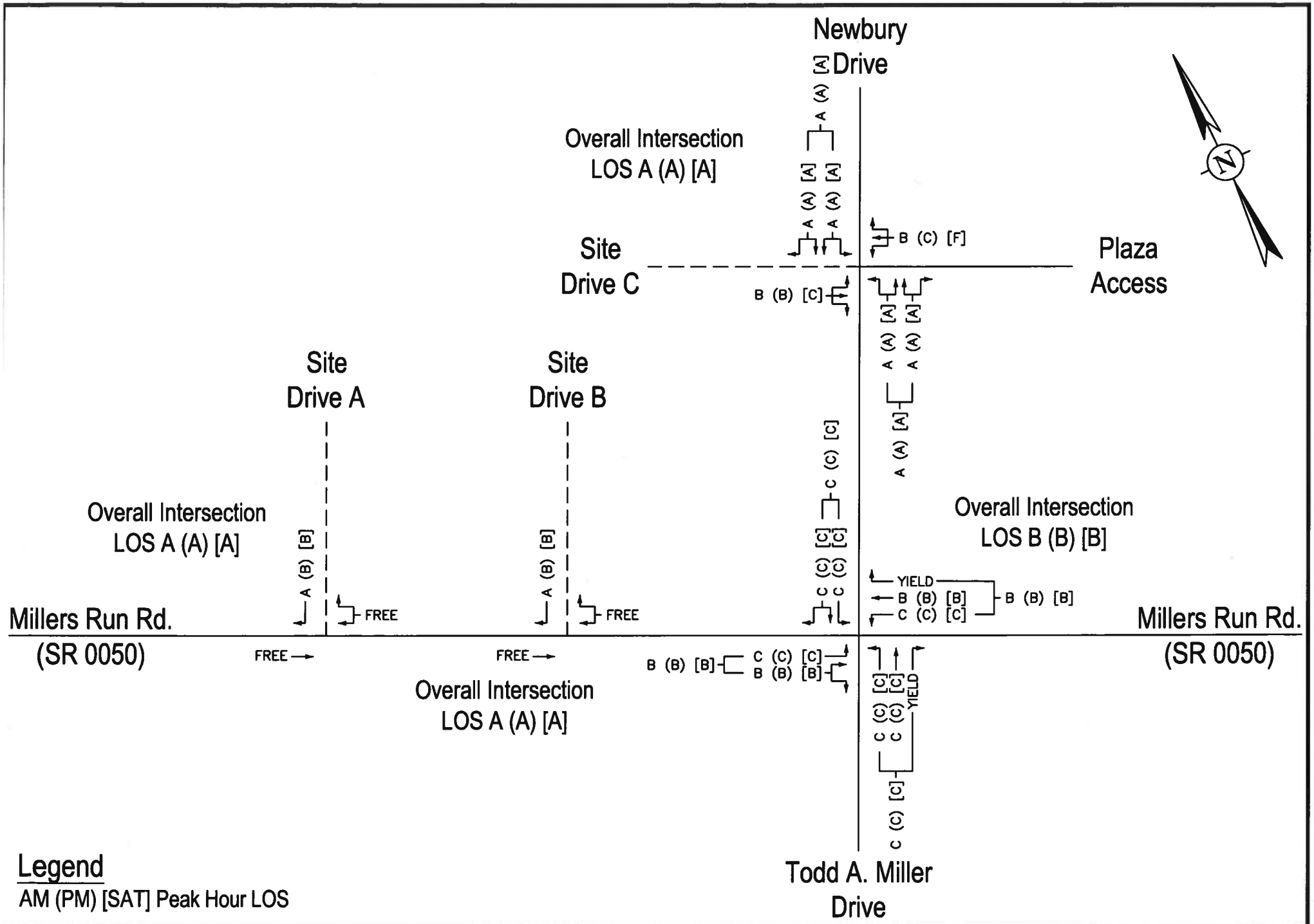
DAVID E. WOOSTER & ASSOC., LLC



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 With Development Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

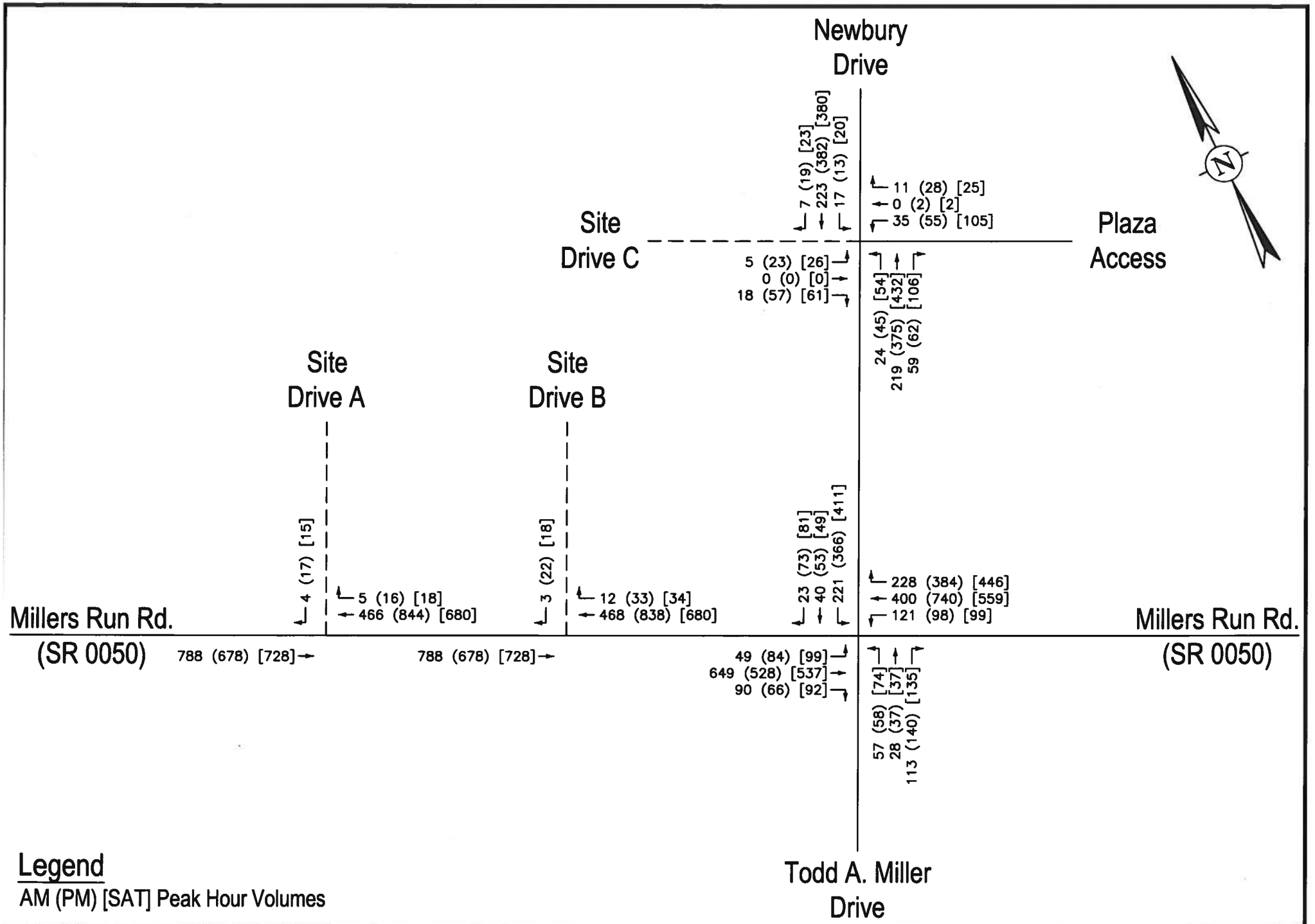
FIGURE 18a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 With Development Condition Peak Hour LOS

FIGURE 18b
4392-092723

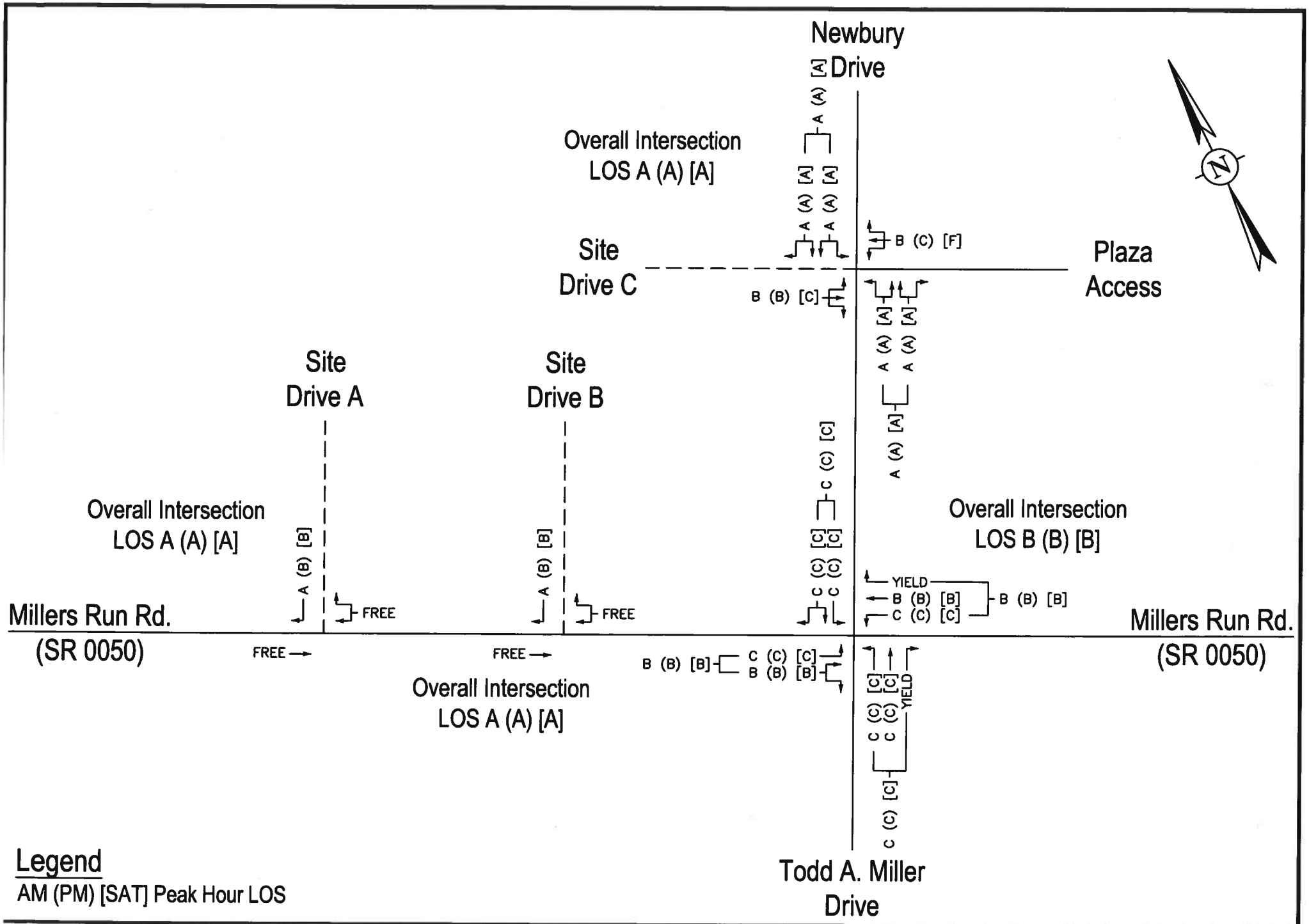
DAVID E. WOOSTER & ASSOC., LLC



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 With Development Condition Peak Hour Traffic Volumes

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 19a
4392-092723



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 With Development Condition Peak Hour LOS

DAVID E. WOOSTER & ASSOC., LLC

FIGURE 19b
4392-092723

APPENDICES

APPENDIX A

TIS Scoping Checklist

TRANSPORTATION IMPACT STUDY (TIS)
SCOPING MEETING CHECKLIST

Scoping Meeting Date: **Thursday, September 14, 2023 – 10:00 AM – via Microsoft Teams**

Applicant: **Cozza Enterprises, LLC**

Applicants Consultant: **David E. Wooster and Associates, LLC** Phone: **412-921-3303**

Applicant's Primary Contact: **Craig Cozza** Phone: **412-417-9700**

(1) LOCATION OF PROPOSED DEVELOPMENT:

***Site location map is attached to this checklist.**

PennDOT Engineering Dist.: **11-0** County: **Allegheny**

Municipality: **South Fayette Township**

State Route(s): **SR 0050**

From Segment / Offset: **0090 / 1000**

To Segment / Offset: **0090 / 1359**

(2) DESCRIPTION OF PROPOSED DEVELOPMENT:

***Preliminary site plan attached to this checklist.**

Proposed site access: **Two (2) right-in right-out only (RIRO) site access proposed along the northern side of Millers Run Road (SR 0050); full access site drive along the western side of Newbury Drive.**

Proposed land uses: **Retail**

Community linkages: **Sidewalk requirements per South Fayette Township ordinances (if any); no additional bus stops proposed; no new cross easements proposed; pedestrian accommodations (if any) in the study area to be maintained.**

(3) DEVELOPMENT SCHEDULE AND STAGING:

Anticipated Opening Date: **2024**

Full Buildout Date: -

Describe Proposed Development Schedule/Staging:

None.

(4) TRIP GENERATION:

Trip generation for the proposed development will be based on:

X ITE Trip Generation Manual

- **LU Code #821 (Shopping Plaza 40-150k *without* Supermarket) with gross floor area as the independent variable.**

___ Other independent surveys

List land development and trip generation information, as appropriate. If necessary, attach additional sheets to indicate additional land uses or development phases.

Land Use(s)	Size	ADT (In / Out)	Peak Hour Trips (In / Out)		
			AM Peak	PM Peak	SAT Peak
#821	~ 45,126 SF	3,048 (1,524 / 1,524)	78 (48 / 30)	234 (115 / 119)	251 (131 / 120)
		Primary	*55 (34 / 21)*	*140 (69 / 71)*	*173 (90 / 83)*
		Pass-By	*23 (14 / 9)*	*94 (46 / 48)*	*78 (41 / 37)*
	Totals	**3,048 (1,524 / 1,524)**	**78 (48 / 30)**	**234 (115 / 119)**	**251 (131 / 120)**

***The 11th Edition of the Trip Generation Manual was used.**

(5) ESTIMATED DAILY TRIP GENERATION/DRIVEWAY CLASSIFICATION:

(a) Estimated Daily Trip Generation of Proposed Development – Assuming One Access Point and Full Buildout/Occupancy of Entire Tract: **3,048** trips/day

(b) Driveway Classification Based on Trip Generation and One Access Point:

Minimum Use: _____ Medium Volume: _____

Low Volume: _____ High Volume: X

(6) TRAFFIC IMPACT STUDY REQUIRED?

_____ No

X Yes, based on:
- X 3,000 or more vehicle trips/day generated
- X During any one-hour time period, 100 or more new (added) vehicle trips generated entering or 100 or more new (added) vehicle trips generated exiting development
- _____ Other considerations described below:

(7) TRAFFIC IMPACT ASSESSMENT REQUIRED?

X No

_____ Yes

If a TIS or TIA is required, the following sections of this checklist will be discussed at the TIS Scoping Meeting. The applicant may provide preliminary information.

(8) TIS STUDY AREA:

(a) Roadway and Study Intersections

- **Millers Run Road (SR 0050) with Newbury Drive/Todd A. Miller Drive – *Existing Signalized***
- **Proposed Site Drives**

(b) Land Use Context

Suburban Corridor

(c) Known Congestion Areas

- **Millers Run Road (SR 0050) with Newbury Drive/Todd A. Miller Drive**
- **Proposed Cane's Restaurant (Concern by Township/PennDOT)**

(d) Known Safety Concerns

None.

(e) Known Environmental Constraints

None.

(f) Pedestrian / Bike Review

Not Applicable.

(g) Transit Review

Not Applicable.

(9) STUDY AREA TYPE Urban **X** Rural _____

(10) TIS ANALYSIS PERIODS AND TIMES:

Existing Year 2023 Condition
Opening Year 2024 Without and With Development
Design Year 2029 Without and With Development

(11) TRAFFIC ADJUSTMENT FACTORS:

(a) Seasonal Adjustment:

No Seasonal Adjustment proposed. Counts will be performed during a typical weekday.

Source

(b) Annual Base Traffic Growth: **1.00%** %/yr (linear) **SPC – 4/25/2023**

(c) Pass-By Trips:

Land Use	%	Source – *ITETripGen Web-based App*
#821	**30% AM** **40% PM** **31% SAT**	**PM – 10%** **2021 Pass-By Rates (PM)** **2021 Pass-By Rates (SAT)**

(d) Captured Trips for Multi-Use Sites:

Not Applicable.

(e) Modal Split Reductions:

Not Applicable.

(f) Other Reductions:

Not Applicable.

(12) OTHER PROJECTS WITHIN STUDY AREA TO BE ADDED TO BASE TRAFFIC:

- **The Piazza Development**
  - **6,800 SF High-Turnover (Sit-Down) Restaurant**
  - **4,000 SF Fast Food Restaurant with Drive-Through Window**
  - **4,250 SF Fast Food Restaurant with Drive-Through Window (Cane's)**
- **South Fayette Commons Development**
- **Cigar, Bar, & Restaurant (Newbury)**

(13) TRIP DISTRIBUTION AND ASSIGNMENT:

Distribution of the development trips will be based on the turning movement count data at the existing study intersection, as well as engineering judgment relative to the convenience of accessing the site from various directions.

(14) APPROVAL OF DATA COLLECTION ELEMENTS AND METHODOLOGIES:

Location	Period	Type
Existing intersection(s) listed in Section (8a) above	**7:00-9:00 am (T-Th) 4:00-6:00 pm (T-Th) 11:00 am-2:00 pm (Sat)**	**Turning Movement Counts**

(15) CAPACITY/LOS ANALYSES:

Location	Period	Type
Intersections listed in Section (8a)	**AM, PM, & SAT Peak Hours**	**HCM 6th Ed. Synchro 11**

(16) ROADWAY IMPROVEMENTS/MODIFICATIONS BY OTHERS TO BE INCLUDED:

None.

(17) OTHER NEEDED ANALYSES:

(a) Sight Distance Analyses:

Yes – at all proposed site accesses.

(b) Signal Warrant Analysis:

If/as necessary.

(c) Required Signal Phasing/Timing Modifications:

If/as necessary.

(d) Traffic Signal Corridor/Network Analyses:

If/as necessary.

(e) Analyses of the Need for Turning Lanes:

Wooster will compare forecasted traffic volumes at the proposed site access with criteria outlined in *Publication 46, Chapter 11 Traffic Studies*, dated 2012 for the consideration of auxiliary turn lanes under future With Development conditions.

(f) Turning Lane Lengths:

Length of any required turn lanes will be based on criteria outlined in *Publication 46, Chapter 11 Traffic Studies*, dated 2012, which includes SimTraffic (queuing) analyses. Existing turn lane lengths will be evaluated to ensure that they are equipped to adequately accommodate projected traffic volumes.

(g) Left Turn Signal Phasing Analyses:

If/as necessary.

(h) Queuing Analyses:

Yes – queue analyses will be performed using SimTraffic to determine if existing turn lanes are of sufficient length to accommodate the projected traffic. To perform these analyses, five (5) separate 60-minute simulations with a 10-minute seeding interval will be evaluated for each peak hour and averaged.

(i) Gap Studies:

Not Applicable.

(j) Crash Analyses:

Yes – reportable crash data will be obtained from the Pennsylvania Department of Transportation (PennDOT) Pennsylvania Crash Information Tool (PCIT) for the study area.

(k) Weaving Analyses:

Not Applicable.

(l) Other Required Studies:

None.

(18) ADDITIONAL COMMENTS OR RECOMMENDATIONS RELATIVE TO THE SCOPE OF THE TIS:

None.

Joshua A. Haydo, P.E., PTOE

Signature of Applicant's Engineer
Wooster and Associates

Date: *10-11-2023*

Signature of District Traffic PennDOT Representative
PennDOT District 11-0

Date: _____

Signature of District Permit PennDOT Representative
PennDOT District 11-0

Date: _____

Signature of Municipal Representative
South Fayette Township

Date: _____

APPENDIX B

Turning Movement Counts

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (7-9 am)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Data

Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							Int. Total
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	
7:00 AM	0	4	179	3	0	0	186	0	3	73	30	10	0	116	0	0	0	0	0	0	0	0	32	0	0	1	0	33	335
7:15 AM	0	4	184	4	0	0	192	0	3	79	28	3	0	113	0	0	0	0	0	0	0	0	27	2	0	0	0	29	334
7:30 AM	1	5	160	3	1	0	170	0	5	75	26	3	0	109	0	0	0	1	2	0	3	0	26	3	1	1	0	31	313
7:45 AM	0	11	173	5	1	0	190	0	13	93	47	9	0	162	0	0	0	0	1	0	1	0	45	7	3	0	0	55	408
Hourly Total	1	24	696	15	2	0	738	0	24	320	131	25	0	500	0	0	0	1	3	0	4	0	130	12	4	2	0	148	1390
8:00 AM	0	7	159	5	0	0	171	0	25	92	33	9	0	159	0	1	2	0	4	0	7	0	41	8	2	1	0	52	389
8:15 AM	0	3	174	2	0	0	179	0	13	76	43	11	0	143	0	0	1	0	4	0	5	0	37	4	2	3	0	46	373
8:30 AM	0	8	161	7	0	0	176	0	11	117	51	11	0	190	0	1	3	0	8	0	12	0	65	3	2	3	0	73	451
8:45 AM	0	9	159	5	0	0	173	0	12	103	41	22	0	178	0	1	1	0	11	0	13	0	55	7	0	7	0	69	433
Hourly Total	0	27	653	19	0	0	699	0	61	388	168	53	0	670	0	3	7	0	27	0	37	0	198	22	6	14	0	240	1646
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	1	51	1349	34	2	0	1437	0	85	708	299	78	0	1170	0	3	7	1	30	0	41	0	328	34	10	16	0	388	3036
Approach %	0.1	3.5	93.9	2.4	0.1	-	-	0.0	7.3	60.5	25.6	6.7	-	-	0.0	7.3	17.1	2.4	73.2	-	-	0.0	84.5	8.8	2.6	4.1	-	-	-
Total %	0.0	1.7	44.4	1.1	0.1	-	47.3	0.0	2.8	23.3	9.8	2.6	-	38.5	0.0	0.1	0.2	0.0	1.0	-	1.4	0.0	10.8	1.1	0.3	0.5	-	12.8	-
Lights	1	47	1291	34	2	-	1375	0	83	660	284	78	-	1105	0	3	6	1	28	-	38	0	323	33	10	15	-	381	2899
% Lights	100.0	92.2	95.7	100.0	100.0	-	95.7	-	97.6	93.2	95.0	100.0	-	94.4	-	100.0	85.7	100.0	93.3	-	92.7	-	98.5	97.1	100.0	93.8	-	98.2	95.5
Other Vehicles	0	4	58	0	0	-	62	0	2	48	15	0	-	65	0	0	1	0	2	-	3	0	5	1	0	1	-	7	137
% Other Vehicles	0.0	7.8	4.3	0.0	0.0	-	4.3	-	2.4	6.8	5.0	0.0	-	5.6	-	0.0	14.3	0.0	6.7	-	7.3	-	1.5	2.9	0.0	6.3	-	1.8	4.5
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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2 East Crafton Ave.

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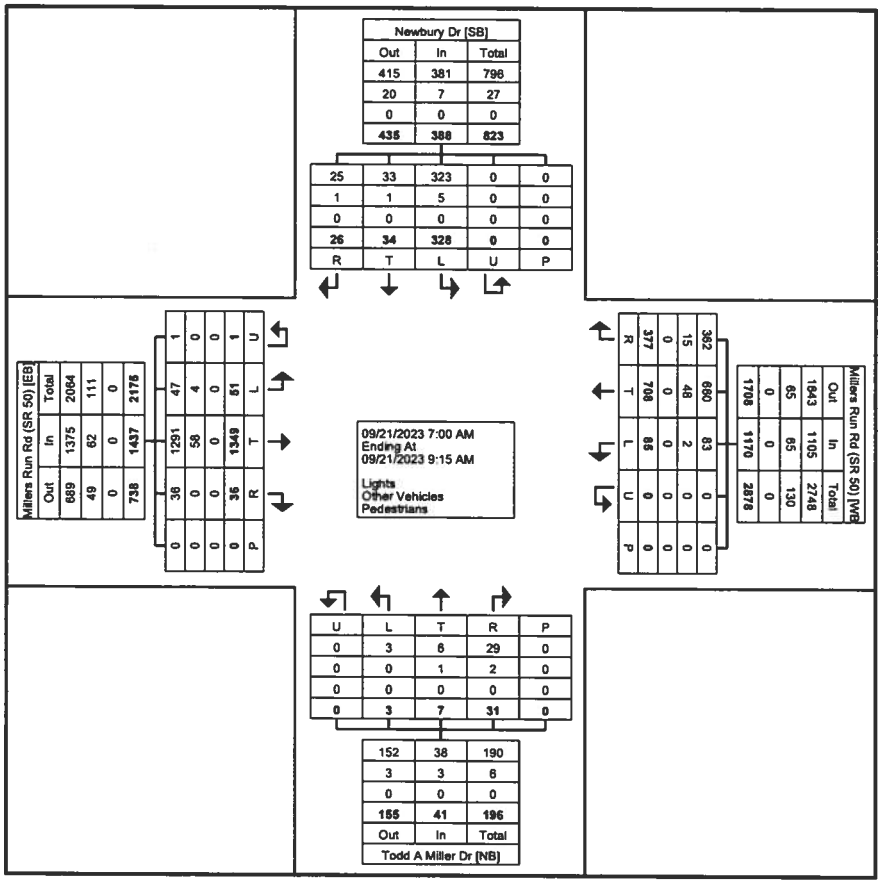
Count Name: SR 50 & Newbury Dr. (7-9 am)
Site Code: 4392
Start Date: 09/21/2023



Turning Movement Data Plot

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (7-9 am)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Peak Hour Data (8:00 AM)

Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							Int. Total
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	
8:00 AM	0	7	159	5	0	0	171	0	25	92	33	9	0	159	0	1	2	0	4	0	7	0	41	8	2	1	0	52	389
8:15 AM	0	3	174	2	0	0	179	0	13	76	43	11	0	143	0	0	1	0	4	0	5	0	37	4	2	3	0	46	373
8:30 AM	0	8	161	7	0	0	176	0	11	117	51	11	0	190	0	1	3	0	8	0	12	0	65	3	2	3	0	73	451
8:45 AM	0	9	159	5	0	0	173	0	12	103	41	22	0	178	0	1	1	0	11	0	13	0	55	7	0	7	0	69	433
Total	0	27	653	19	0	0	699	0	61	388	168	53	0	670	0	3	7	0	27	0	37	0	198	22	6	14	0	240	1646
Approach %	0.0	3.9	93.4	2.7	0.0	-	-	0.0	9.1	57.9	25.1	7.9	-	-	0.0	8.1	18.9	0.0	73.0	-	-	0.0	82.5	9.2	2.5	5.8	-	-	-
Total %	0.0	1.6	39.7	1.2	0.0	-	42.5	0.0	3.7	23.6	10.2	3.2	-	40.7	0.0	0.2	0.4	0.0	1.6	-	2.2	0.0	12.0	1.3	0.4	0.9	-	14.6	-
PHF	0.000	0.750	0.938	0.679	0.000	-	0.976	0.000	0.610	0.829	0.824	0.602	-	0.882	0.000	0.750	0.583	0.000	0.614	-	0.712	0.000	0.762	0.688	0.750	0.500	-	0.822	0.912
Lights	0	25	620	19	0	-	664	0	60	362	162	53	-	637	0	3	6	0	25	-	34	0	195	21	6	13	-	235	1570
% Lights	-	92.6	94.9	100.0	-	-	95.0	-	98.4	93.3	96.4	100.0	-	95.1	-	100.0	85.7	-	92.6	-	91.9	-	98.5	95.5	100.0	92.9	-	97.9	95.4
Other Vehicles	0	2	33	0	0	-	35	0	1	26	6	0	-	33	0	0	1	0	2	-	3	0	3	1	0	1	-	5	76
% Other Vehicles	-	7.4	5.1	0.0	-	-	5.0	-	1.6	6.7	3.6	0.0	-	4.9	-	0.0	14.3	-	7.4	-	8.1	-	1.5	4.5	0.0	7.1	-	2.1	4.6
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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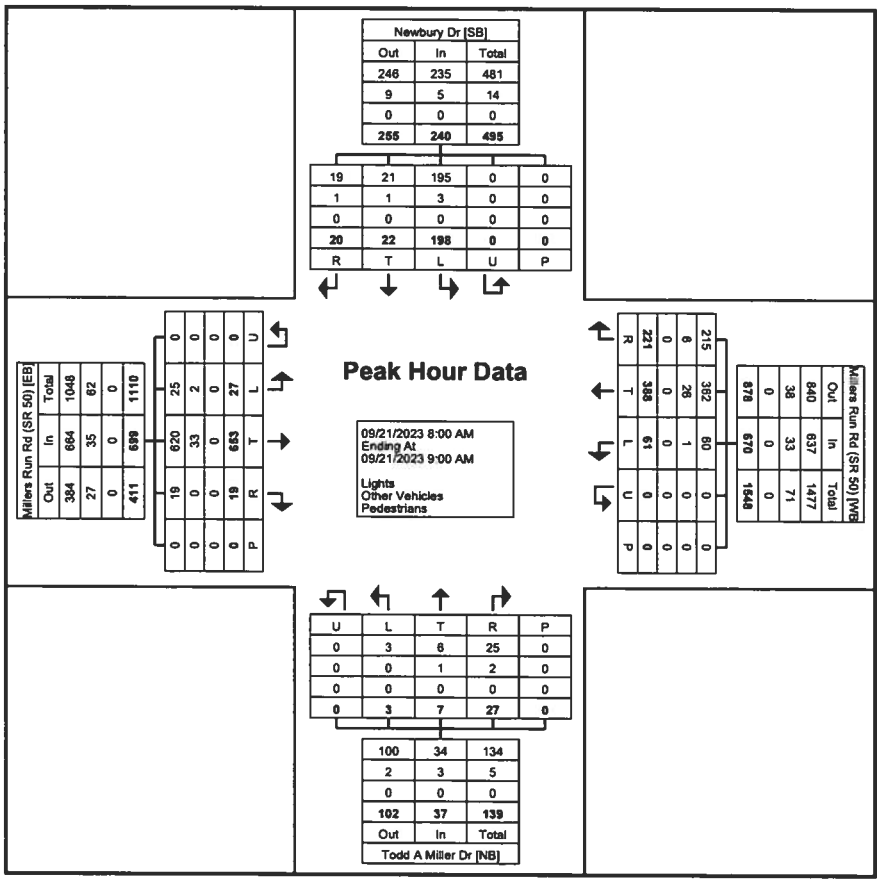

Turning Movement Peak Hour Data Plot (8:00 AM)

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Data

Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							Int. Total
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	
4:00 PM	0	10	133	4	0	0	147	0	7	184	60	20	0	271	0	3	1	6	22	0	32	0	82	13	8	8	0	111	561
4:15 PM	0	10	146	4	1	0	161	0	15	167	50	17	0	249	0	4	4	9	7	0	24	0	86	10	13	7	0	116	550
4:30 PM	0	13	121	4	1	0	139	1	8	156	68	34	0	267	0	4	6	8	15	0	33	0	64	4	6	11	0	85	524
4:45 PM	0	11	118	7	1	0	137	0	12	179	67	35	0	293	0	0	6	8	12	0	26	0	67	8	5	6	0	86	542
Hourly Total	0	44	518	19	3	0	584	1	42	686	245	106	0	1080	0	11	17	31	56	0	115	0	299	35	32	32	0	398	2177
5:00 PM	0	12	102	8	1	0	123	0	15	150	39	35	0	239	0	8	5	3	7	0	23	0	77	9	4	7	0	97	482
5:15 PM	0	7	110	7	1	0	125	0	17	176	57	15	0	265	0	3	3	9	6	0	21	0	54	11	10	2	0	77	488
5:30 PM	0	6	106	9	0	0	121	0	17	171	63	38	2	289	0	6	1	13	8	0	28	0	77	6	7	3	0	93	531
5:45 PM	0	6	96	7	0	0	109	0	11	141	64	23	2	239	0	4	3	3	7	0	17	0	77	5	1	9	0	92	457
Hourly Total	0	31	414	31	2	0	478	0	60	638	223	111	4	1032	0	21	12	28	28	0	89	0	285	31	22	21	0	359	1958
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	75	932	50	5	0	1062	1	102	1324	468	217	4	2112	0	32	29	59	84	0	204	0	584	66	54	53	0	757	4135
Approach %	0.0	7.1	87.8	4.7	0.5	-	-	0.0	4.8	62.7	22.2	10.3	-	-	0.0	15.7	14.2	28.9	41.2	-	-	0.0	77.1	8.7	7.1	7.0	-	-	-
Total %	0.0	1.8	22.5	1.2	0.1	-	25.7	0.0	2.5	32.0	11.3	5.2	-	51.1	0.0	0.8	0.7	1.4	2.0	-	4.9	0.0	14.1	1.6	1.3	1.3	-	18.3	-
Lights	0	75	916	50	5	-	1046	1	101	1291	463	215	-	2071	0	32	29	58	83	-	202	0	579	65	53	53	-	750	4069
% Lights	-	100.0	98.3	100.0	100.0	-	98.5	100.0	99.0	97.5	98.9	99.1	-	98.1	-	100.0	100.0	98.3	98.8	-	99.0	-	99.1	98.5	98.1	100.0	-	99.1	98.4
Other Vehicles	0	0	16	0	0	-	16	0	1	33	5	2	-	41	0	0	0	1	1	-	2	0	5	1	1	0	-	7	66
% Other Vehicles	-	0.0	1.7	0.0	0.0	-	1.5	0.0	1.0	2.5	1.1	0.9	-	1.9	-	0.0	0.0	1.7	1.2	-	1.0	-	0.9	1.5	1.9	0.0	-	0.9	1.6
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	4	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

David E. Wooster and Associates : Main Account
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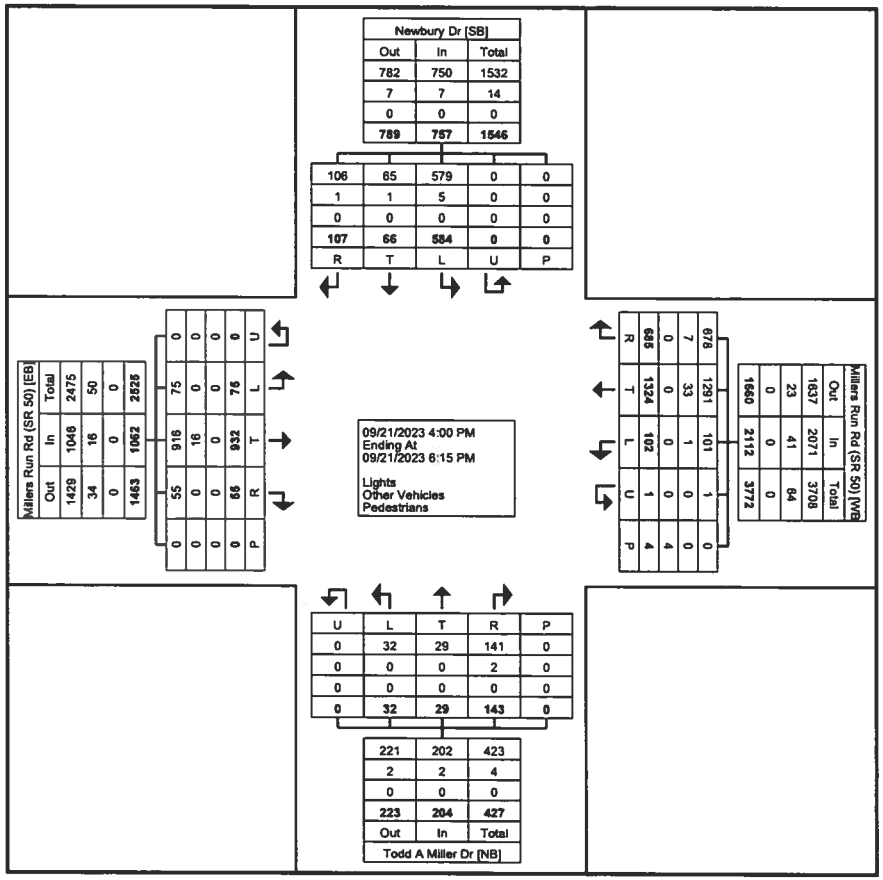
Count Name: SR 50 & Newbury Dr. (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023
Page No: 2



Turning Movement Data Plot

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Peak Hour Data (4:00 PM)

Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							Int. Total
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	
4:00 PM	0	10	133	4	0	0	147	0	7	184	60	20	0	271	0	3	1	6	22	0	32	0	82	13	8	8	0	111	561
4:15 PM	0	10	146	4	1	0	161	0	15	167	50	17	0	249	0	4	4	9	7	0	24	0	86	10	13	7	0	116	550
4:30 PM	0	13	121	4	1	0	139	1	8	156	68	34	0	267	0	4	6	8	15	0	33	0	64	4	6	11	0	85	524
4:45 PM	0	11	118	7	1	0	137	0	12	179	67	35	0	293	0	0	6	8	12	0	26	0	67	8	5	6	0	86	542
Total	0	44	518	19	3	0	584	1	42	686	245	106	0	1080	0	11	17	31	56	0	115	0	299	35	32	32	0	398	2177
Approach %	0.0	7.5	88.7	3.3	0.5	-	-	0.1	3.9	63.5	22.7	9.8	-	-	0.0	9.6	14.8	27.0	48.7	-	-	0.0	75.1	8.8	8.0	8.0	-	-	-
Total %	0.0	2.0	23.8	0.9	0.1	-	26.8	0.0	1.9	31.5	11.3	4.9	-	49.6	0.0	0.5	0.8	1.4	2.6	-	5.3	0.0	13.7	1.6	1.5	1.5	-	18.3	-
PHF	0.000	0.846	0.887	0.679	0.750	-	0.907	0.250	0.700	0.932	0.901	0.757	-	0.922	0.000	0.688	0.708	0.861	0.636	-	0.871	0.000	0.869	0.673	0.615	0.727	-	0.858	0.970
Lights	0	44	508	19	3	-	574	1	41	666	241	104	-	1053	0	11	17	30	55	-	113	0	294	34	31	32	-	391	2131
% Lights	-	100.0	98.1	100.0	100.0	-	98.3	100.0	97.6	97.1	98.4	98.1	-	97.5	-	100.0	100.0	96.8	98.2	-	98.3	-	98.3	97.1	96.9	100.0	-	98.2	97.9
Other Vehicles	0	0	10	0	0	-	10	0	1	20	4	2	-	27	0	0	0	1	1	-	2	0	5	1	1	0	-	7	46
% Other Vehicles	-	0.0	1.9	0.0	0.0	-	1.7	0.0	2.4	2.9	1.6	1.9	-	2.5	-	0.0	0.0	3.2	1.8	-	1.7	-	1.7	2.9	3.1	0.0	-	1.8	2.1
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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Pittsburgh, Pennsylvania, United States 15205
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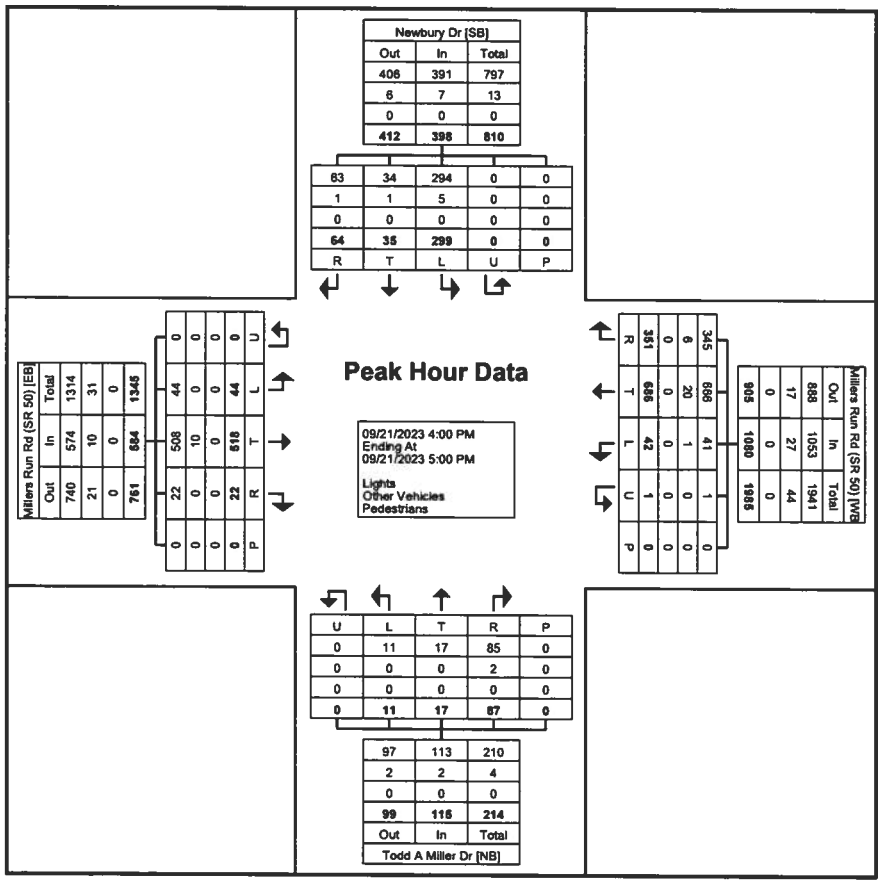


Turning Movement Peak Hour Data Plot (4:00 PM)

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023

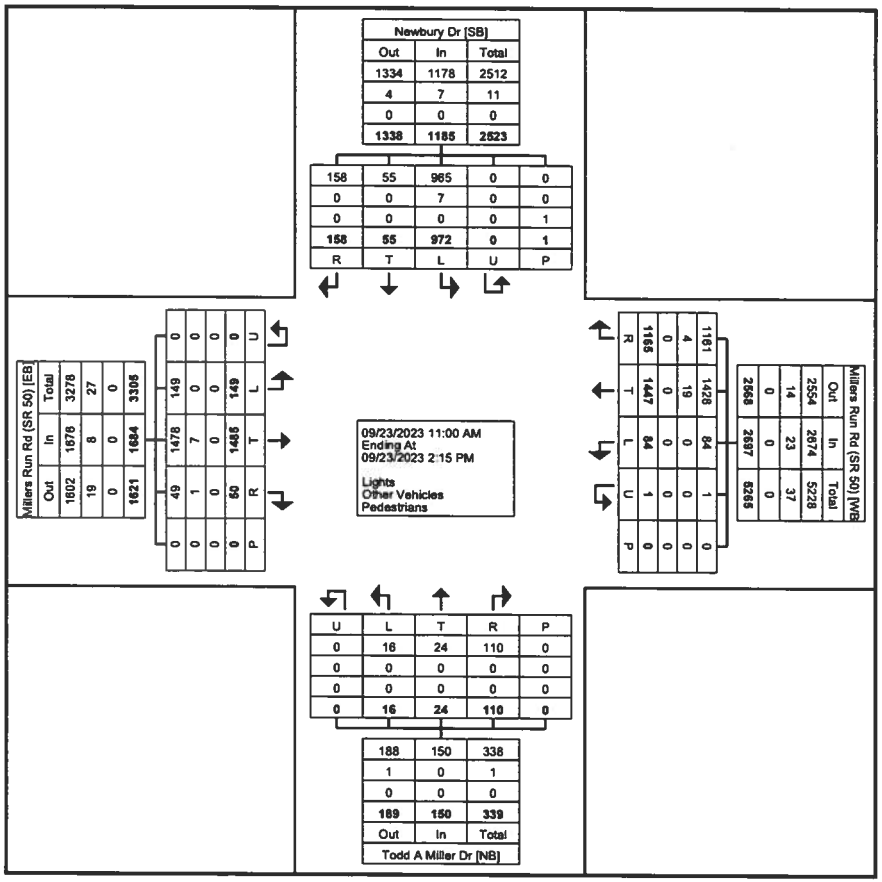
Turning Movement Data

Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	Int. Total
11:00 AM	0	16	116	2	0	0	134	1	2	121	79	24	0	227	0	0	1	3	5	0	9	0	64	8	2	5	0	79	449
11:15 AM	0	10	130	4	0	0	144	0	7	138	62	17	0	224	0	0	1	5	5	0	11	0	88	8	3	9	0	108	487
11:30 AM	0	13	108	2	0	0	123	0	9	78	96	15	0	198	0	2	0	0	2	0	4	0	73	1	4	10	0	88	413
11:45 AM	0	12	119	6	0	0	137	0	12	111	71	22	0	216	0	2	3	2	6	0	13	0	100	9	4	8	1	121	487
Hourly Total	0	51	473	14	0	0	538	1	30	448	308	78	0	865	0	4	5	10	18	0	37	0	325	26	13	32	1	396	1836
12:00 PM	0	9	132	3	0	0	144	0	7	117	57	15	0	196	0	1	6	5	3	0	15	0	71	3	5	7	0	86	441
12:15 PM	0	10	171	5	2	0	188	0	6	113	82	19	0	220	0	2	1	2	6	0	11	0	81	7	5	3	0	96	515
12:30 PM	0	14	130	9	2	0	155	0	5	115	78	21	0	219	0	2	0	4	2	0	8	0	75	3	5	9	0	92	474
12:45 PM	0	11	122	1	0	0	134	0	6	142	70	38	0	256	0	1	5	3	5	0	14	0	104	4	14	16	0	138	542
Hourly Total	0	44	555	18	4	0	621	0	24	487	287	93	0	891	0	6	12	14	16	0	48	0	331	17	29	35	0	412	1972
1:00 PM	0	9	108	2	0	0	119	0	10	134	74	23	0	241	0	0	2	9	2	0	13	0	78	6	9	7	0	100	473
1:15 PM	0	11	106	3	1	0	121	0	8	124	65	30	0	227	0	1	1	3	5	0	10	0	77	4	4	3	0	88	446
1:30 PM	0	19	129	4	0	0	152	0	6	140	86	32	0	264	0	4	2	8	12	0	26	0	83	2	5	5	0	95	537
1:45 PM	0	15	114	4	0	0	133	0	6	114	64	25	0	209	0	1	2	3	10	0	16	0	78	0	6	10	0	94	452
Hourly Total	0	54	457	13	1	0	525	0	30	512	289	110	0	941	0	6	7	23	29	0	65	0	316	12	24	25	0	377	1908
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	149	1485	45	5	0	1684	1	84	1447	884	281	0	2697	0	16	24	47	63	0	150	0	972	55	66	92	1	1185	5716
Approach %	0.0	8.8	88.2	2.7	0.3	-	-	0.0	3.1	53.7	32.8	10.4	-	-	0.0	10.7	16.0	31.3	42.0	-	-	0.0	82.0	4.6	5.6	7.8	-	-	-
Total %	0.0	2.6	26.0	0.8	0.1	-	29.5	0.0	1.5	25.3	15.5	4.9	-	47.2	0.0	0.3	0.4	0.8	1.1	-	2.6	0.0	17.0	1.0	1.2	1.6	-	20.7	-
Lights	0	149	1478	44	5	-	1676	1	84	1428	880	281	-	2674	0	16	24	47	63	-	150	0	965	55	66	92	-	1178	5678
% Lights	-	100.0	99.5	97.8	100.0	-	99.5	100.0	100.0	98.7	99.5	100.0	-	99.1	-	100.0	100.0	100.0	100.0	-	100.0	-	99.3	100.0	100.0	100.0	-	99.4	99.3
Other Vehicles	0	0	7	1	0	-	8	0	0	19	4	0	-	23	0	0	0	0	0	-	0	0	7	0	0	0	-	7	38
% Other Vehicles	-	0.0	0.5	2.2	0.0	-	0.5	0.0	0.0	1.3	0.5	0.0	-	0.9	-	0.0	0.0	0.0	0.0	-	0.0	-	0.7	0.0	0.0	0.0	-	0.6	0.7
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023



Turning Movement Data Plot

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TRAFFIC ▲ PERMITS ▲ SIGNALS

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Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023

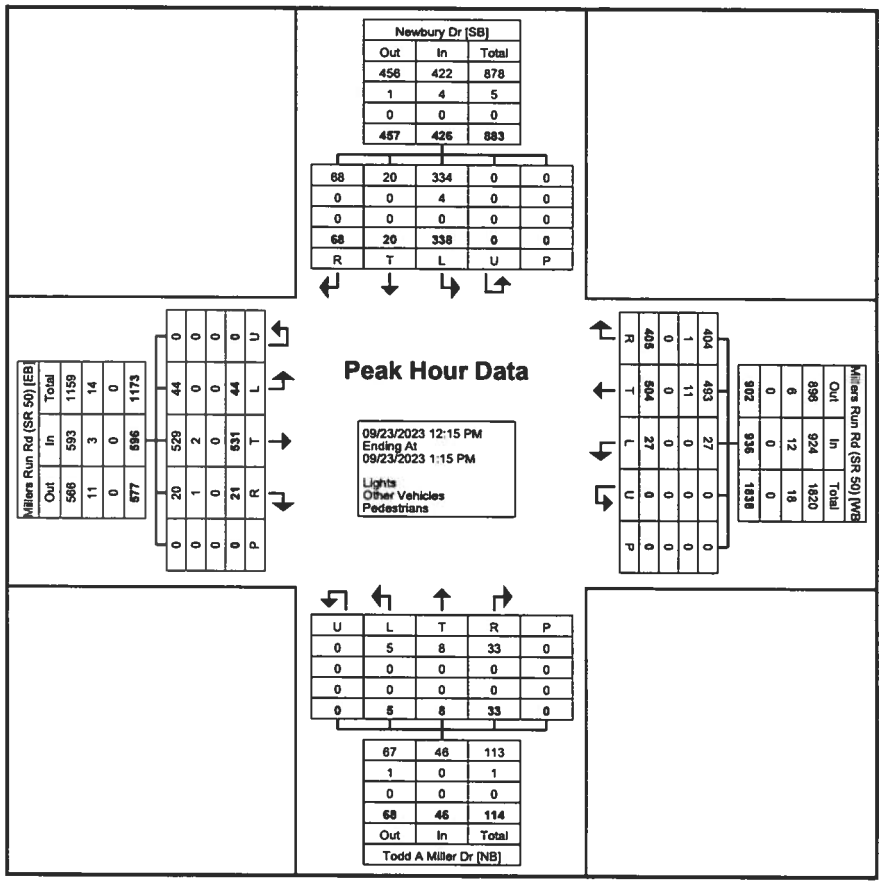
Turning Movement Peak Hour Data (12:15 PM)

Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							Int. Total
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	
12:15 PM	0	10	171	5	2	0	188	0	6	113	82	19	0	220	0	2	1	2	6	0	11	0	81	7	5	3	0	96	515
12:30 PM	0	14	130	9	2	0	155	0	5	115	78	21	0	219	0	2	0	4	2	0	8	0	75	3	5	9	0	92	474
12:45 PM	0	11	122	1	0	0	134	0	6	142	70	38	0	256	0	1	5	3	5	0	14	0	104	4	14	16	0	138	542
1:00 PM	0	9	108	2	0	0	119	0	10	134	74	23	0	241	0	0	2	9	2	0	13	0	78	6	9	7	0	100	473
Total	0	44	531	17	4	0	596	0	27	504	304	101	0	936	0	5	8	18	15	0	46	0	338	20	33	35	0	426	2004
Approach %	0.0	7.4	89.1	2.9	0.7	-	-	0.0	2.9	53.8	32.5	10.8	-	-	0.0	10.9	17.4	39.1	32.6	-	-	0.0	79.3	4.7	7.7	8.2	-	-	-
Total %	0.0	2.2	26.5	0.8	0.2	-	29.7	0.0	1.3	25.1	15.2	5.0	-	46.7	0.0	0.2	0.4	0.9	0.7	-	2.3	0.0	16.9	1.0	1.6	1.7	-	21.3	-
PHF	0.000	0.786	0.776	0.472	0.500	-	0.793	0.000	0.675	0.887	0.927	0.664	-	0.914	0.000	0.625	0.400	0.500	0.625	-	0.821	0.000	0.813	0.714	0.589	0.547	-	0.772	0.924
Lights	0	44	529	16	4	-	593	0	27	493	303	101	-	924	0	5	8	18	15	-	46	0	334	20	33	35		422	1985
% Lights	-	100.0	99.6	94.1	100.0		99.5	-	100.0	97.8	99.7	100.0		98.7	-	100.0	100.0	100.0	100.0	-	100.0	-	98.8	100.0	100.0	100.0	-	99.1	99.1
Other Vehicles	0	0	2	1	0	-	3	0	0	11	1	0		12	0	0	0	0	0	-	0	0	4	0	0	0		4	19
% Other Vehicles	-	0.0	0.4	5.9	0.0	-	0.5	-	0.0	2.2	0.3	0.0	-	1.3	-	0.0	0.0	0.0	0.0	-	0.0	-	1.2	0.0	0.0	0.0	-	0.9	0.9
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023



Turning Movement Peak Hour Data Plot (12:15 PM)

WOOSTER
TRAFFIC ▲ PERMITS ▲ SIGNALS

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (7-9 am)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Data

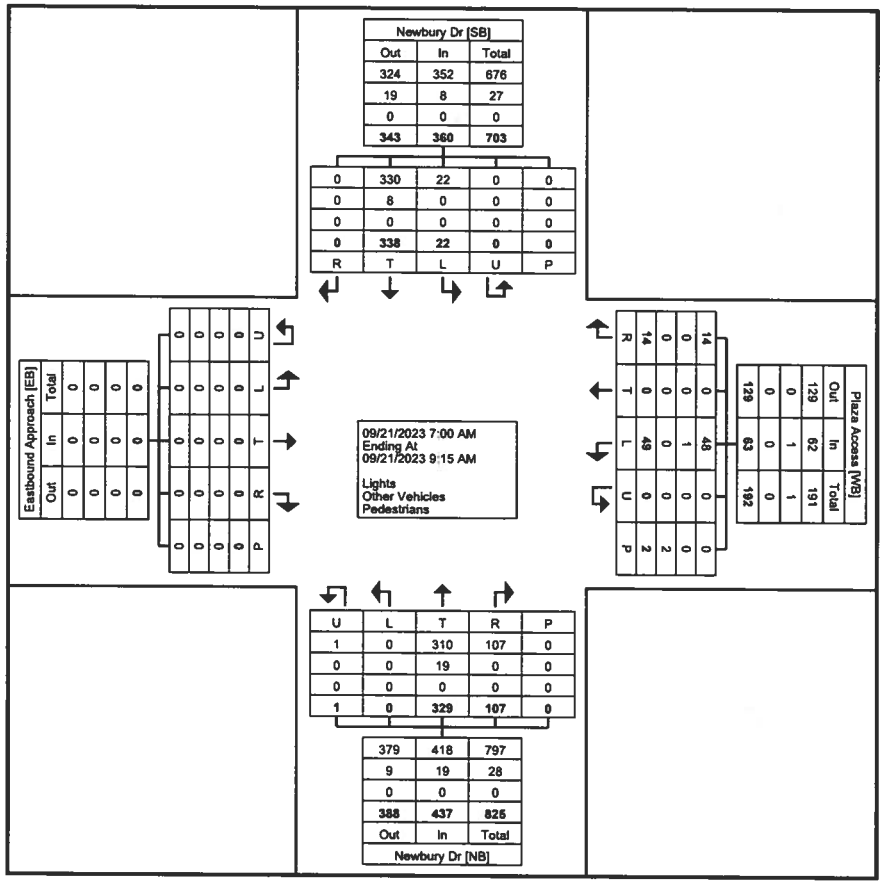
Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	37	7	0	44	0	0	32	0	0	32	77
7:15 AM	0	0	0	0	0	0	0	5	0	0	0	5	0	0	21	15	0	36	0	2	25	0	0	27	68
7:30 AM	0	0	0	0	0	0	0	5	0	0	0	5	0	0	29	6	0	35	0	1	37	0	0	38	78
7:45 AM	0	0	0	0	0	0	0	4	0	2	0	6	0	0	46	20	0	66	0	2	47	0	0	49	121
Hourly Total	0	0	0	0	0	0	0	14	0	3	0	17	0	0	133	48	0	181	0	5	141	0	0	146	344
8:00 AM	0	0	0	0	0	0	0	10	0	3	0	13	0	0	35	15	0	50	0	4	37	0	0	41	104
8:15 AM	0	0	0	0	0	0	0	6	0	4	0	10	0	0	46	13	0	59	0	5	42	0	0	47	116
8:30 AM	0	0	0	0	0	0	0	11	0	1	0	12	1	0	57	16	0	74	0	3	63	0	0	66	152
8:45 AM	0	0	0	0	0	0	0	8	0	3	2	11	0	0	58	15	0	73	0	5	55	0	0	60	144
Hourly Total	0	0	0	0	0	0	0	35	0	11	2	46	1	0	196	59	0	256	0	17	197	0	0	214	516
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	49	0	14	2	63	1	0	329	107	0	437	0	22	338	0	0	360	860
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	77.8	0.0	22.2	-	-	0.2	0.0	75.3	24.5	-	-	0.0	6.1	93.9	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	5.7	0.0	1.6	-	7.3	0.1	0.0	38.3	12.4	-	50.8	0.0	2.6	39.3	0.0	-	41.9	-
Lights	0	0	0	0	-	0	0	48	0	14	-	62	1	0	310	107	-	418	0	22	330	0	-	352	832
% Lights	-	-	-	-	-	-	-	98.0	-	100.0	-	98.4	100.0	-	94.2	100.0	-	95.7	-	100.0	97.6	-	-	97.8	96.7
Other Vehicles	0	0	0	0	-	0	0	1	0	0	-	1	0	0	19	0	-	19	0	0	8	0	-	8	28
% Other Vehicles	-	-	-	-	-	-	-	2.0	-	0.0	-	1.6	0.0	-	5.8	0.0	-	4.3	-	0.0	2.4	-	-	2.2	3.3
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-

WOOSTER
TRAFFIC ▲ PERMITS ▲ SIGNALS

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (7-9 am)
Site Code: 4392
Start Date: 09/21/2023



Turning Movement Data Plot

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (7-9 am)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Peak Hour Data (8:00 AM)

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
8:00 AM	0	0	0	0	0	0	0	10	0	3	0	13	0	0	35	15	0	50	0	4	37	0	0	41	104
8:15 AM	0	0	0	0	0	0	0	6	0	4	0	10	0	0	46	13	0	59	0	5	42	0	0	47	116
8:30 AM	0	0	0	0	0	0	0	11	0	1	0	12	1	0	57	16	0	74	0	3	63	0	0	66	152
8:45 AM	0	0	0	0	0	0	0	8	0	3	2	11	0	0	58	15	0	73	0	5	55	0	0	60	144
Total	0	0	0	0	0	0	0	35	0	11	2	46	1	0	196	59	0	256	0	17	197	0	0	214	516
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	76.1	0.0	23.9	-	-	0.4	0.0	76.6	23.0	-	-	0.0	7.9	92.1	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	6.8	0.0	2.1	-	8.9	0.2	0.0	38.0	11.4	-	49.6	0.0	3.3	38.2	0.0	-	41.5	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.795	0.000	0.688	-	0.885	0.250	0.000	0.845	0.922	-	0.865	0.000	0.850	0.782	0.000	-	0.811	0.849
Lights	0	0	0	0	-	0	0	34	0	11	-	45	1	0	187	59	-	247	0	17	193	0	-	210	502
% Lights	-	-	-	-	-	-	-	97.1	-	100.0	-	97.8	100.0	-	95.4	100.0	-	96.5	-	100.0	98.0	-	-	98.1	97.3
Other Vehicles	0	0	0	0	-	0	0	1	0	0	-	1	0	0	9	0	-	9	0	0	4	0	-	4	14
% Other Vehicles	-	-	-	-	-	-	-	2.9	-	0.0	-	2.2	0.0	-	4.6	0.0	-	3.5	-	0.0	2.0	-	-	1.9	2.7
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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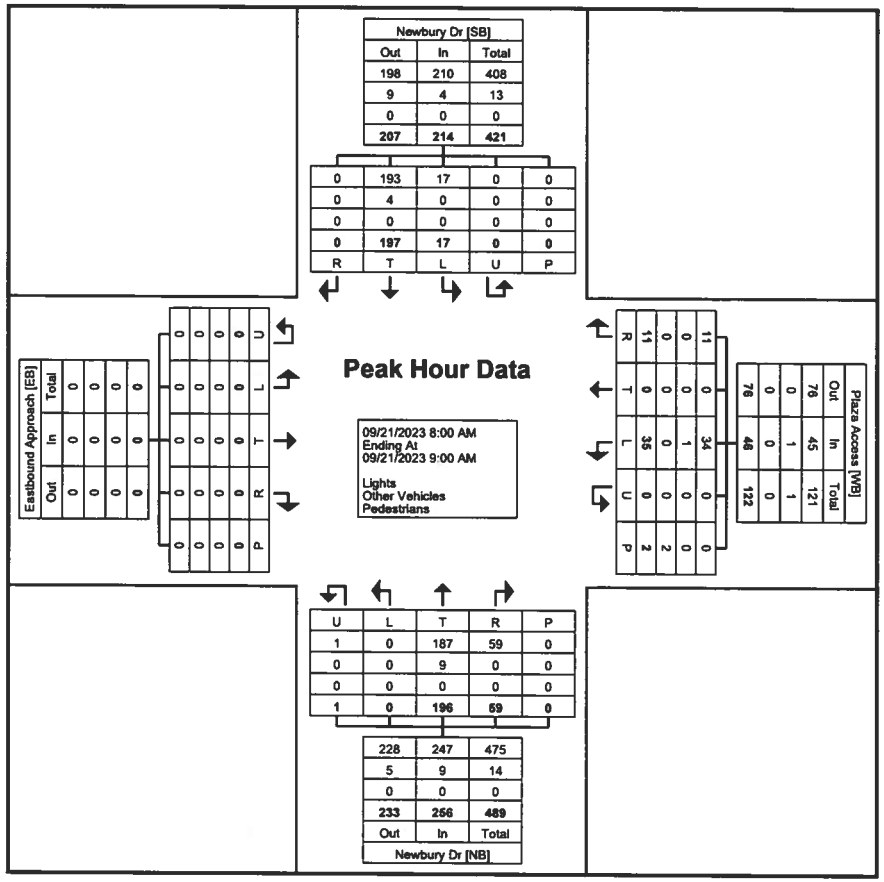


Turning Movement Peak Hour Data Plot (8:00 AM)

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023

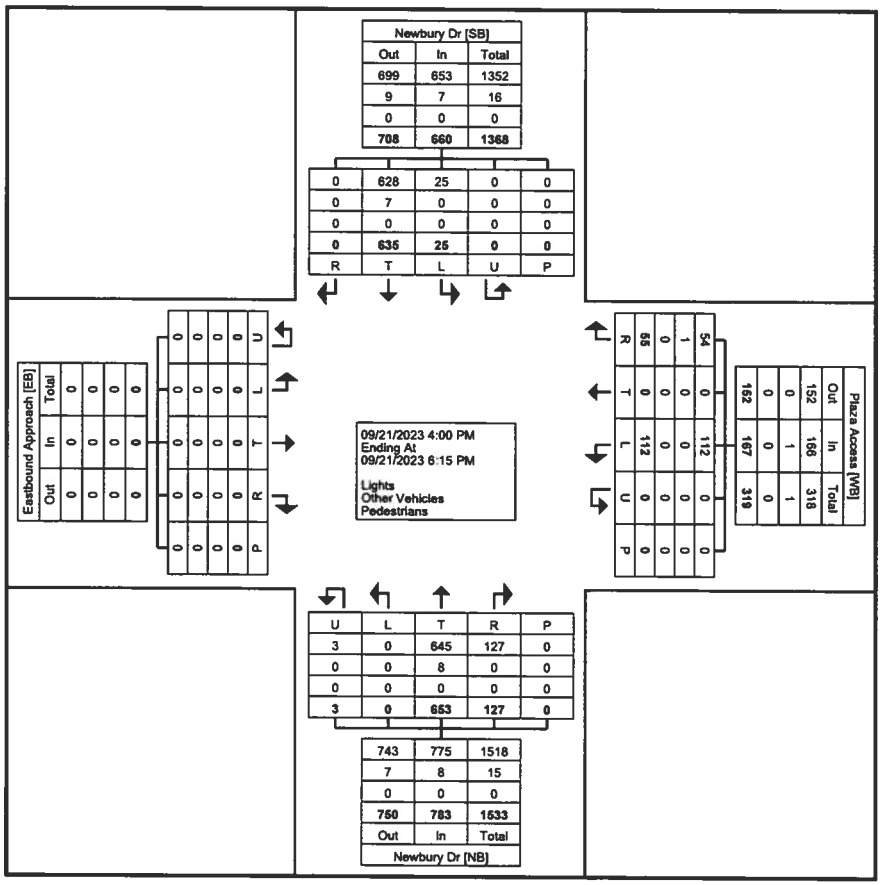
Turning Movement Data

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:00 PM	0	0	0	0	0	0	0	10	0	9	0	19	0	0	75	11	0	86	0	4	115	0	0	119	224
4:15 PM	0	0	0	0	0	0	0	17	0	7	0	24	0	0	64	12	0	76	0	6	80	0	0	86	186
4:30 PM	0	0	0	0	0	0	0	15	0	5	0	20	0	0	101	17	0	118	0	1	65	0	0	66	204
4:45 PM	0	0	0	0	0	0	0	15	0	7	0	22	0	0	91	22	0	113	0	2	74	0	0	76	211
Hourly Total	0	0	0	0	0	0	0	57	0	28	0	85	0	0	331	62	0	393	0	13	334	0	0	347	825
5:00 PM	0	0	0	0	0	0	0	19	0	9	0	28	0	0	82	15	0	97	0	4	76	0	0	80	205
5:15 PM	0	0	0	0	0	0	0	18	0	8	0	26	0	0	69	15	0	84	0	5	66	0	0	71	181
5:30 PM	0	0	0	0	0	0	0	7	0	8	0	15	0	0	89	18	0	107	0	0	81	0	0	81	203
5:45 PM	0	0	0	0	0	0	0	11	0	2	0	13	3	0	82	17	0	102	0	3	78	0	0	81	196
Hourly Total	0	0	0	0	0	0	0	55	0	27	0	82	3	0	322	65	0	390	0	12	301	0	0	313	785
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	112	0	55	0	167	3	0	653	127	0	783	0	25	635	0	0	660	1610
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	67.1	0.0	32.9	-	-	0.4	0.0	83.4	16.2	-	-	0.0	3.8	96.2	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	7.0	0.0	3.4	-	10.4	0.2	0.0	40.6	7.9	-	48.6	0.0	1.6	39.4	0.0	-	41.0	-
Lights	0	0	0	0	-	0	0	112	0	54	-	166	3	0	645	127	-	775	0	25	628	0	-	653	1594
% Lights	-	-	-	-	-	-	-	100.0	-	98.2	-	99.4	100.0	-	98.8	100.0	-	99.0	-	100.0	98.9	-	-	98.9	99.0
Other Vehicles	0	0	0	0	-	0	0	0	0	1	-	1	0	0	8	0	-	8	0	0	7	0	-	7	16
% Other Vehicles	-	-	-	-	-	-	-	0.0	-	1.8	-	0.6	0.0	-	1.2	0.0	-	1.0	-	0.0	1.1	-	-	1.1	1.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023



Turning Movement Data Plot

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023

Turning Movement Peak Hour Data (4:00 PM)

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
4:00 PM	0	0	0	0	0	0	0	10	0	9	0	19	0	0	75	11	0	86	0	4	115	0	0	119	224
4:15 PM	0	0	0	0	0	0	0	17	0	7	0	24	0	0	64	12	0	76	0	6	80	0	0	86	186
4:30 PM	0	0	0	0	0	0	0	15	0	5	0	20	0	0	101	17	0	118	0	1	65	0	0	66	204
4:45 PM	0	0	0	0	0	0	0	15	0	7	0	22	0	0	91	22	0	113	0	2	74	0	0	76	211
Total	0	0	0	0	0	0	0	57	0	28	0	85	0	0	331	62	0	393	0	13	334	0	0	347	825
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	67.1	0.0	32.9	-	-	0.0	0.0	84.2	15.8	-	-	0.0	3.7	96.3	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	6.9	0.0	3.4	-	10.3	0.0	0.0	40.1	7.5	-	47.6	0.0	1.6	40.5	0.0	-	42.1	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.838	0.000	0.778	-	0.885	0.000	0.000	0.819	0.705	-	0.833	0.000	0.542	0.726	0.000	-	0.729	0.921
Lights	0	0	0	0	-	0	0	57	0	28	-	85	0	0	323	62	-	385	0	13	327	0	-	340	810
% Lights	-	-	-	-	-	-	-	100.0	-	100.0	-	100.0	-	-	97.6	100.0	-	98.0	-	100.0	97.9	-	-	98.0	98.2
Other Vehicles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	8	0	-	8	0	0	7	0	-	7	15
% Other Vehicles	-	-	-	-	-	-	-	0.0	-	0.0	-	0.0	-	-	2.4	0.0	-	2.0	-	0.0	2.1	-	-	2.0	1.8
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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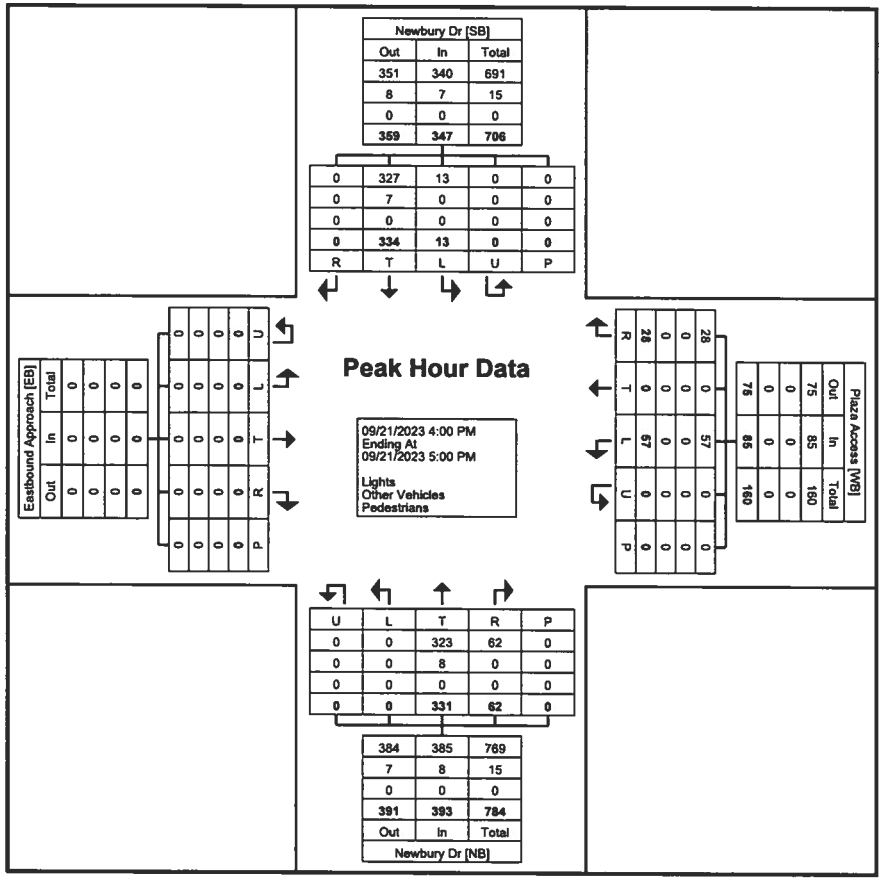

Turning Movement Peak Hour Data Plot (4:00 PM)

David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023

Turning Movement Data

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
11:00 AM	0	0	0	0	0	0	0	14	0	7	0	21	0	1	87	28	0	116	0	4	68	0	0	72	209
11:15 AM	0	0	0	0	0	0	0	29	0	3	0	32	0	0	74	18	0	92	0	3	79	0	0	82	206
11:30 AM	0	0	0	0	0	0	0	24	0	7	0	31	0	0	95	33	0	128	0	5	65	0	0	70	229
11:45 AM	0	0	0	0	0	0	0	22	0	10	0	32	0	0	71	31	0	102	0	4	98	0	0	102	236
Hourly Total	0	0	0	0	0	0	0	89	0	27	0	116	0	1	327	110	0	438	0	16	310	0	0	326	880
12:00 PM	0	0	0	0	0	0	0	24	0	4	0	28	1	0	67	21	0	89	0	4	66	0	0	70	187
12:15 PM	0	0	0	0	0	0	0	22	0	8	2	30	0	0	78	36	4	114	0	3	71	0	0	74	218
12:30 PM	0	0	0	1	0	1	0	26	0	5	0	31	0	0	91	24	0	115	0	3	66	0	0	69	216
12:45 PM	0	0	0	0	0	0	0	36	0	9	0	45	0	0	91	31	0	122	0	6	104	0	0	110	277
Hourly Total	0	0	0	1	0	1	0	108	0	26	2	134	1	0	327	112	4	440	0	16	307	0	0	323	898
1:00 PM	0	0	0	0	0	0	0	19	0	8	0	27	1	0	88	22	0	111	0	3	76	0	0	79	217
1:15 PM	0	0	0	0	0	0	0	27	0	1	0	28	0	0	85	20	0	105	0	4	62	0	0	66	199
1:30 PM	0	0	0	0	0	0	0	25	0	7	0	32	0	0	102	33	0	135	0	7	73	0	0	80	247
1:45 PM	0	0	0	0	0	0	0	26	0	3	0	29	0	0	89	16	0	105	0	6	71	0	0	77	211
Hourly Total	0	0	0	0	0	0	0	97	0	19	0	116	1	0	364	91	0	456	0	20	282	0	0	302	874
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	1	0	1	0	294	0	72	2	366	2	1	1018	313	4	1334	0	52	899	0	0	951	2652
Approach %	0.0	0.0	0.0	100.0	-	-	0.0	80.3	0.0	19.7	-	-	0.1	0.1	76.3	23.5	-	-	0.0	5.5	94.5	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	11.1	0.0	2.7	-	13.8	0.1	0.0	38.4	11.8	-	50.3	0.0	2.0	33.9	0.0	-	35.9	-
Lights	0	0	0	0	-	0	0	293	0	71	-	364	2	0	1014	310	-	1326	0	52	893	0	-	945	2635
% Lights	-	-	-	0.0	-	0.0	-	99.7	-	98.6	-	99.5	100.0	0.0	99.6	99.0	-	99.4	-	100.0	99.3	-	-	99.4	99.4
Other Vehicles	0	0	0	1	-	1	0	1	0	1	-	2	0	1	4	3	-	8	0	0	6	0	-	6	17
% Other Vehicles	-	-	-	100.0	-	100.0	-	0.3	-	1.4	-	0.5	0.0	100.0	0.4	1.0	-	0.6	-	0.0	0.7	-	-	0.6	0.6
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	4	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-

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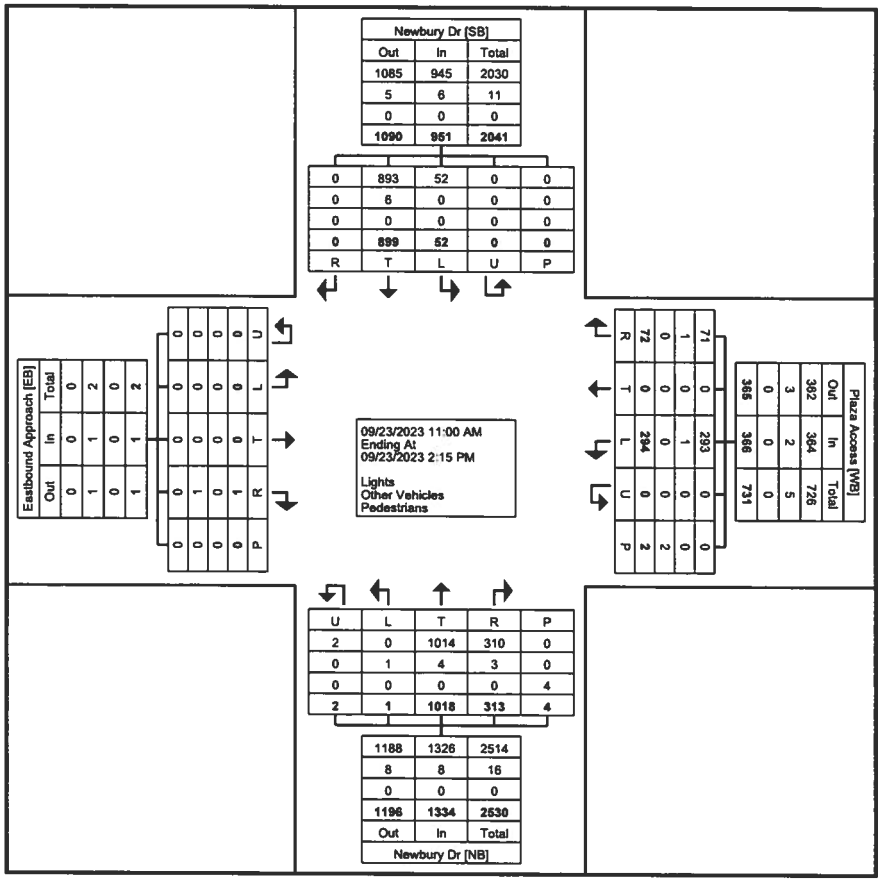
Count Name: Newbury Dr. & Plaza Access (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023



Turning Movement Data Plot

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412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023

Turning Movement Peak Hour Data (12:45 PM)

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	0	0	0	0	0	0	36	0	9	0	45	0	0	91	31	0	122	0	6	104	0	0	110	277
1:00 PM	0	0	0	0	0	0	0	19	0	8	0	27	1	0	88	22	0	111	0	3	76	0	0	79	217
1:15 PM	0	0	0	0	0	0	0	27	0	1	0	28	0	0	85	20	0	105	0	4	62	0	0	66	199
1:30 PM	0	0	0	0	0	0	0	25	0	7	0	32	0	0	102	33	0	135	0	7	73	0	0	80	247
Total	0	0	0	0	0	0	0	107	0	25	0	132	1	0	366	106	0	473	0	20	315	0	0	335	940
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	81.1	0.0	18.9	-	-	0.2	0.0	77.4	22.4	-	-	0.0	6.0	94.0	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	11.4	0.0	2.7	-	14.0	0.1	0.0	38.9	11.3	-	50.3	0.0	2.1	33.5	0.0	-	35.6	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.743	0.000	0.694	-	0.733	0.250	0.000	0.897	0.803	-	0.876	0.000	0.714	0.757	0.000	-	0.761	0.848
Lights	0	0	0	0	-	0	0	106	0	24	-	130	1	0	366	104	-	471	0	20	313	0	-	333	934
% Lights	-	-	-	-	-	-	-	99.1	-	96.0	-	98.5	100.0	-	100.0	98.1	-	99.6	-	100.0	99.4	-	-	99.4	99.4
Other Vehicles	0	0	0	0	-	0	0	1	0	1	-	2	0	0	0	2	-	2	0	0	2	0	-	2	6
% Other Vehicles	-	-	-	-	-	-	-	0.9	-	4.0	-	1.5	0.0	-	0.0	1.9	-	0.4	-	0.0	0.6	-	-	0.6	0.6
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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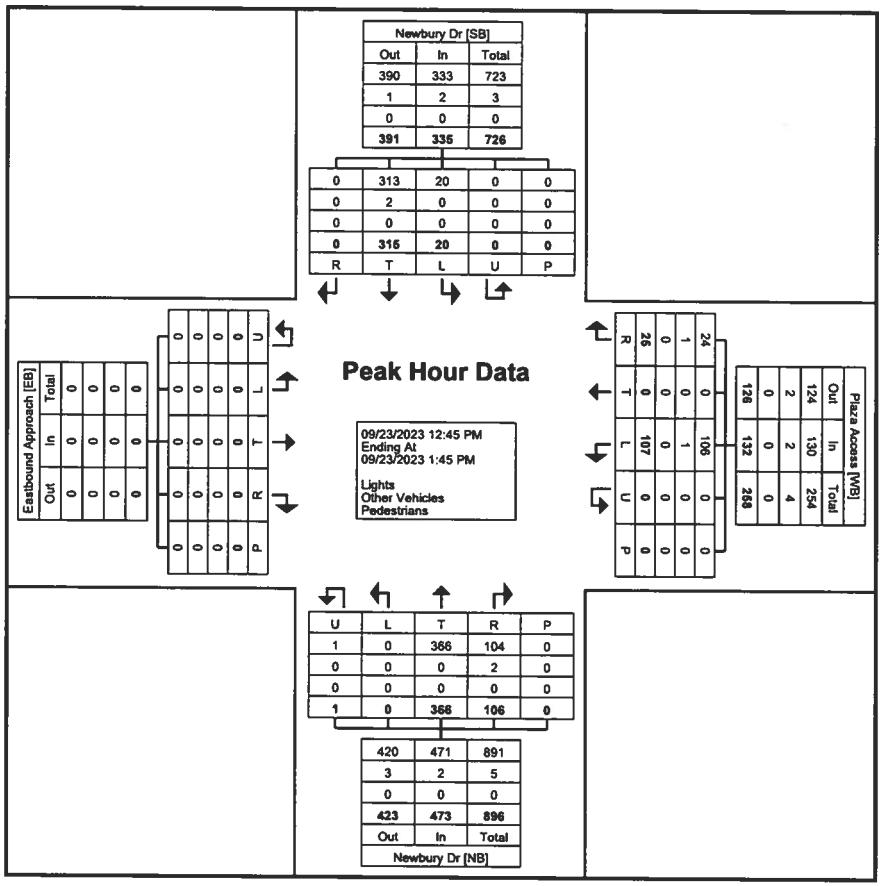


Turning Movement Peak Hour Data Plot (12:45 PM)

APPENDIX C

Photo Log of Existing Study Intersections

On Todd A .Miller Drive, looking north toward the intersection with Millers Run Road (SR 0050) / Newbury Drive

On Newbury Drive, looking south toward the intersection with Millers Run Road (SR 0050) / Todd A .Miller Drive

On Millers Run Road (SR 0050), looking east toward the intersection with Newbury Drive / Todd A .Miller Drive

On Millers Run Road (SR 0050), looking west toward the intersection with Newbury Drive / Todd A .Miller Drive

On Newbury Drive, looking north toward the intersection with the Plaza Access / Proposed Site Drive C

On Newbury Drive, looking south toward the intersection with the Plaza Access / Proposed Site Drive C

At the Proposed Site Drive C, looking east across Newbury Drive to the intersection with the Plaza Access

At the Plaza Access, looking west across Newbury Drive to the Proposed Site Drive C

APPENDIX D

Traffic Signal Permit Plans

PERMIT NO. 1-105-14 SHEET 2 OF 3

DATE ISSUED _____ DATE REVISED 9/10/2018

SIGNS

PLAN SYMBOL	SERIES DESIGNATION	SIZE W x H	DESCRIPTION	QTY.
A	R10-3EL	9"x15"	EDUC. PUSH BUTTON FOR WALK SIGNAL WITH COUNTDOWN TIMER SIGN ←	4
B	R10-3ER	9"x15"	EDUC. PUSH BUTTON FOR WALK SIGNAL WITH COUNTDOWN TIMER SIGN →	4
C	R3-5L	30"x36"	LEFT TURN	5
D	R3-5S	30"x36"	STRAIGHT THRU	4
E	R3-5R	30"x36"	RIGHT TURN	2
F	R3-6SR	30"x36"	OPTIONAL RIGHT TURN	2
G	D3-4*	**	Millers Run Rd	2
H	D3-5*	**	← Newbury Dr / Municipal Dr →	1
I	D3-5*	**	← Municipal Dr / Newbury Dr →	1
J	R1-2	36"x36"	YIELD	2
K	R1-5L	18"x18"	YIELD HERE TO PEDESTRIANS ↙	2
L	R10-10L	30"x36"	LEFT TURN SIGNAL	5
M	OM1-3	18"x18"	OBJECT MARKER	1
N	R4-7	24"x30"	KEEP RIGHT	1
O	R4-102	30"x36"	LEFT LANE NO TRUCKS	1

*WHITE LEGEND ON GREEN BACKGROUND

**THE CONTRACTOR IS RESPONSIBLE TO PROVIDE SHOP DRAWINGS, DETAILING THE REQUIRED SIGN SIZE, IN ACCORDANCE WITH PENNDOT PUBLICATION 111M, TC-8700 SERIES MOST RECENT VERSION, AND THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

SIGNAL ASSEMBLY NOTES:

EQUIP VEHICLE SIGNALS WITH SCOOP TUNNEL VISORS.

EQUIP ALL VEHICLE SIGNALS WITH METAL LOUVERED REFLECTIVE BACK PLATES.

ALL SIGNALS L.E.D. MODULES.

MIN/MAX HEIGHT FOR VEHICULAR SIGNALS OVER ROADWAY SHALL BE 17'/18'.

MIN/MAX HEIGHT FOR PEDESTRIAN SIGNALS SHALL BE 10'/15'.

ALL PEDESTRIAN SIGNALS SINGLE UNIT, HAND/MAN OVERLAY, EQUIPPED WITH L.E.D. LENSES.

FINAL PLACEMENT OF SIGNALS DETERMINED BY REPRESENTATIVE OF TRAFFIC ENGINEERING UNIT.

LASH SIGNAL CABLE TO SPAN. NO CABLE TIES PERMITTED.

NOTES:

1. SEE SHEET 2 of 20 FOR:
   - DETECTOR LAYOUT DETAIL
   - ELECTRICAL SERVICE TYPE B MODIFIED DETAIL
   - ELECTRICAL SERVICE TYPE B MODIFIED GROUND DETAIL
   - TYPICAL SIGNAL SUPPORT GROUNDING DETAIL
   - GENERAL NOTES
2. SEE SIGNING, PAVEMENT MARKING & DELINEATION PLANS FOR ADDITIONAL INFORMATION

REVISIONS:
- INSTALLED CONTROLLER ASSEMBLY
- INSTALLED RADIO COMMUNICATION SYSTEM
- INSTALLED ADAPTIVE TRAFFIC SIGNAL SYSTEM AND VIDEO DETECTION
- INSTALLED LUMINAIRE ARMS

LEGEND

DY/4" - DOUBLE YELLOW/WIDTH
BW/6" - BROKEN WHITE LINE/WIDTH
W/6" - SOLID WHITE LINE/WIDTH
Y/4" - SOLID YELLOW LINE/WIDTH
dW/6" - DASHED WHITE LINE/WIDTH
6 - MAST ARM
2 - STRAIN POLE
2 - PEDESTAL POLE
C/2" - CONDUIT/SIZE
- CONTROLLER ASSEMBLY
- DETECTABLE WARNING SURFACE
- WORK DONE BY OTHERS
- RADIO COMMUNICATION ANTENNA
1 - VEHICLE VIDEO DETECTOR
(1) - VIDEO DETECTION ZONE

14 - PEDESTRIAN SIGNAL HEAD
4 - VEHICULAR SIGNAL HEAD
A - SIGN
4 - VEHICLE DETECTOR
4 - PEDESTRIAN PUSH BUTTON
4 - JUNCTION BOX
0 - PREEMPTION DETECTOR
- CONFIRMATION LIGHT
- EXISTING FENCE
- EXISTING VEHICLE DETECTOR
- EXISTING SIGN
- EXISTING JUNCTION BOX
C/2 - EXISTING CONDUIT/SIZE
- LUMINAIRE ARM

NEWBURY DRIVE
NO POSTED SPEED LIMIT
NO SIGNALS WITHIN ONE MILE
+1.00%
BW/6"
W/6"
Y/4"

32' STRAIN POLE 3

14' PEDESTAL POLE 4
INCORPORATE BREAKAWAY POLE FOUNDATION INTO CURB RAMP BACKWALL

SR 0050 STA 209+73.75
SEG 0090 OFFSET 1219
SEG 0091 OFFSET 1304

PAVE AREA TO PEDESTRIAN ACCESSIBLE CONDITION

1280' TO SIGNAL AT SR 0050 & MILLERS RUN ROAD

SR 0050 MILLERS RUN ROAD

SHOULDER
10' SHOULDER
40 MPH -0.5%
(2) 6'x6' Ø2
(2) C/1"
JB12
250' TO STOP BAR
55' MAST ARM 7
L.A. 25'
L.A. 15'
L.A. 30'
W/24" TYP
W/12" TYP
10' TYP
4-C/1"
(6) C/1"
JB27
C/2"
C/3"
2-C/3"
2-C/1"
(2) C/1"
JB12

32' POLE 8
60' MAST ARM 6
40' MAST ARM 8
250' TO STOP BAR
50' MA
TO HICKORY GRADE RD
LEGAL RIGHT OF WAY LINE
515' TO SIGNAL AT HICKORY GRADE ROAD

CONTROLLER ASSMEBLY TYPE II MOUNTING ELECTRICAL SERVICE TYPE C MODIFIED

*EXISTING ITEM FROM SIGNAL AT HICKORY GRADE RD

32' STRAIN POLE 2
INCORPORATE POLE FOUNDATION INTO CURB RAMP BACKWALL

14' PEDESTAL POLE 9
INCORPORATE BREAKAWAY POLE FOUNDATION INTO CURB RAMP BACKWALL

MUNICIPAL DRIVE
NO SIGNALS WITHIN ONE MILE
NO POSTED SPEED LIMIT

NOTE:

DRIVEWAY CONSTRUCTION BEYOND THE RIGHT OF WAY TO BE DONE BY OTHERS. COORDINATE INSTALLATION OF TRAFFIC POLES, SIGNS, MARKINGS AND DETECTORS WITH DRIVEWAY CONTRACTOR(S).

SIGNALS

SIGNALS 1-4-7-8-11
SIGNALS 2-3-5-6-9 10-12-13
SYMBOL TYPE B PEDESTRIAN SIGNAL WITH COUNTDOWN TIMER
SIGNALS 14-15-16-17 18-19-20-21
a-b-c-d

REVISIONS ONLY

DANIEL RAY FRITZ — REGISTERED PROFESSIONAL — ENGINEER — 10/16/2018

RAYMOND FRANCIS CARUSO — COMMONWEALTH OF PENNSYLVANIA — REGISTERED PROFESSIONAL — ENGINEER

COUNTY: ALLEGHENY
MUNICIPALITY: SOUTH FAYETTE TOWNSHIP
INTERSECTION: S.R. 0050 (MILLERS RUN RD.) AND NEWBURY DR. / MUNICIPAL DR.

APPROVED BY: _____ 10-24-2018
MUNICIPAL OFFICIAL DATE

RECOMMENDED: _____ 10/24/18
DISTRICT TRAFFIC ENGINEER DATE

SCALE: 0 25 50 FEET

PLOTTED: 9/17/2018 2:53:11 PM
OPERATOR: cbaker
FILE NAME: N:\35013-000\CADD\Traffic\2015 GLG 127 - South Fayette\Final Permit Plans\26-003-35013-TSP (SR 50 AT MUNICIPAL DR AND NEWBURY DR) REVISED 110-041

PERMIT NO. 1-105-14 SHEET 3 OF 3

DATE ISSUED _____ DATE REVISED 9/10/2018

PHASING DIAGRAM

NEMA RACKMOUNT CONTROLLER WITH MANUAL CORD UPS AND BATTERY BACKUP

	PHASE 1+5	PHASE 1+6	PHASE 2+5	PHASE 2+6	PHASE 3+7	PHASE 3+8	PHASE 4+7	PHASE 4+8	PREEMPT 1+6	PREEMPT 2+5	PREEMPT 3+8	PREEMPT 4+7	EMERGENCY FLASHING
	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	
SIGNALS	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	1 2 3 4	
1	-G- -Y- R	-G- -G- -Y- R	R R R R	R R R R	R R R	R R R	R R R R	R R R R	SELECTIVE CLEARANCE -G- -Y- R	SELECTIVE CLEARANCE R R R	SELECTIVE CLEARANCE R R R	SELECTIVE CLEARANCE R R R	OFF
2-3	R R R	G G Y R	R R R R	G G Y R	R R R	R R R R	R R R R	R R R R	G Y R	R R R	R R R	R R R	Y
4	-G- -Y- R	R R R R	-G- -G- -Y- R	R R R R	R R R	R R R R	R R R R	R R R R	R R R	-G- -Y- R	R R R	R R R	OFF
5-6	R R R	R R R R	G G Y R	G G Y R	R R R	R R R R	R R R R	R R R R	R R R	G Y R	R R R	R R R	Y
7-8	R R R	R R R R	R R R R	R R R R	-G- -Y- R	R R R R	-G- -Y- R	R R R R	R R R	R R R	R R R	-G- -Y- R	OFF
9-10	R R R	R R R R	R R R R	R R R R	R R R	R R R R	G Y R R	R R R R	R R R	R R R	R R R	G Y R	R
11	R R R	R R R R	R R R R	R R R R	-G- -Y- R	-G- -G- -Y- R	R R R R	R R R R	R R R	R R R	-G- -Y- R	R R R	OFF
12-13	R R R	R R R R	R R R R	R R R R	G G Y R	R R R R	R R R R	G G Y R	R R R	R R R	G Y R	R R R	R
◇P 14-19	DW DW DW	DW DW DW DW	DW DW DW DW	DW DW DW DW	DW DW DW	W FD DW DW	DW DW DW DW	W FD DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF
◇P 15-16	DW DW DW	DW DW DW DW	W FD DW DW	DW DW DW DW	DW DW DW	DW DW DW DW	DW DW DW DW	DW DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF
◇P 17-18	DW DW DW	W FD DW DW	DW DW DW DW	W FD DW DW	DW DW DW	DW DW DW DW	DW DW DW DW	W FD DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF
◇P 20-21	DW DW DW	DW DW DW DW	DW DW DW DW	DW DW DW DW	DW DW DW	DW DW DW DW	DW DW DW DW	W FD DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF DW DW DW	OFF
a	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF ON OFF OFF	OFF OFF OFF OFF	OFF
b	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF ON OFF OFF	OFF
c	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF ON OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF
d	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF ON OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF OFF OFF OFF	OFF
FIXED	X 4 2	X 4 2	X 4 2	X 4 2	X 3 3	X 3 3	X 3 3	X 3 3	(X) 4 2	(X) 4 2	(X) 3 3	(X) 3 3	
MIN. INITIAL	5	5	5	15	5	5	5	5					
SEC. ACT.				2.1									
MAX. INITIAL				25									
PASSAGE	2	2	2	10	2	2	2	2	(11)	(11)	(11)	(11)	
BEFORE RED.				15									
TO REDUCE.				10									
MIN. GAP				6									
MAX. I	25	25	10	60	20	20	20	35					
MAX. II	25	25	10	60	20	20	20	35					
			10				20						
PEDESTRIAN		(P)	(P)	7 23		(P2)	(P2)	7 23					
MEMORY	L	L	L	MN	L	L	L	NL					

MEMORY INCLUDES

MAX I - ALL OTHER TIMES
MAX II - 0600 TO 0900, MON THRU FRI
MAX III - 1500 TO 1800, MON THRU FRI

◇P UPON PEDESTRIAN ACTIVATION, OTHERWISE "DON'T WALK" AT ALL TIMES.
(P) TIMING WILL BE AS SHOWN IN PHASE 2+6. IT MAY TIME OUT IN THIS PHASE OR BE COMPLETED IN PHASE 2+6
(P2) TIMING WILL BE AS SHOWN IN PHASE 4+8 IT MAY TIME OUT IN THIS PHASE OR BE COMPLETED IN PHASE 4+8
(X) DURATION OF EMERGENCY VEHICLE ACTUATION OR MAXIMUM OF 60 SECONDS.

COORDINATION NOTES

CONTROLLERS INTERCONNECTED USING WIRELESS RADIO COMMUNICATION SYSTEM AND ARE PART OF AN ADAPTIVE SIGNAL SYSTEM AT THE FOLLOWING INTERSECTIONS:

S.R. 0050 AT S.R. 3026 (MILLERS RUN RD)
S.R. 0050 AT MUNICIPAL DR & NEWBURY DR
S.R. 0050 AT HICKORY GRADE RD
S.R. 0050 AT I-79 SB RAMPS
S.R. 0050 AT I-79 NB RAMPS
S.R. 0050 AT S.R. 3003 (WASHINGTON PIKE)
S.R. 0050 AT S.R. 3034 (CHARTIERS ST) & CHURCH ST
S.R. 3003 (WASHINGTON PIKE) AT DANIELL DR

PHASING NOTES

(1) →G- IF PHASE 1+6 FOLLOWS
(2) →G- IF PHASE 1+5 FOLLOWS
(3) →G- IF PHASE 2+5 FOLLOWS
(4) G IF PHASE 2+6 FOLLOWS
(5) G IF PHASE 1+6 FOLLOWS
(6) G IF PHASE 2+5 FOLLOWS
(7) →G- IF PHASE 4+7 FOLLOWS
(8) →G- IF PHASE 3+7 FOLLOWS
(9) →G- IF PHASE 3+8 FOLLOWS
(10) G IF PHASE 4+8 FOLLOWS
(11) PASSAGE TIME EQUALS THE TIME THE EMERGENCY VEHICLE ACTUATION IS IN CONTROL OF THE INTERSECTION.
(X) DURATION OF EMERGENCY VEHICLE ACTUATION OR MAXIMUM OF 60 SECONDS.

EMERGENCY VEHICLE PREEMPTION NOTES

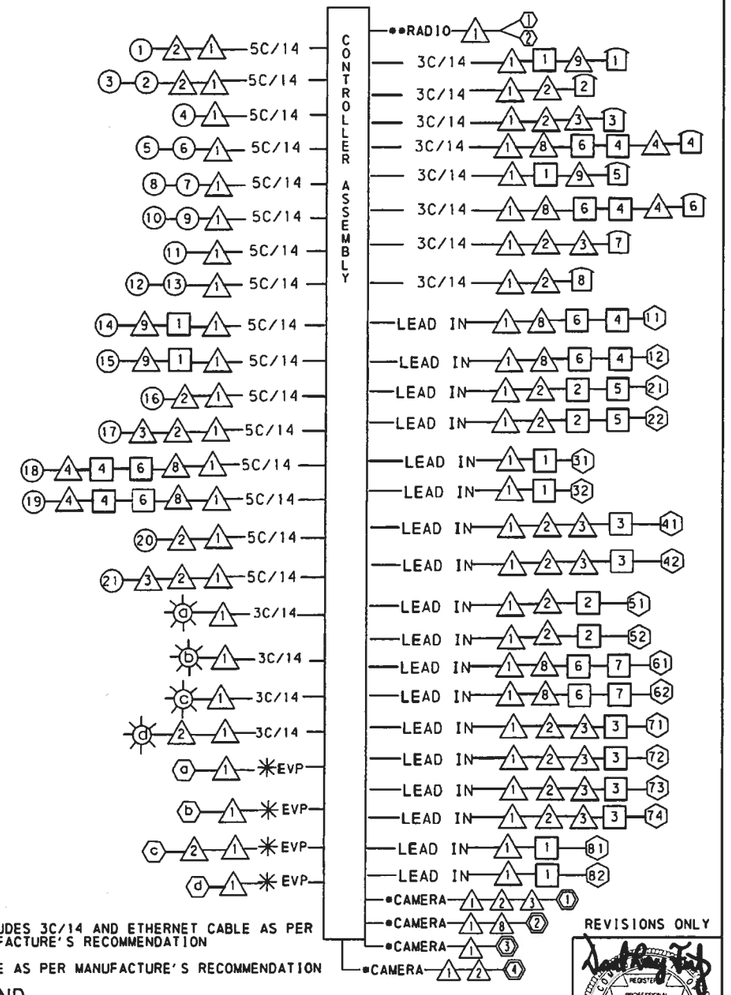
EMERGENCY PREEMPTION MAY OCCUR DURING ANY INTERVAL OF THE NORMAL CONTROLLER OPERATION. DEPENDING ON THE DIRECTION OF TRAVEL OF THE EMERGENCY VEHICLE, ONE OF THE FOLLOWING SHALL BE DISPLAYED: EMERGENCY PREEMPT PHASE 1+6, 2+5, 3+8, OR 4+7. THE SYSTEM SHALL PROVIDE SERVICE ON A FIRST-COME-FIRST-SERVED BASIS. ONCE THE FIRST PRIORITY VEHICLE CALLS THE SYSTEM, OTHER PREEMPTIVE VEHICLES SHALL BE PREVENTED FROM ENTERING CALLS UNTIL THE FIRST EMERGENCY VEHICLE RELEASES CONTROL AND CLEARS THE INTERSECTION.

UPON ACTIVATION OF AN EMERGENCY VEHICLE:

1. IF THE CONTROLLER OPERATION IS IN INTERVAL 1 OF A NON-PREEMPTION PHASE, THE CONTROLLER SHALL TERMINATE THE INTERVAL IMMEDIATELY AND PROCEED NORMALLY THROUGH THE YELLOW AND ALL RED INTERVALS PROCEEDING TO THE PREEMPTION PHASE.
2. IF THE CONTROLLER OPERATION IS IN INTERVAL 1 OF A PREEMPTION PHASE, THE CONTROLLER SHALL REMAIN IN THAT INTERVAL.
3. IF THE CONTROLLER OPERATION IS IN THE YELLOW OR ALL RED INTERVAL OF ANY PHASE, THE CONTROLLER SHALL TIME OUT THOSE INTERVALS NORMALLY BEFORE PROCEEDING TO THE PREEMPTION PHASES.
4. PROVIDE A FAIL SAFE INDICATION CONSISTING OF A FLASHING WHITE LIGHT FOR THE DIRECTION ON WHICH THE EMERGENCY VEHICLE IS APPROACHING. WHEN A CALL IS RECEIVING, THE FAIL SAFE INDICATION SHALL BE ACTIVATED. FLASH AT A RATE NOT LESS THAN 50 NOT MORE THAN 60 TIMES PER MINUTE.
5. UPON TERMINATION OF THE PREEMPTION PHASES, THE CONTROLLER SHALL PROCEED NORMALLY THROUGH THE YELLOW AND ALL RED INTERVALS TO NORMAL "PHASE NEXT" OPERATION.
6. ANY WALK INDICATION SHALL TERMINATE IMMEDIATELY FOLLOWED BY A FLASHING DON'T WALK INDICATION FOR THE NORMAL PEDESTRIAN CLEARANCE INTERVAL BEFORE PROCEEDING TO THE EMERGENCY PREEMPTION ROUTINE IN NOTES 1 & 2.
7. DISPLAY ANY FLASHING DON'T WALK INDICATION FOR NORMAL PEDESTRIAN CLEARANCE INTERVAL BEFORE PROCEEDING TO THE EMERGENCY PREEMPTION ROUTINE IN NOTES 1 & 2.
8. THE PREEMPTION PHASE GREEN INTERVAL SHALL BE 10 SECONDS AND THEN EXTEND FOR THE LENGTH OF THE PREEEMPTION ACTUATION OR A MAXIMUM OF 60 SECONDS.
9. IF THE PREEMPTION OCCURS DURING CONFLICT/TIME CLOCK FLASH THE TRAFFIC SIGNAL SHALL CONTINUE FLASHING.
10. PREEMPT TO COORDINATION: USED WHEN EMERGENCY PREEMPTION IS ACTIVATED DURING COORDINATION OPERATION TO ALLOW THE NEXT PERMISSIVE PHASE IN THE COORDINATION CYCLE TO BE SERVICED FOLLOWING PREEMPTION.

WIRING DIAGRAM

DISPLAYS DETECTORS



* INCLUDES 3C/14 AND ETHERNET CABLE AS PER MANUFACTURE'S RECOMMENDATION

** CABLE AS PER MANUFACTURE'S RECOMMENDATION

LEGEND

5C/14 - CABLE.
(NO. OF CONDUCTORS/SIZE AWG.)

- SIGNAL SUPPORT
- SIGNAL HEAD
- DETECTOR
- CONFIRMATION LIGHT
- PREEMPTION DETECTOR
- JUNCTION BOX
- PEDESTRIAN PUSH BUTTON
- VIDEO DETECTOR
- RADIO COMMUNICATION ANTENNA

REVISIONS ONLY

COMMONWEALTH OF PENNSYLVANIA REGISTERED PROFESSIONAL DANIEL RAY FRITZ ENGINEER NO. PE076640
10/16/2018

COMMONWEALTH OF PENNSYLVANIA REGISTERED PROFESSIONAL RAYMOND FRANCIS CARUSO ENGINEER No. 44106
FVC

COUNTY: ALLEGHENY
MUNICIPALITY: SOUTH FAYETTE TOWNSHIP
INTERSECTION: S.R. 0050 (MILLERS RUN RD.) AND NEWBURY DR. / MUNICIPAL DR.

APPROVED BY:
MUNICIPAL OFFICIAL — DATE 10-24-2018

RECOMMENDED:
DISTRICT TRAFFIC ENGINEER — DATE

PLOTTED: 9/17/2018 2:53:30 PM
OPERATOR: cbaker
FILE NAME: N:\35013-000\CADD\Traffic\2015 GLG 127 - South Fayette\Final Permit Plans\26-004-35013-TSP (SR 50 AT MUNICIPAL DR AND NEWBURY DR) REVISED 10-041 2013-TSP.dgn

APPENDIX E

Level of Service (LOS) Criteria Summary

LEVEL-OF-SERVICE CRITERIA
SIGNALIZED INTERSECTIONS

Level-of-Service for signalized intersections is defined in terms of delay. Delay is a measure of driver discomfort, frustration, fuel consumption, and lost travel time. Specifically, Level-of-Service criteria are stated in terms of the average stopped delay per vehicle for a 15-minute analysis period.

Level-of-Service A describes operations with very low delay, i.e., less than 10.0 seconds per vehicle.

Level-of-Service B describes operations with delay in the range of 10.1 to 20.0 seconds per vehicle.

Level-of-Service C describes operations with delay in the range of 20.1 to 35.0 seconds per vehicle.

Level-of-Service D describes operations with delay in the range of 35.1 to 55.0 seconds per vehicle.

Level-of-Service E describes operations with delay in the range of 55.1 to 80.0 seconds per vehicle. This is considered to be the limit of acceptable delay.

Level-of-Service F describes operations with delay in excess of 80.0 seconds per vehicle. This is considered to be unacceptable to most drivers.

UNSIGNALIZED INTERSECTIONS

AVERAGE TOTAL DELAY (sec/veh)	LEVEL OF SERVICE	EXPECTED DELAY TO MINOR STREET TRAFFIC
≤ 10	A	Little or no delay
>10 and $\leq$15	B	Short traffic delays
>15 and $\leq$25	C	Average traffic delays
>25 and $\leq$35	D	Long traffic delays
>35 and $\leq$50	E	Very long delays
>50	F	*

* -- When demand volume exceeds the capacity of the lane, extreme delays will be encountered with queuing which may cause severe congestion affecting other traffic movements in the intersection. This condition usually warrants improvements to the intersection.

APPENDIX F

Synchro Printouts – *Existing Year 2023 Condition*

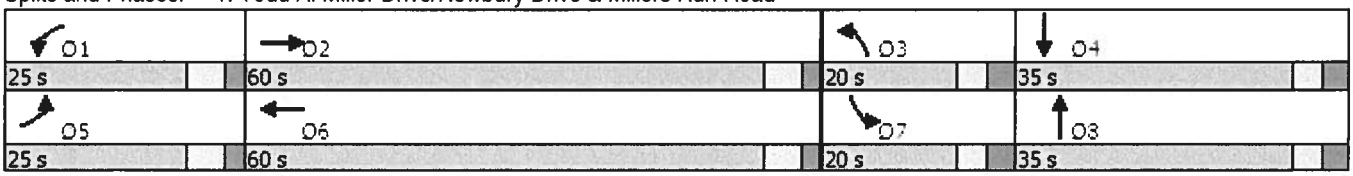
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	27	653	19	61	388	221	3	7	27	198	22	20
Future Volume (vph)	27	653	19	61	388	221	3	7	27	198	22	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.996				0.850			0.850		0.928	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3331	0	1778	3278	1613	1841	1700	1540	3416	1656	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3331	0	1778	3278	1613	1841	1700	1540	3416	1656	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2				243			117		22	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	30	718	21	67	426	243	3	8	30	218	24	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	30	739	0	67	426	243	3	8	30	218	46	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	6.8	24.8		8.0	31.2	31.2	5.8	5.9	5.9	9.9	12.8	
Actuated g/C Ratio	0.11	0.38		0.12	0.48	0.48	0.09	0.09	0.09	0.15	0.20	
v/c Ratio	0.18	0.58		0.30	0.27	0.27	0.02	0.05	0.12	0.42	0.13	
Control Delay	37.7	19.8		36.6	12.9	3.3	38.3	38.1	1.0	32.2	18.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	37.7	19.8		36.6	12.9	3.3	38.3	38.1	1.0	32.2	18.6	
LOS	D	B		D	B	A	D	D	A	C	B	
Approach Delay		20.5			11.9			11.0			29.8	
Approach LOS		C			B			B			C	
Queue Length 50th (ft)	13	139		28	47	0	1	3	0	46	8	
Queue Length 95th (ft)	44	226		76	118	42	11	19	0	94	42	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	554	2678		601	2635	1344	458	877	851	851	865	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.05	0.28		0.11	0.16	0.18	0.01	0.01	0.04	0.26	0.05	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 64.6
Natural Cycle: 55
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.58
Intersection Signal Delay: 18.1 Intersection LOS: B
Intersection Capacity Utilization 50.1% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



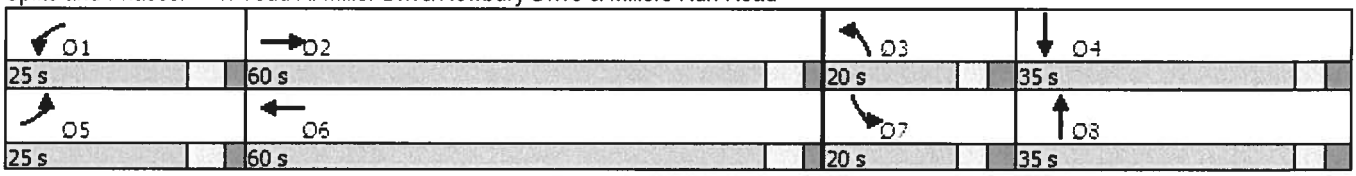
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	44	518	22	42	686	351	11	17	87	299	35	64
Future Volume (vph)	44	518	22	42	686	351	11	17	87	299	35	64
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850			0.850		0.903	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3420	0	1778	3405	1644	1841	1938	1599	3416	1657	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3420	0	1778	3405	1644	1841	1938	1599	3416	1657	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4				362			117		59	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	45	534	23	43	707	362	11	18	90	308	36	66
Shared Lane Traffic (%)												
Lane Group Flow (vph)	45	557	0	43	707	362	11	18	90	308	102	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.5	31.1		7.4	31.0	31.0	6.1	6.3	6.3	13.7	19.2	
Actuated g/C Ratio	0.10	0.42		0.10	0.42	0.42	0.08	0.08	0.08	0.18	0.26	
v/c Ratio	0.25	0.39		0.24	0.50	0.40	0.07	0.11	0.37	0.49	0.22	
Control Delay	44.2	16.5		44.2	18.0	3.2	45.3	45.1	9.6	36.6	17.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	44.2	16.5		44.2	18.0	3.2	45.3	45.1	9.6	36.6	17.8	
LOS	D	B		D	B	A	D	D	A	D	B	
Approach Delay		18.6			14.2			18.3			31.9	
Approach LOS		B			B			B			C	
Queue Length 50th (ft)	22	105		21	143	0	6	9	0	76	16	
Queue Length 95th (ft)	66	156		64	206	47	26	35	31	154	78	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	524	2559		531	2547	1321	405	885	793	753	788	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.09	0.22		0.08	0.28	0.27	0.03	0.02	0.11	0.41	0.13	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 74.4
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.50
Intersection Signal Delay: 18.8 Intersection LOS: B
Intersection Capacity Utilization 53.3% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



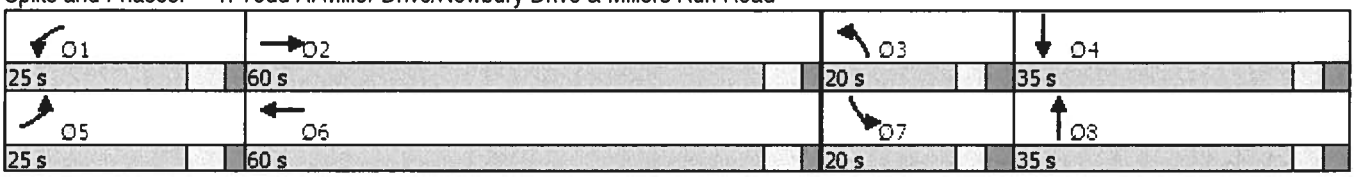
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	44	531	21	27	504	405	5	8	33	338	20	68
Future Volume (vph)	44	531	21	27	504	405	5	8	33	338	20	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850			0.850		0.884	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3478	0	1814	3438	1677	1841	1938	1647	3450	1671	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3478	0	1814	3438	1677	1841	1938	1647	3450	1671	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3				440			117		74	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	48	577	23	29	548	440	5	9	36	367	22	74
Shared Lane Traffic (%)												
Lane Group Flow (vph)	48	600	0	29	548	440	5	9	36	367	96	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.2	24.4		6.4	21.5	21.5	5.7	5.7	5.7	15.6	17.0	
Actuated g/C Ratio	0.11	0.39		0.10	0.34	0.34	0.09	0.09	0.09	0.25	0.27	
v/c Ratio	0.24	0.45		0.16	0.47	0.51	0.03	0.05	0.14	0.43	0.19	
Control Delay	36.1	16.5		36.8	19.3	4.3	37.8	37.5	1.2	27.8	11.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	36.1	16.5		36.8	19.3	4.3	37.8	37.5	1.2	27.8	11.6	
LOS	D	B		D	B	A	D	D	A	C	B	
Approach Delay		17.9			13.3			11.4			24.4	
Approach LOS		B			B			B			C	
Queue Length 50th (ft)	18	81		11	104	0	2	3	0	68	5	
Queue Length 95th (ft)	63	163		45	155	53	14	21	0	#162	57	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	588	2878		608	2844	1463	455	992	901	853	892	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.21		0.05	0.19	0.30	0.01	0.01	0.04	0.43	0.11	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 63.1
Natural Cycle: 55
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.51
Intersection Signal Delay: 17.0 Intersection LOS: B
Intersection Capacity Utilization 50.8% ICU Level of Service A
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



APPENDIX G

HCM Printouts – *Existing Year 2023 Condition*

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	27	653	19	61	388	221	3	7	27	198	22	20
Future Volume (veh/h)	27	653	19	61	388	221	3	7	27	198	22	20
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	30	718	21	67	426	0	3	8	0	218	24	7
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	60	1115	33	113	1202		8	19		353	148	43
Arrive On Green	0.03	0.32	0.32	0.06	0.34	0.00	0.00	0.01	0.00	0.10	0.11	0.11
Sat Flow, veh/h	1747	3514	103	1818	3485	1656	1959	1847	1654	3445	1354	395
Grp Volume(v), veh/h	30	362	377	67	426	0	3	8	0	218	0	31
Grp Sat Flow(s),veh/h/ln	1747	1771	1846	1818	1743	1656	1959	1847	1654	1722	0	1749
Q Serve(g_s), s	0.8	8.3	8.3	1.7	4.3	0.0	0.1	0.2	0.0	2.9	0.0	0.8
Cycle Q Clear(g_c), s	0.8	8.3	8.3	1.7	4.3	0.0	0.1	0.2	0.0	2.9	0.0	0.8
Prop In Lane	1.00		0.06	1.00		1.00	1.00		1.00	1.00		0.23
Lane Grp Cap(c), veh/h	60	562	586	113	1202		8	19		353	0	191
V/C Ratio(X)	0.50	0.64	0.64	0.60	0.35		0.37	0.41		0.62	0.00	0.16
Avail Cap(c_a), veh/h	702	2023	2108	731	3981		580	1133		1020	0	1073
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.4	13.8	13.8	21.6	11.6	0.0	23.5	23.2	0.0	20.3	0.0	19.1
Incr Delay (d2), s/veh	2.4	0.6	0.5	1.9	0.1	0.0	10.4	5.1	0.0	0.7	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.6	2.8	0.7	1.3	0.0	0.1	0.1	0.0	1.1	0.0	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.8	14.4	14.4	23.5	11.6	0.0	33.9	28.3	0.0	21.0	0.0	19.2
LnGrp LOS	C	B	B	C	B		C	C		C	A	B
Approach Vol, veh/h		769			493			11			249	
Approach Delay, s/veh		14.8			13.2			29.8			20.8	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	8.9	21.0	6.2	11.2	7.6	22.3	10.8	6.5
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	3.7	10.3	2.1	2.8	2.8	6.3	4.9	2.2
Green Ext Time (p_c), s	0.1	3.0	0.0	0.1	0.0	1.9	0.3	0.0

Intersection Summary	
HCM 6th Ctrl Delay	15.4
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	44	518	22	42	686	351	11	17	87	299	35	64
Future Volume (veh/h)	44	518	22	42	686	351	11	17	87	299	35	64
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	45	534	20	43	707	0	11	18	0	308	36	33
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	86	1101	41	82	1106		28	46		456	125	114
Arrive On Green	0.05	0.31	0.31	0.05	0.31	0.00	0.01	0.02	0.00	0.13	0.14	0.14
Sat Flow, veh/h	1847	3566	133	1818	3599	1683	1959	2057	1705	3445	889	814
Grp Volume(v), veh/h	45	271	283	43	707	0	11	18	0	308	0	69
Grp Sat Flow(s),veh/h/ln	1847	1814	1885	1818	1800	1683	1959	2057	1705	1722	0	1703
Q Serve(g_s), s	1.2	5.9	6.0	1.1	8.3	0.0	0.3	0.4	0.0	4.2	0.0	1.8
Cycle Q Clear(g_c), s	1.2	5.9	6.0	1.1	8.3	0.0	0.3	0.4	0.0	4.2	0.0	1.8
Prop In Lane	1.00		0.07	1.00		1.00	1.00		1.00	1.00		0.48
Lane Grp Cap(c), veh/h	86	560	582	82	1106		28	46		456	0	239
V/C Ratio(X)	0.52	0.48	0.49	0.52	0.64		0.40	0.39		0.68	0.00	0.29
Avail Cap(c_a), veh/h	719	2006	2085	708	3981		562	1222		988	0	1012
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.7	13.7	13.7	22.8	14.6	0.0	23.9	23.5	0.0	20.2	0.0	18.8
Incr Delay (d2), s/veh	1.8	0.3	0.3	1.9	0.3	0.0	3.4	2.0	0.0	0.7	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	2.0	2.0	0.5	2.7	0.0	0.1	0.2	0.0	1.6	0.0	0.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.5	14.0	14.0	24.7	14.9	0.0	27.2	25.6	0.0	20.8	0.0	19.1
LnGrp LOS	C	B	B	C	B		C	C		C	A	B
Approach Vol, veh/h		599			750			29			377	
Approach Delay, s/veh		14.8			15.4			26.2			20.5	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	8.2	21.1	6.7	12.8	8.3	21.0	12.5	7.1
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	3.1	8.0	2.3	3.8	3.2	10.3	6.2	2.4
Green Ext Time (p_c), s	0.0	2.1	0.0	0.2	0.0	3.4	0.4	0.0

Intersection Summary	
HCM 6th Ctrl Delay	16.5
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	44	531	21	27	504	405	5	8	33	338	20	68
Future Volume (veh/h)	44	531	21	27	504	405	5	8	33	338	20	68
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	48	577	19	29	548	0	5	9	0	367	22	36
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	90	1163	38	61	1102		13	24		519	100	163
Arrive On Green	0.05	0.32	0.32	0.03	0.30	0.00	0.01	0.01	0.00	0.15	0.15	0.15
Sat Flow, veh/h	1847	3640	120	1847	3628	1709	1959	2057	1743	3472	646	1058
Grp Volume(v), veh/h	48	292	304	29	548	0	5	9	0	367	0	58
Grp Sat Flow(s),veh/h/ln	1847	1842	1918	1847	1814	1709	1959	2057	1743	1736	0	1704
Q Serve(g_s), s	1.3	6.3	6.3	0.8	6.1	0.0	0.1	0.2	0.0	5.0	0.0	1.5
Cycle Q Clear(g_c), s	1.3	6.3	6.3	0.8	6.1	0.0	0.1	0.2	0.0	5.0	0.0	1.5
Prop In Lane	1.00		0.06	1.00		1.00	1.00		1.00	1.00		0.62
Lane Grp Cap(c), veh/h	90	588	613	61	1102		13	24		519	0	263
V/C Ratio(X)	0.53	0.50	0.50	0.47	0.50		0.38	0.37		0.71	0.00	0.22
Avail Cap(c_a), veh/h	711	2015	2097	711	3968		556	1208		985	0	1001
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.9	13.6	13.6	23.4	14.1	0.0	24.4	24.2	0.0	20.0	0.0	18.3
Incr Delay (d2), s/veh	1.8	0.3	0.3	2.1	0.2	0.0	6.6	3.5	0.0	0.7	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	2.1	2.2	0.3	2.0	0.0	0.1	0.1	0.0	1.9	0.0	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.7	13.9	13.9	25.5	14.3	0.0	31.0	27.7	0.0	20.6	0.0	18.4
LnGrp LOS	C	B	B	C	B		C	C		C	A	B
Approach Vol, veh/h		644			577			14			425	
Approach Delay, s/veh		14.7			14.8			28.9			20.3	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	7.6	21.8	6.3	13.6	8.4	21.0	13.4	6.6
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	2.8	8.3	2.1	3.5	3.3	8.1	7.0	2.2
Green Ext Time (p_c), s	0.0	2.3	0.0	0.2	0.0	2.5	0.5	0.0

Intersection Summary	
HCM 6th Ctrl Delay	16.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection						
Int Delay, s/veh	1.3					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	35	11	196	59	17	197
Future Vol, veh/h	35	11	196	59	17	197
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	0	5	0	0	2
Mvmt Flow	41	13	231	69	20	232

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	422	150	0	0	300	0
Stage 1	266	-	-	-	-	-
Stage 2	156	-	-	-	-	-
Critical Hdwy	6.86	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-
Follow-up Hdwy	3.53	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	557	876	-	-	1273	-
Stage 1	751	-	-	-	-	-
Stage 2	853	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	547	876	-	-	1273	-
Mov Cap-2 Maneuver	547	-	-	-	-	-
Stage 1	751	-	-	-	-	-
Stage 2	838	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.6	0	0.7
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	601	1273	-
HCM Lane V/C Ratio	-	-	0.09	0.016	-
HCM Control Delay (s)	-	-	11.6	7.9	0.1
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0	-

Intersection	
Int Delay, s/veh	1.6

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	57	28	331	62	13	334
Future Vol, veh/h	57	28	331	62	13	334
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	62	30	360	67	14	363

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	604	214	0	0	427	0
Stage 1	394	-	-	-	-	-
Stage 2	210	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	435	797	-	-	1143	-
Stage 1	656	-	-	-	-	-
Stage 2	811	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	428	797	-	-	1143	-
Mov Cap-2 Maneuver	428	-	-	-	-	-
Stage 1	656	-	-	-	-	-
Stage 2	799	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.7	0	0.4
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	505	1143	-
HCM Lane V/C Ratio	-	-	0.183	0.012	-
HCM Control Delay (s)	-	-	13.7	8.2	0.1
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.7	0	-

Intersection						
Int Delay, s/veh	3.1					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	107	25	366	106	20	315
Future Vol, veh/h	107	25	366	106	20	315
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	1	4	0	2	0	1
Mvmt Flow	126	29	431	125	24	371

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	728	278	0	0	556	0
Stage 1	494	-	-	-	-	-
Stage 2	234	-	-	-	-	-
Critical Hdwy	6.82	6.98	-	-	4.1	-
Critical Hdwy Stg 1	5.82	-	-	-	-	-
Critical Hdwy Stg 2	5.82	-	-	-	-	-
Follow-up Hdwy	3.51	3.34	-	-	2.2	-
Pot Cap-1 Maneuver	361	713	-	-	1025	-
Stage 1	582	-	-	-	-	-
Stage 2	786	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	351	713	-	-	1025	-
Mov Cap-2 Maneuver	351	-	-	-	-	-
Stage 1	582	-	-	-	-	-
Stage 2	763	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	20.3	0	0.6
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	388	1025	-
HCM Lane V/C Ratio	-	-	0.4	0.023	-
HCM Control Delay (s)	-	-	20.3	8.6	0.1
HCM Lane LOS	-	-	C	A	A
HCM 95th %tile Q(veh)	-	-	1.9	0.1	-

APPENDIX H

Source Data for Background Developments

From: Michael J. Haberman, P.E. <mhaberman@gatewayengineers.com>
Sent: Tuesday, October 3, 2023 2:54 PM
To: Josh Haydo <jhaydo@dewooster.com>
Cc: Mark Szewcow <mark.szewcow@gibson-thomas.com>; Paone, Talia <tpaone@pa.gov>; John M. Barrett <JBarrett@sftwp.com>; Eileen Botti <eileen.botti@gibson-thomas.com>; Simmons, Michael (PENNDOT) <MICSIMMONS@pa.gov>; Miller, Ed M. <edmille@pa.gov>; Siewe, Emmanuel <esiewe@pa.gov>; Fedio, Daniel <dafedio@pa.gov>; Jesse Nelson <jnelson@dewooster.com>; Suleiman Swai <sswai@dewooster.com>; Joseph M. Galbraith, P.E. <jgalbraith@gatewayengineers.com>
Subject: RE: [External] #4392 - Retail Development - Former Pro Bike + Run - TIS Scoping

Josh,

A formal study was not completed for the Raising Cane's. The developer's consultant submitted a trip generation addendum for the purposes of developing an updated amount for traffic impact fees. I've attached the original TIS for The Piazza development, the trip generation addendum for the Raising Cane's, and the amended site plan for The Piazza with the removal of the drive-in bank and inclusion of the Raising Cane's (note that the final size of the Raising Cane's is 4,250 s.f., not what is show on the plan). Note the following as it relates to The Piazza development:

- To date, the following uses within The Piazza are built/occupied:
  - 11,700 s.f. of high-turnover sit-down restaurants (building #1 and Building #4 on site plan)
  - 21,200 s.f. of shopping center (building #3 and part of building #5 on site plan)
  - 3,000 s.f. fast-food restaurant without drive-through (within building #5 on site plan)
- The following uses within The Piazza are not yet constructed/occupied:
  - 6,800 s.f. of high-turnover sit-down restaurant (building #2 on site plan)
  - 4,000 s.f. fast-food restaurant with drive-through (building #6 on site plan)
  - 4,250 s.f. fast-food restaurant with drive-through (Raising Cane's)

You'll need to use the above summary and attached reports to establish the additional approved background trips for your study associated with The Piazza.

As for the South Fayette Commons development (Dunkin Donuts and Washington Federal), I've attached the original approved TIS for that development (prepared by Wooster) and the trip generation for the Taco Bell (prepared by Wooster). To date, the only uses that have been constructed are the Dunkin Donuts and the Washington Federal Bank. The Taco Bell is approved and under construction. To simplify the trip generation and background trips associated with South Fayette Commons, I would recommend taking the difference between the total trips in the approved TIS and the estimated trips associated with the Dunkin Donuts and Washington Federal Bank and adding those to the study area. That way the trips associated with the Taco Bell and the remainder of the development will be included in the background for this study. If PennDOT and/or Gibson Thomas feel differently about that approach, I will defer to them.

There is no study for the Cigar Bar that I am aware of, so you'll have to estimate the trips associated with that land use as well.

Let me know if you want to discuss any of this in more detail or if you have any questions.

Thanks,
Mike

From: Mark Szewcow
Sent: Friday, September 15, 2023 11:31 AM
To: Paone, Talia <tpaone@pa.gov>; John M. Barrett <JBarrett@sftwp.com>; Eileen Botti <eileen.botti@gibson-thomas.com>; Josh Haydo <jhaydo@dewooster.com>; Simmons, Michael (PENNDOT) <MICSIMMONS@pa.gov>; Miller, Ed M. <edmille@pa.gov>; Siewe, Emmanuel <esiewe@pa.gov>; Fedio, Daniel <dafedio@pa.gov>
Subject: RE: [External] #4392 - Retail Development - Former Pro Bike + Run - TIS Scoping

Hello all,

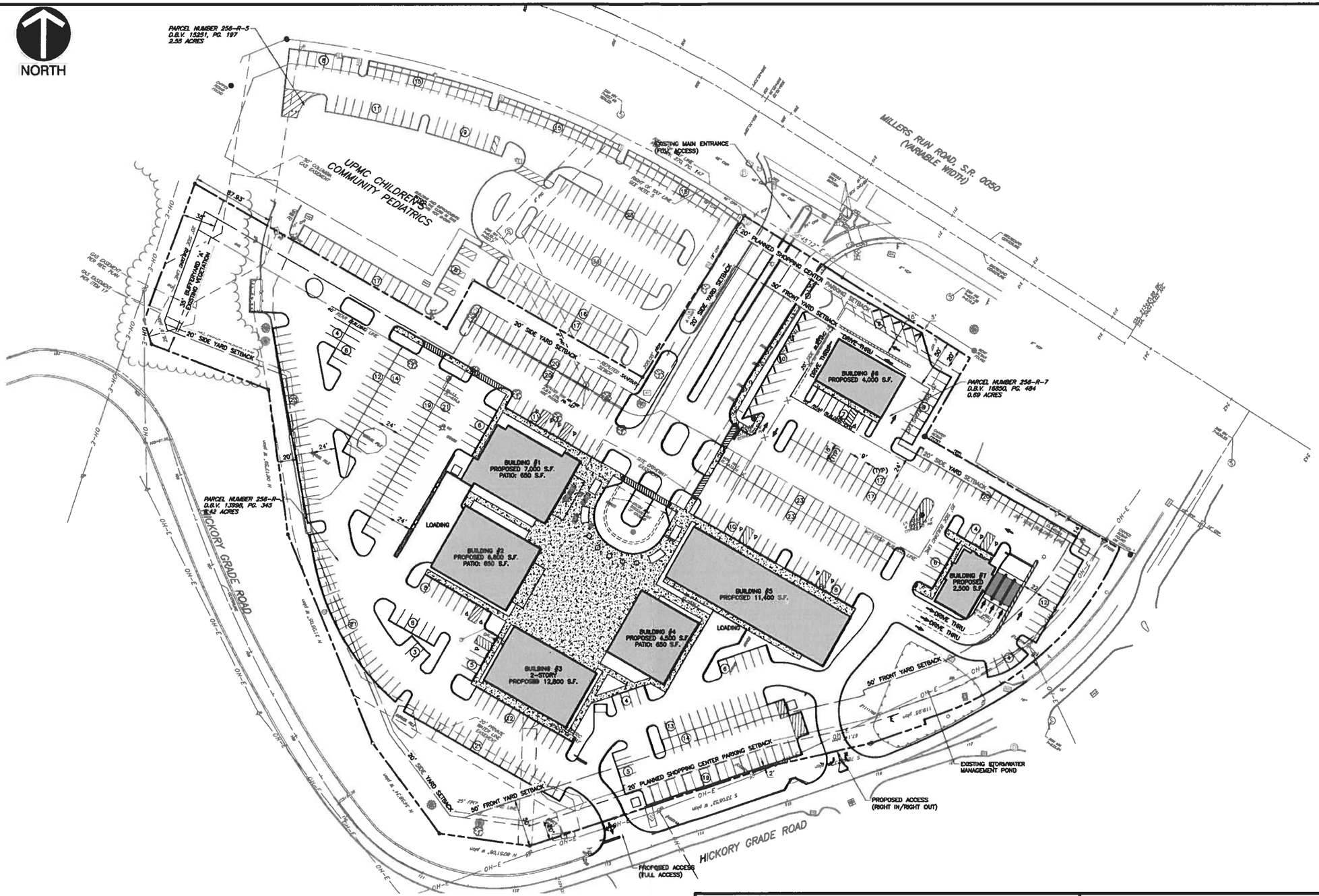
I wanted to follow up on some issues that were discussed at the meeting yesterday. After talking with John Barrett, here is a summary of the responses from the Township.

1.) The Township agrees to only add traffic from developments that are currently approved but not yet constructed in the Newberry Site. There is currently a 9,377 SF Cigar Bar and Restaurant approved to be constructed near Top Golf.
2.) The Township is agreeable to the intersections and time periods to be studied.

If anybody has any questions concerning this matter, please do not hesitate to call or email me.

Thanks

Mark Szewcow, PE
Phone: 724-539-8562
Cell: 412-417-7712
mark.szewcow@gibson-thomas.com



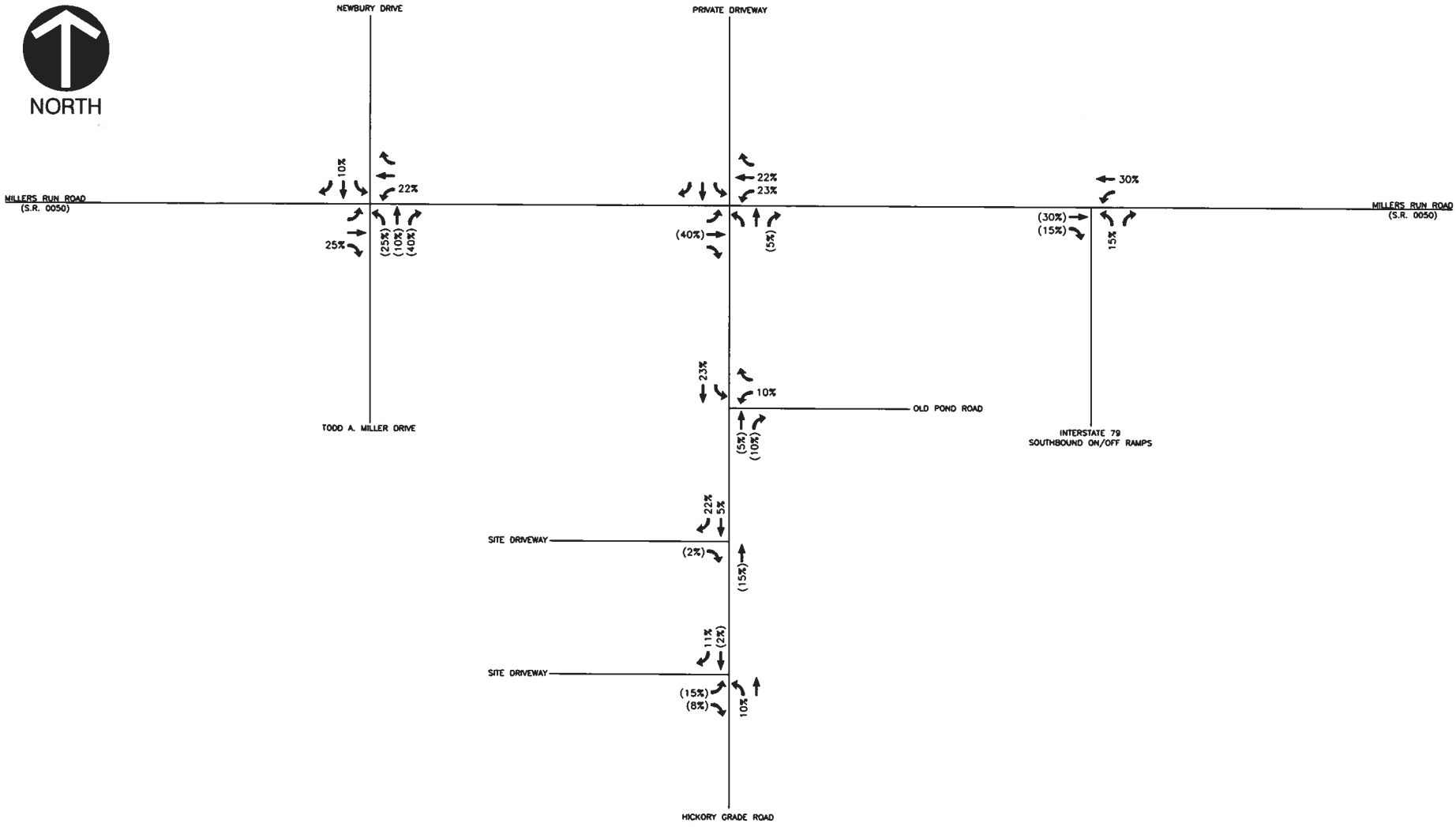
Civil & Environmental Consultants, Inc.
333 Baldwin Road · Pittsburgh, PA 15205
412-429-2324 · 800-365-2324
www.cecinc.com

THE PIAZZA RETAIL DEVELOPMENT
TRAFFIC IMPACT STUDY
SOUTH FAYETTE TOWNSHIP, ALLEGHENY COUNTY, PENNSYLVANIA

SITE PLAN

DRAWN BY:	QAB	CHECKED BY:	JRT	APPROVED BY:	JMD	FIGURE NO.:	2
DATE:	APRIL 2020	DWG SCALE:	NOT TO SCALE	PROJECT NO:	191-730		

P:\2019\191-730\-CADD\Dwg\TR02\191730-TR02-Figures-11x17.dwg[SITE PLAN] LS:(5/5/2020 - qberkey) - LP: 5/5/2020 12:27 PM



LEGEND

12% Arrival Trip Distribution

(12%) Departure Trip Distribution

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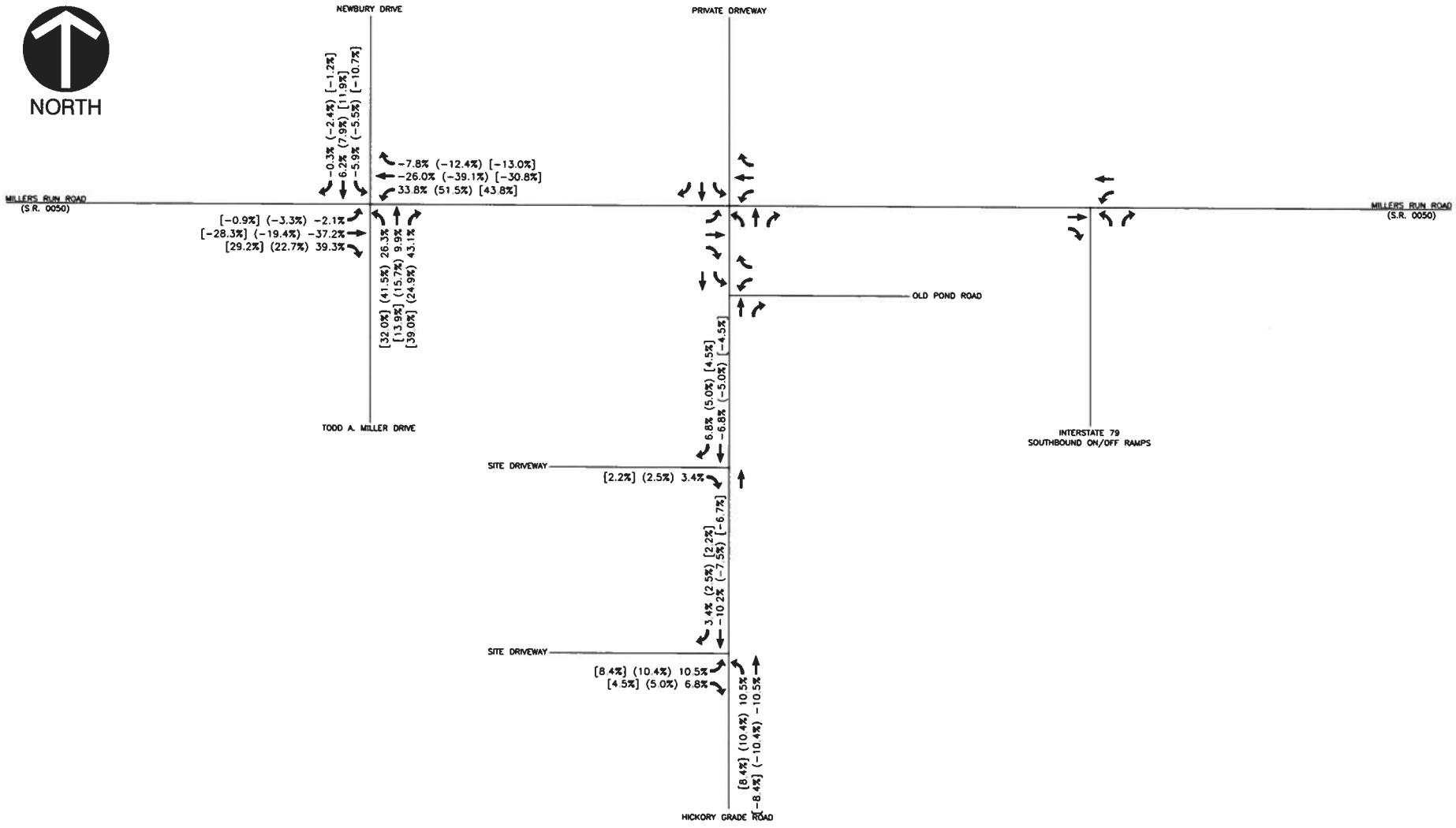
THE PIAZZA RETAIL DEVELOPMENT
TRAFFIC IMPACT STUDY
SOUTH FAYETTE TOWNSHIP,
ALLEGHENY COUNTY, PENNSYLVANIA

SITE GENERATED
PRIMARY TRIP DISTRIBUTION

DRAWN BY:	QAB	CHECKED BY:	JRT	APPROVED BY:	JMD	FIGURE NO.: 15
DATE:	APRIL 2020	DWG SCALE:	NOT TO SCALE	PROJECT NO:	191-730	



LEGEND

12%	A.M. Trip Distribution Percentage
(12%)	P.M. Trip Distribution Percentage
[12%]	Saturday Trip Distribution Percentage

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THE PIAZZA RETAIL DEVELOPMENT
TRAFFIC IMPACT STUDY
SOUTH FAYETTE TOWNSHIP,
ALLEGHENY COUNTY, PENNSYLVANIA

SITE GENERATED
PASS-BY TRIP DISTRIBUTION

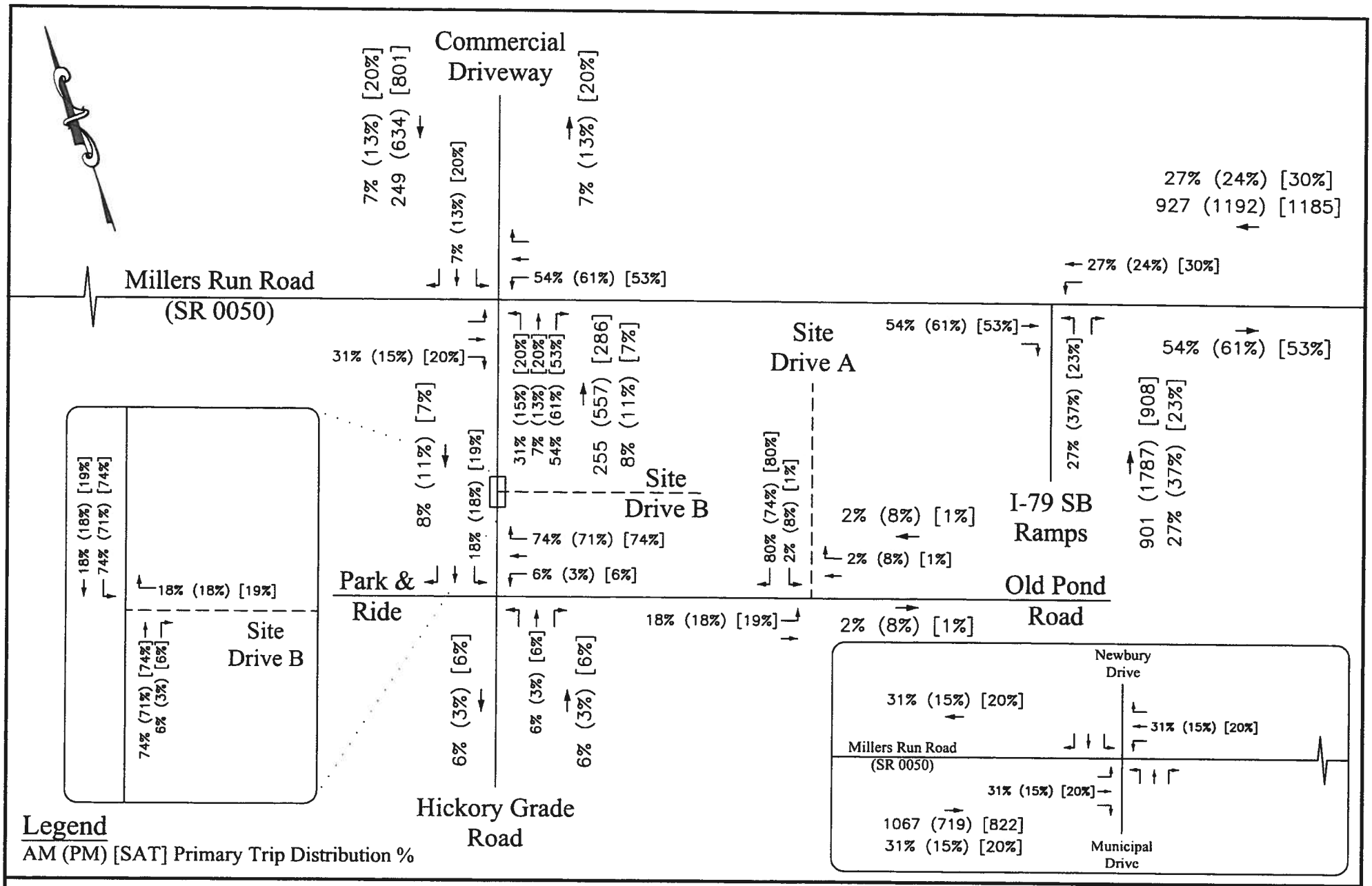
DRAWN BY:	QAB	CHECKED BY:	JRT	APPROVED BY:	JMD	FIGURE NO.:	16
DATE:	APRIL 2020	DWG SCALE:	NOT TO SCALE	PROJECT NO:	191-730		

P:\2019\191-730\-CADD\Dwg\TR02\191730-TR02-Figures-11x17.dwg[PASS BY TRIP DISTRIB.] LS:(5/5/2020 - qbertay) - LP: 5/5/2020 12:27 PM

Table 2

Trip Generation Summary

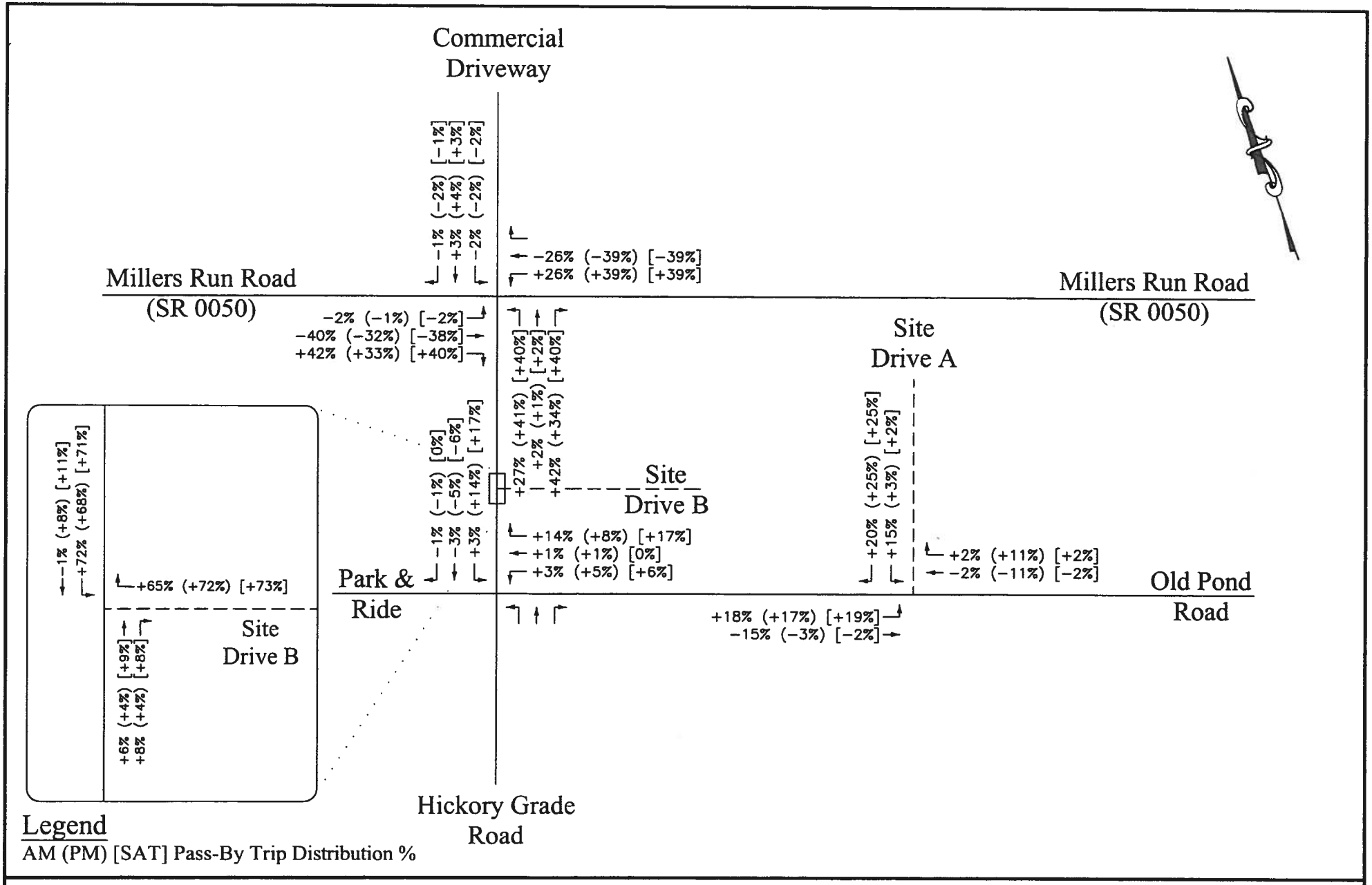
Time Period	Anticipated Trip Generation		
	In	Out	Total
LU Code #826 - Specialty Retail Center - 10,500 SF			
ADT	244	244	488
AM Peak Hour	0	0	0
Internal Trips (18%)	*0*	*0*	*0*
Primary Trips	*0*	*0*	*0*
Pass-By Trips (0%)	*0*	*0*	*0*
PM Peak Hour	21	26	47
Internal Trips (40%)	*8*	*10*	*18*
Primary Trips	*13*	*16*	*29*
Pass-By Trips (0%)	*0*	*0*	*0*
SAT Peak Hour	23	21	44
Internal Trips (53%)	*12*	*11*	*23*
Primary Trips	*11*	*10*	*21*
Pass-By Trips (0%)	*0*	*0*	*0*
LU Code #912 - Drive-in Bank - 3 drive-in lanes			
ADT	209	209	418
AM Peak Hour	17	11	28
Internal Trips (18%)	*3*	*2*	*5*
Primary Trips	*9*	*6*	*15*
Pass-By Trips (37%)	*5*	*3*	*8*
PM Peak Hour	49	51	100
Internal Trips (40%)	*20*	*20*	*40*
Primary Trips	*15*	*16*	*31*
Pass-By Trips (47%)	*14*	*15*	*29*
SAT Peak Hour	42	44	86
Internal Trips (53%)	*22*	*23*	*45*
Primary Trips	*13*	*13*	*26*
Pass-By Trips (37%)	*7*	*8*	*15*
LU Code #932 - High-Turnover (Sit-Down) Restaurant - 3,000 SF			
ADT	191	191	382
AM Peak Hour	18	14	32
Internal Trips (18%)	*3*	*3*	*6*
Primary Trips	*10*	*7*	*17*
Pass-By Trips (33%)	*5*	*4*	*9*
PM Peak Hour	18	12	30
Internal Trips (40%)	*7*	*5*	*12*
Primary Trips	*6*	*4*	*10*
Pass-By Trips (43%)	*5*	*3*	*8*
SAT Peak Hour	22	20	42
Internal Trips (53%)	*12*	*11*	*23*
Primary Trips	*7*	*6*	*13*
Pass-By Trips (33%)	*3*	*3*	*6*
LU Code #934 - Fast-Food Restaurant with Drive-Through Window - 2,800 SF			
ADT	695	695	1,390
AM Peak Hour	65	62	127
Internal Trips (18%)	*12*	*11*	*23*
Primary Trips	*27*	*26*	*53*
Pass-By Trips (49%)	*26*	*25*	*51*
PM Peak Hour	47	44	91
Internal Trips (40%)	*19*	*18*	*37*
Primary Trips	*14*	*13*	*27*
Pass-By Trips (50%)	*14*	*13*	*27*
SAT Peak Hour	84	81	165
Internal Trips (53%)	*45*	*43*	*88*
Primary Trips	*23*	*23*	*46*
Pass-By Trips (40%)	*16*	*15*	*31*
Total Trip Generation			
ADT	1,339	1,339	2,678
AM Peak Hour	100	87	187
Internal Trips	*18*	*16*	*34*
Primary Trips	*46*	*39*	*85*
Pass-By Trips	*36*	*32*	*68*
PM Peak Hour	135	133	268
Internal Trips	*54*	*53*	*107*
Primary Trips	*48*	*49*	*97*
Pass-By Trips	*33*	*31*	*64*
SAT Peak Hour	171	166	337
Internal Trips	*91*	*88*	*179*
Primary Trips	*54*	*52*	*106*
Pass-By Trips	*26*	*26*	*52*



PROPOSED COMMERCIAL DEVELOPMENT – South Fayette Township, PA
Primary Trip Distribution Percentages

DAVID E. WOOSTER & ASSOC., INC.

FIGURE 7a
2876051815



PROPOSED COMMERCIAL DEVELOPMENT – South Fayette Township, PA
Total Pass-By Trip Distribution Percentages

DAVID E. WOOSTER & ASSOC., INC.

FIGURE 10a
2876051815

APPENDIX I

Synchro Printouts – *Opening Year 2024 Without Development Condition*

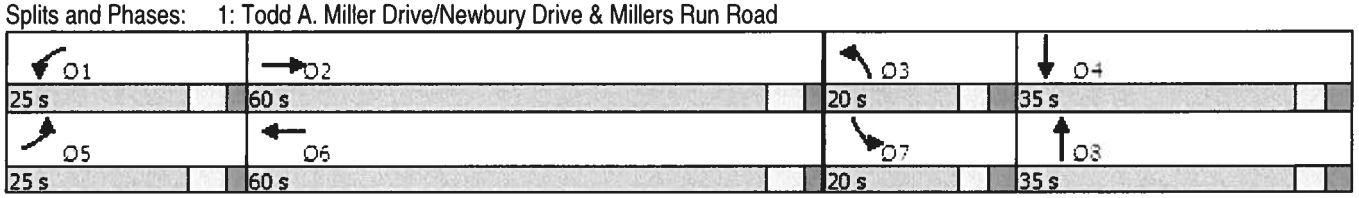
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	623	90	122	367	216	57	28	113	196	40	21
Future Volume (vph)	26	623	90	122	367	216	57	28	113	196	40	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.981				0.850			0.850		0.949	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3296	0	1778	3278	1613	1841	1700	1540	3416	1698	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3296	0	1778	3278	1613	1841	1700	1540	3416	1698	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13				237			124		17	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	29	685	99	134	403	237	63	31	124	215	44	23
Shared Lane Traffic (%)												
Lane Group Flow (vph)	29	784	0	134	403	237	63	31	124	215	67	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	6.5	28.9		11.5	39.7	39.7	7.8	6.8	6.8	10.3	8.7	
Actuated g/C Ratio	0.08	0.35		0.14	0.48	0.48	0.09	0.08	0.08	0.12	0.11	
v/c Ratio	0.22	0.68		0.54	0.26	0.26	0.36	0.22	0.52	0.51	0.35	
Control Delay	46.6	25.9		45.1	14.6	2.9	46.4	45.6	17.1	41.5	36.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	46.6	25.9		45.1	14.6	2.9	46.4	45.6	17.1	41.5	36.5	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		26.6			16.3			29.6			40.3	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	14	168		64	67	0	31	15	0	53	24	
Queue Length 95th (ft)	49	287		150	119	41	85	52	56	113	78	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	395	2263		428	2247	1180	327	625	645	607	635	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.07	0.35		0.31	0.18	0.20	0.19	0.05	0.19	0.35	0.11	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 82.7
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 25.0 Intersection LOS: C
Intersection Capacity Utilization 54.1% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



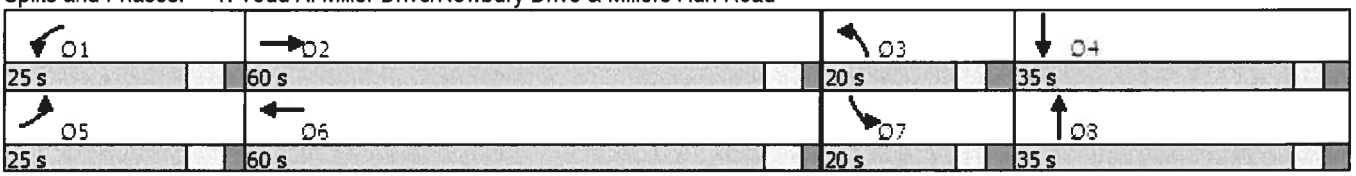
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	50	513	66	99	671	364	58	35	142	306	52	68
Future Volume (vph)	50	513	66	99	671	364	58	35	142	306	52	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.983				0.850			0.850		0.915	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12				375			146		42	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	52	529	68	102	692	375	60	36	146	315	54	70
Shared Lane Traffic (%)												
Lane Group Flow (vph)	52	597	0	102	692	375	60	36	146	315	124	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.6	28.6		10.0	30.7	30.7	7.8	7.2	7.2	13.5	11.3	
Actuated g/C Ratio	0.09	0.35		0.12	0.38	0.38	0.10	0.09	0.09	0.17	0.14	
v/c Ratio	0.32	0.50		0.47	0.54	0.44	0.34	0.21	0.53	0.56	0.46	
Control Delay	47.6	22.2		47.0	21.4	3.7	47.3	45.1	15.9	40.6	33.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.6	22.2		47.0	21.4	3.7	47.3	45.1	15.9	40.6	33.5	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		24.2			17.9			28.0			38.6	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	26	123		51	143	0	30	18	0	79	39	
Queue Length 95th (ft)	77	203		128	226	51	86	58	60	#178	118	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	448	2373		454	2382	1262	346	755	712	643	680	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.12	0.25		0.22	0.29	0.30	0.17	0.05	0.21	0.49	0.18	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 81.6
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.56
Intersection Signal Delay: 24.2 Intersection LOS: C
Intersection Capacity Utilization 53.1% ICU Level of Service A
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



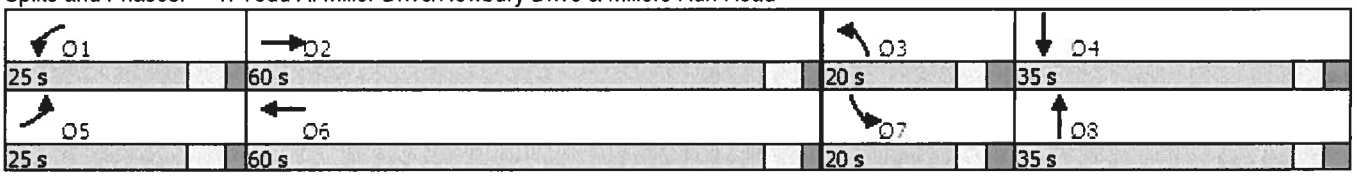
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	56	521	92	100	492	422	74	36	136	345	48	76
Future Volume (vph)	56	521	92	100	492	422	74	36	136	345	48	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.977				0.850			0.850		0.908	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3396	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3396	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				459			148		52	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	61	566	100	109	535	459	80	39	148	375	52	83
Shared Lane Traffic (%)												
Lane Group Flow (vph)	61	666	0	109	535	459	80	39	148	375	135	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.7	24.4		9.7	29.2	29.2	8.4	7.3	7.3	14.5	16.3	
Actuated g/C Ratio	0.10	0.30		0.12	0.36	0.36	0.10	0.09	0.09	0.18	0.20	
v/c Ratio	0.37	0.64		0.50	0.43	0.51	0.42	0.22	0.52	0.60	0.35	
Control Delay	45.1	26.7		44.7	21.4	4.2	44.9	41.4	14.6	38.7	26.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	45.1	26.7		44.7	21.4	4.2	44.9	41.4	14.6	38.7	26.0	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.2			16.5			27.6			35.3	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	28	140		49	105	0	36	18	0	84	35	
Queue Length 95th (ft)	83	237		127	179	60	101	57	58	#210	115	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	428	2360		442	2384	1303	331	722	706	620	672	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.14	0.28		0.25	0.22	0.35	0.24	0.05	0.21	0.60	0.20	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 80.8
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.64
Intersection Signal Delay: 24.6 Intersection LOS: C
Intersection Capacity Utilization 56.9% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



APPENDIX J

HCM Printouts – *Opening Year 2024 Without Development Condition*

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	26	623	90	122	367	216	57	28	113	196	40	21
Future Volume (veh/h)	26	623	90	122	367	216	57	28	113	196	40	21
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	29	685	99	134	403	0	63	31	0	215	44	8
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	58	935	135	176	1270		113	64		342	114	21
Arrive On Green	0.03	0.30	0.30	0.10	0.36	0.00	0.06	0.03	0.00	0.10	0.08	0.08
Sat Flow, veh/h	1747	3106	448	1818	3485	1656	1959	1847	1654	3445	1499	272
Grp Volume(v), veh/h	29	390	394	134	403	0	63	31	0	215	0	52
Grp Sat Flow(s),veh/h/ln	1747	1771	1784	1818	1743	1656	1959	1847	1654	1722	0	1771
Q Serve(g_s), s	0.8	10.1	10.1	3.7	4.3	0.0	1.6	0.8	0.0	3.1	0.0	1.4
Cycle Q Clear(g_c), s	0.8	10.1	10.1	3.7	4.3	0.0	1.6	0.8	0.0	3.1	0.0	1.4
Prop In Lane	1.00		0.25	1.00		1.00	1.00		1.00	1.00		0.15
Lane Grp Cap(c), veh/h	58	533	537	176	1270		113	64		342	0	135
V/C Ratio(X)	0.50	0.73	0.73	0.76	0.32		0.56	0.48		0.63	0.00	0.38
Avail Cap(c_a), veh/h	648	1867	1880	674	3673		535	1046		941	0	1002
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.4	16.1	16.1	22.6	11.7	0.0	23.5	24.3	0.0	22.2	0.0	22.5
Incr Delay (d2), s/veh	2.5	0.9	0.9	2.6	0.1	0.0	1.6	2.1	0.0	0.7	0.0	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	3.4	3.5	1.5	1.3	0.0	0.7	0.4	0.0	1.2	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.9	17.0	17.0	25.2	11.8	0.0	25.1	26.3	0.0	22.9	0.0	23.2
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		813			537			94			267	
Approach Delay, s/veh		17.3			15.1			25.5			22.9	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.9	21.4	9.0	9.9	7.7	24.7	11.1	7.8
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.7	12.1	3.6	3.4	2.8	6.3	5.1	2.8
Green Ext Time (p_c), s	0.1	3.3	0.0	0.1	0.0	1.8	0.3	0.1

Intersection Summary	
HCM 6th Ctrl Delay	18.0
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	50	513	66	99	671	364	58	35	142	306	52	68
Future Volume (veh/h)	50	513	66	99	671	364	58	35	142	306	52	68
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	52	529	65	102	692	0	60	36	0	315	54	37
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	94	944	116	135	1129		109	80		456	118	81
Arrive On Green	0.05	0.29	0.29	0.07	0.31	0.00	0.06	0.04	0.00	0.13	0.12	0.12
Sat Flow, veh/h	1847	3253	398	1818	3599	1683	1959	2057	1705	3445	1023	701
Grp Volume(v), veh/h	52	294	300	102	692	0	60	36	0	315	0	91
Grp Sat Flow(s),veh/h/ln	1847	1814	1838	1818	1800	1683	1959	2057	1705	1722	0	1724
Q Serve(g_s), s	1.4	7.1	7.2	2.8	8.4	0.0	1.5	0.9	0.0	4.5	0.0	2.5
Cycle Q Clear(g_c), s	1.4	7.1	7.2	2.8	8.4	0.0	1.5	0.9	0.0	4.5	0.0	2.5
Prop In Lane	1.00		0.22	1.00		1.00	1.00		1.00	1.00		0.41
Lane Grp Cap(c), veh/h	94	526	533	135	1129		109	80		456	0	199
V/C Ratio(X)	0.55	0.56	0.56	0.75	0.61		0.55	0.45		0.69	0.00	0.46
Avail Cap(c_a), veh/h	679	1894	1919	668	3759		531	1154		933	0	967
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.0	15.5	15.6	23.5	15.1	0.0	23.8	24.3	0.0	21.4	0.0	21.4
Incr Delay (d2), s/veh	1.9	0.4	0.4	3.2	0.2	0.0	1.6	1.5	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.5	2.5	1.2	2.8	0.0	0.7	0.4	0.0	1.7	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	25.8	16.0	16.0	26.7	15.3	0.0	25.4	25.7	0.0	22.1	0.0	22.0
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		646			794			96			406	
Approach Delay, s/veh		16.8			16.8			25.5			22.1	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	9.8	21.0	8.9	12.0	8.6	22.2	12.8	8.0
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	4.8	9.2	3.5	4.5	3.4	10.4	6.5	2.9
Green Ext Time (p_c), s	0.1	2.3	0.0	0.3	0.0	3.3	0.4	0.1

Intersection Summary	
HCM 6th Ctrl Delay	18.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	56	521	92	100	492	422	74	36	136	345	48	76
Future Volume (veh/h)	56	521	92	100	492	422	74	36	136	345	48	76
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	61	566	96	109	535	0	80	39	0	375	52	45
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	103	888	150	144	1102		128	85		516	117	101
Arrive On Green	0.06	0.28	0.28	0.08	0.30	0.00	0.07	0.04	0.00	0.15	0.12	0.12
Sat Flow, veh/h	1847	3153	533	1847	3628	1709	1959	2057	1743	3472	937	811
Grp Volume(v), veh/h	61	330	332	109	535	0	80	39	0	375	0	97
Grp Sat Flow(s),veh/h/ln	1847	1842	1843	1847	1814	1709	1959	2057	1743	1736	0	1748
Q Serve(g_s), s	1.7	8.3	8.4	3.1	6.4	0.0	2.1	1.0	0.0	5.5	0.0	2.7
Cycle Q Clear(g_c), s	1.7	8.3	8.4	3.1	6.4	0.0	2.1	1.0	0.0	5.5	0.0	2.7
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.46
Lane Grp Cap(c), veh/h	103	519	519	144	1102		128	85		516	0	218
V/C Ratio(X)	0.59	0.64	0.64	0.76	0.49		0.63	0.46		0.73	0.00	0.44
Avail Cap(c_a), veh/h	659	1868	1869	659	3679		515	1120		913	0	952
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	16.7	16.8	24.1	15.1	0.0	24.3	25.0	0.0	21.6	0.0	21.6
Incr Delay (d2), s/veh	2.0	0.6	0.6	3.1	0.2	0.0	1.9	1.4	0.0	0.7	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	3.0	3.0	1.3	2.2	0.0	1.0	0.5	0.0	2.1	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.6	17.3	17.4	27.1	15.3	0.0	26.1	26.4	0.0	22.4	0.0	22.1
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		723			644			119			472	
Approach Delay, s/veh		18.1			17.3			26.2			22.3	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.1	21.0	9.5	12.6	9.0	22.2	13.9	8.2
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.1	10.4	4.1	4.7	3.7	8.4	7.5	3.0
Green Ext Time (p_c), s	0.1	2.6	0.1	0.3	0.0	2.4	0.5	0.1

Intersection Summary	
HCM 6th Ctrl Delay	19.4
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection						
Int Delay, s/veh	1.3					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	35	11	211	59	17	215
Future Vol, veh/h	35	11	211	59	17	215
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	0	5	0	0	2
Mvmt Flow	41	13	248	69	20	253

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	450	159	0	0	317	0
Stage 1	283	-	-	-	-	-
Stage 2	167	-	-	-	-	-
Critical Hdwy	6.86	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-
Follow-up Hdwy	3.53	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	535	864	-	-	1255	-
Stage 1	737	-	-	-	-	-
Stage 2	842	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	525	864	-	-	1255	-
Mov Cap-2 Maneuver	525	-	-	-	-	-
Stage 1	737	-	-	-	-	-
Stage 2	826	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.9	0	0.7
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	579	1255	-
HCM Lane V/C Ratio	-	-	0.093	0.016	-
HCM Control Delay (s)	-	-	11.9	7.9	0.1
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0	-

Intersection						
Int Delay, s/veh	1.5					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	57	28	367	62	13	372
Future Vol, veh/h	57	28	367	62	13	372
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	62	30	399	67	14	404

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	663	233	0	0	466	0
Stage 1	433	-	-	-	-	-
Stage 2	230	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	399	775	-	-	1106	-
Stage 1	627	-	-	-	-	-
Stage 2	792	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	393	775	-	-	1106	-
Mov Cap-2 Maneuver	393	-	-	-	-	-
Stage 1	627	-	-	-	-	-
Stage 2	779	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	14.5	0	0.4
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	469	1106	-
HCM Lane V/C Ratio	-	-	0.197	0.013	-
HCM Control Delay (s)	-	-	14.5	8.3	0.1
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.7	0	-

Intersection						
Int Delay, s/veh	3.3					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	107	25	423	106	20	371
Future Vol, veh/h	107	25	423	106	20	371
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	1	4	0	2	0	1
Mvmt Flow	126	29	498	125	24	436

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	827	312	0	0	623	0
Stage 1	561	-	-	-	-	-
Stage 2	266	-	-	-	-	-
Critical Hdwy	6.82	6.98	-	-	4.1	-
Critical Hdwy Stg 1	5.82	-	-	-	-	-
Critical Hdwy Stg 2	5.82	-	-	-	-	-
Follow-up Hdwy	3.51	3.34	-	-	2.2	-
Pot Cap-1 Maneuver	312	678	-	-	968	-
Stage 1	538	-	-	-	-	-
Stage 2	757	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	302	678	-	-	968	-
Mov Cap-2 Maneuver	302	-	-	-	-	-
Stage 1	538	-	-	-	-	-
Stage 2	732	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	24.5	0	0.5
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	337	968	-
HCM Lane V/C Ratio	-	-	0.461	0.024	-
HCM Control Delay (s)	-	-	24.5	8.8	0.1
HCM Lane LOS	-	-	C	A	A
HCM 95th %tile Q(veh)	-	-	2.3	0.1	-

APPENDIX K

Synchro Printouts – *Design Year 2029 Without Development Condition*

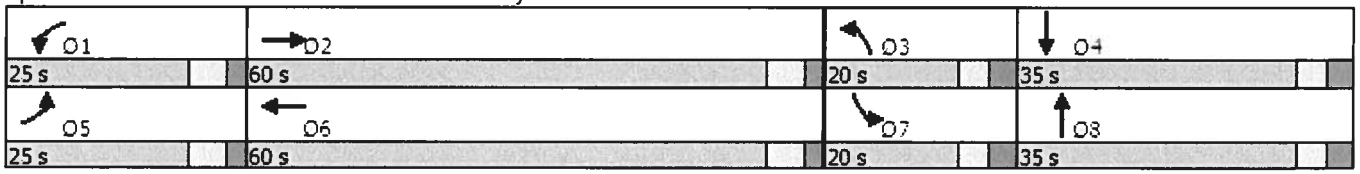
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	655	90	122	386	227	57	28	113	206	40	22
Future Volume (vph)	28	655	90	122	386	227	57	28	113	206	40	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.982				0.850			0.850		0.947	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12				249			124		18	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	31	720	99	134	424	249	63	31	124	226	44	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	819	0	134	424	249	63	31	124	226	68	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	6.7	30.9		11.7	41.7	41.7	7.9	6.8	6.8	10.7	9.0	
Actuated g/C Ratio	0.08	0.36		0.14	0.49	0.49	0.09	0.08	0.08	0.13	0.11	
v/c Ratio	0.24	0.68		0.55	0.26	0.27	0.37	0.23	0.52	0.53	0.35	
Control Delay	48.4	26.0		46.9	14.6	2.9	48.0	47.3	17.5	43.2	37.3	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	48.4	26.0		46.9	14.6	2.9	48.0	47.3	17.5	43.2	37.3	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		26.8			16.3			30.6			41.8	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	16	181		67	73	0	32	16	0	58	25	
Queue Length 95th (ft)	53	304		154	125	41	87	52	57	122	80	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	383	2198		416	2194	1162	317	607	629	589	616	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.37		0.32	0.19	0.21	0.20	0.05	0.20	0.38	0.11	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 85.3
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 25.3 Intersection LOS: C
Intersection Capacity Utilization 55.3% ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



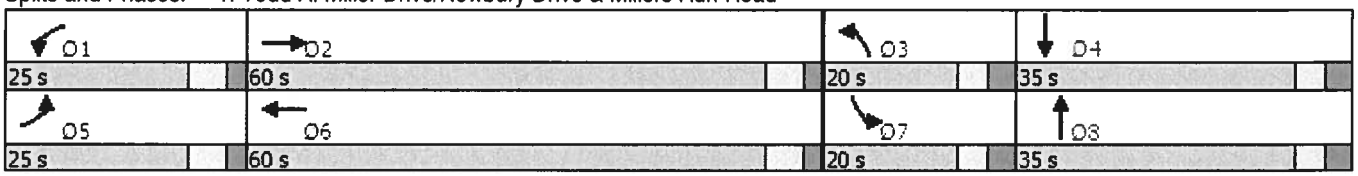
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	53	539	66	99	705	381	58	35	142	321	52	71
Future Volume (vph)	53	539	66	99	705	381	58	35	142	321	52	71
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.984				0.850			0.850		0.914	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3391	0	1778	3405	1644	1841	1938	1599	3416	1678	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3391	0	1778	3405	1644	1841	1938	1599	3416	1678	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11				393			146		44	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	55	556	68	102	727	393	60	36	146	331	54	73
Shared Lane Traffic (%)												
Lane Group Flow (vph)	55	624	0	102	727	393	60	36	146	331	127	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.8	32.3		10.1	34.3	34.3	7.9	7.3	7.3	14.4	11.5	
Actuated g/C Ratio	0.09	0.38		0.12	0.40	0.40	0.09	0.08	0.08	0.17	0.13	
v/c Ratio	0.35	0.49		0.49	0.54	0.44	0.36	0.22	0.54	0.58	0.48	
Control Delay	49.7	21.7		49.7	21.2	3.5	49.5	46.6	16.2	42.8	34.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	49.7	21.7		49.7	21.2	3.5	49.5	46.6	16.2	42.8	34.7	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		24.0			17.9			29.0			40.5	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	29	131		54	154	0	32	19	0	88	42	
Queue Length 95th (ft)	81	215		128	243	52	87	58	61	#194	120	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	420	2255		426	2260	1223	325	709	678	603	642	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.13	0.28		0.24	0.32	0.32	0.18	0.05	0.22	0.55	0.20	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 86
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.58
Intersection Signal Delay: 24.5 Intersection LOS: C
Intersection Capacity Utilization 57.0% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



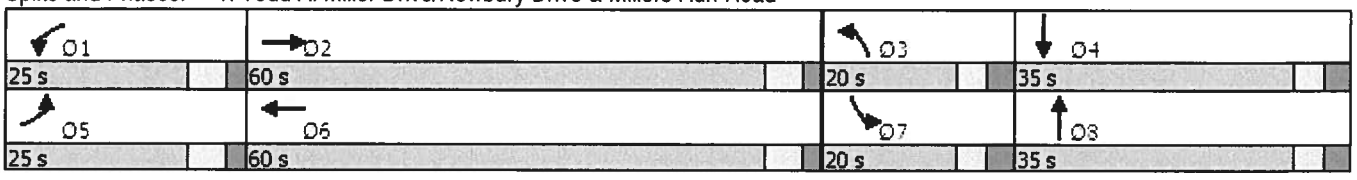
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	59	548	92	100	517	442	74	36	136	362	48	79
Future Volume (vph)	59	548	92	100	517	442	74	36	136	362	48	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.978				0.850			0.850		0.907	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3401	0	1814	3438	1677	1841	1938	1647	3450	1715	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3401	0	1814	3438	1677	1841	1938	1647	3450	1715	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		16				480			148		54	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	64	596	100	109	562	480	80	39	148	393	52	86
Shared Lane Traffic (%)												
Lane Group Flow (vph)	64	696	0	109	562	480	80	39	148	393	138	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	8.0	26.2		10.0	31.1	31.1	8.5	7.5	7.5	14.6	16.5	
Actuated g/C Ratio	0.10	0.31		0.12	0.37	0.37	0.10	0.09	0.09	0.18	0.20	
v/c Ratio	0.38	0.65		0.50	0.44	0.52	0.43	0.23	0.52	0.65	0.36	
Control Delay	47.3	26.6		46.5	21.4	4.1	46.9	43.2	14.8	41.4	27.1	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.3	26.6		46.5	21.4	4.1	46.9	43.2	14.8	41.4	27.1	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.3			16.5			28.6			37.7	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	30	148		50	112	0	37	18	0	92	37	
Queue Length 95th (ft)	89	254		134	192	60	106	59	59	#246	122	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	418	2309		432	2329	1290	323	705	693	605	658	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.15	0.30		0.25	0.24	0.37	0.25	0.06	0.21	0.65	0.21	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 83.4
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.65
Intersection Signal Delay: 25.2 Intersection LOS: C
Intersection Capacity Utilization 58.1% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



APPENDIX L

HCM Printouts – *Design Year 2029 Without Development Condition*

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	655	90	122	386	227	57	28	113	206	40	22
Future Volume (veh/h)	28	655	90	122	386	227	57	28	113	206	40	22
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	31	720	99	134	424	0	63	31	0	226	44	9
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	60	970	133	175	1296		112	64		353	117	24
Arrive On Green	0.03	0.31	0.31	0.10	0.37	0.00	0.06	0.03	0.00	0.10	0.08	0.08
Sat Flow, veh/h	1747	3128	430	1818	3485	1656	1959	1847	1654	3445	1466	300
Grp Volume(v), veh/h	31	407	412	134	424	0	63	31	0	226	0	53
Grp Sat Flow(s),veh/h/ln	1747	1771	1787	1818	1743	1656	1959	1847	1654	1722	0	1766
Q Serve(g_s), s	0.9	10.8	10.9	3.8	4.6	0.0	1.6	0.9	0.0	3.3	0.0	1.5
Cycle Q Clear(g_c), s	0.9	10.8	10.9	3.8	4.6	0.0	1.6	0.9	0.0	3.3	0.0	1.5
Prop In Lane	1.00		0.24	1.00		1.00	1.00		1.00	1.00		0.17
Lane Grp Cap(c), veh/h	60	549	554	175	1296		112	64		353	0	141
V/C Ratio(X)	0.51	0.74	0.74	0.76	0.33		0.56	0.48		0.64	0.00	0.38
Avail Cap(c_a), veh/h	631	1818	1835	657	3578		522	1019		917	0	974
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.9	16.3	16.3	23.2	11.8	0.0	24.2	24.9	0.0	22.7	0.0	23.0
Incr Delay (d2), s/veh	2.5	0.9	0.9	2.6	0.1	0.0	1.6	2.1	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	3.7	3.7	1.6	1.4	0.0	0.8	0.4	0.0	1.3	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.4	17.2	17.2	25.8	11.9	0.0	25.8	27.0	0.0	23.4	0.0	23.6
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		850			558			94			279	
Approach Delay, s/veh		17.6			15.2			26.2			23.4	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	11.1	22.3	9.0	10.2	7.8	25.6	11.4	7.8
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.8	12.9	3.6	3.5	2.9	6.6	5.3	2.9
Green Ext Time (p_c), s	0.1	3.5	0.0	0.1	0.0	1.9	0.3	0.1

Intersection Summary	
HCM 6th Ctrl Delay	18.2
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	53	539	66	99	705	381	58	35	142	321	52	71
Future Volume (veh/h)	53	539	66	99	705	381	58	35	142	321	52	71
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	55	556	65	102	727	0	60	36	0	331	54	40
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	97	944	110	135	1115		109	80		472	119	88
Arrive On Green	0.05	0.29	0.29	0.07	0.31	0.00	0.06	0.04	0.00	0.14	0.12	0.12
Sat Flow, veh/h	1847	3273	382	1818	3599	1683	1959	2057	1705	3445	987	731
Grp Volume(v), veh/h	55	307	314	102	727	0	60	36	0	331	0	94
Grp Sat Flow(s),veh/h/ln	1847	1814	1841	1818	1800	1683	1959	2057	1705	1722	0	1718
Q Serve(g_s), s	1.5	7.6	7.6	2.9	9.1	0.0	1.6	0.9	0.0	4.8	0.0	2.6
Cycle Q Clear(g_c), s	1.5	7.6	7.6	2.9	9.1	0.0	1.6	0.9	0.0	4.8	0.0	2.6
Prop In Lane	1.00		0.21	1.00		1.00	1.00		1.00	1.00		0.43
Lane Grp Cap(c), veh/h	97	523	531	135	1115		109	80		472	0	207
V/C Ratio(X)	0.57	0.59	0.59	0.76	0.65		0.55	0.45		0.70	0.00	0.45
Avail Cap(c_a), veh/h	675	1883	1911	664	3737		527	1147		927	0	958
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.1	15.9	15.9	23.6	15.5	0.0	23.9	24.4	0.0	21.4	0.0	21.3
Incr Delay (d2), s/veh	1.9	0.5	0.5	3.2	0.3	0.0	1.6	1.5	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.6	2.7	1.2	3.1	0.0	0.7	0.4	0.0	1.8	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.0	16.3	16.4	26.9	15.8	0.0	25.5	25.9	0.0	22.1	0.0	21.9
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		676			829			96			425	
Approach Delay, s/veh		17.1			17.2			25.7			22.1	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	9.9	21.0	8.9	12.3	8.7	22.1	13.1	8.0
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	4.9	9.6	3.6	4.6	3.5	11.1	6.8	2.9
Green Ext Time (p_c), s	0.1	2.4	0.0	0.3	0.0	3.5	0.4	0.1

Intersection Summary	
HCM 6th Ctrl Delay	18.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	59	548	92	100	517	442	74	36	136	362	48	79
Future Volume (veh/h)	59	548	92	100	517	442	74	36	136	362	48	79
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	64	596	96	109	562	0	80	39	0	393	52	48
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	106	889	143	144	1089		127	85		534	118	109
Arrive On Green	0.06	0.28	0.28	0.08	0.30	0.00	0.06	0.04	0.00	0.15	0.13	0.13
Sat Flow, veh/h	1847	3179	511	1847	3628	1709	1959	2057	1743	3472	907	837
Grp Volume(v), veh/h	64	345	347	109	562	0	80	39	0	393	0	100
Grp Sat Flow(s),veh/h/ln	1847	1842	1847	1847	1814	1709	1959	2057	1743	1736	0	1743
Q Serve(g_s), s	1.8	8.9	8.9	3.1	6.9	0.0	2.1	1.0	0.0	5.8	0.0	2.8
Cycle Q Clear(g_c), s	1.8	8.9	8.9	3.1	6.9	0.0	2.1	1.0	0.0	5.8	0.0	2.8
Prop In Lane	1.00		0.28	1.00		1.00	1.00		1.00	1.00		0.48
Lane Grp Cap(c), veh/h	106	515	517	144	1089		127	85		534	0	226
V/C Ratio(X)	0.60	0.67	0.67	0.76	0.52		0.63	0.46		0.74	0.00	0.44
Avail Cap(c_a), veh/h	655	1856	1861	655	3654		512	1113		907	0	943
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.7	17.1	17.1	24.2	15.5	0.0	24.4	25.1	0.0	21.7	0.0	21.5
Incr Delay (d2), s/veh	2.1	0.7	0.7	3.1	0.2	0.0	1.9	1.5	0.0	0.8	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	3.2	3.3	1.3	2.4	0.0	1.0	0.5	0.0	2.3	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.7	17.8	17.8	27.3	15.7	0.0	26.3	26.6	0.0	22.4	0.0	22.0
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		756			671			119			493	
Approach Delay, s/veh		18.6			17.6			26.4			22.3	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.2	21.0	9.5	13.0	9.1	22.1	14.2	8.2
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.1	10.9	4.1	4.8	3.8	8.9	7.8	3.0
Green Ext Time (p_c), s	0.1	2.8	0.1	0.3	0.0	2.6	0.5	0.1

Intersection Summary	
HCM 6th Ctrl Delay	19.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection	
Int Delay, s/veh	1.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	35	11	221	59	17	225
Future Vol, veh/h	35	11	221	59	17	225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	0	5	0	0	2
Mvmt Flow	41	13	260	69	20	265

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	468	165	0	0	329	0
Stage 1	295	-	-	-	-	-
Stage 2	173	-	-	-	-	-
Critical Hdwy	6.86	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-
Follow-up Hdwy	3.53	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	521	857	-	-	1242	-
Stage 1	727	-	-	-	-	-
Stage 2	837	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	511	857	-	-	1242	-
Mov Cap-2 Maneuver	511	-	-	-	-	-
Stage 1	727	-	-	-	-	-
Stage 2	821	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12	0	0.7
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	566	1242	-
HCM Lane V/C Ratio	-	-	0.096	0.016	-
HCM Control Delay (s)	-	-	12	7.9	0.1
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0	-

Intersection						
Int Delay, s/veh	1.5					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	57	28	384	62	13	389
Future Vol, veh/h	57	28	384	62	13	389
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	62	30	417	67	14	423

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	691	242	0	0	484	0
Stage 1	451	-	-	-	-	-
Stage 2	240	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	383	765	-	-	1089	-
Stage 1	614	-	-	-	-	-
Stage 2	783	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	376	765	-	-	1089	-
Mov Cap-2 Maneuver	376	-	-	-	-	-
Stage 1	614	-	-	-	-	-
Stage 2	770	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	15	0	0.4
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	452	1089	-
HCM Lane V/C Ratio	-	-	0.204	0.013	-
HCM Control Delay (s)	-	-	15	8.3	0.1
HCM Lane LOS	-	-	C	A	A
HCM 95th %tile Q(veh)	-	-	0.8	0	-

Intersection	
Int Delay, s/veh	3.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	107	25	441	106	20	387
Future Vol, veh/h	107	25	441	106	20	387
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	1	4	0	2	0	1
Mvmt Flow	126	29	519	125	24	455

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	858	322	0	0	644	0
Stage 1	582	-	-	-	-	-
Stage 2	276	-	-	-	-	-
Critical Hdwy	6.82	6.98	-	-	4.1	-
Critical Hdwy Stg 1	5.82	-	-	-	-	-
Critical Hdwy Stg 2	5.82	-	-	-	-	-
Follow-up Hdwy	3.51	3.34	-	-	2.2	-
Pot Cap-1 Maneuver	298	668	-	-	951	-
Stage 1	525	-	-	-	-	-
Stage 2	749	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	288	668	-	-	951	-
Mov Cap-2 Maneuver	288	-	-	-	-	-
Stage 1	525	-	-	-	-	-
Stage 2	724	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	26	0	0.5
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	323	951	-
HCM Lane V/C Ratio	-	-	0.481	0.025	-
HCM Control Delay (s)	-	-	26	8.9	0.1
HCM Lane LOS	-	-	D	A	A
HCM 95th %tile Q(veh)	-	-	2.5	0.1	-

APPENDIX M

Trip Generation Calculations

South Fayette Commons
Trip Generation Summary

Time Period	Anticipated Trip Generation		
	In	Out	Total
LU Code #826 - Specialty Retail Center - 10,500 SF			
ADT	**244**	**244**	**488**
AM Peak Hour	**0**	**0**	**0**
Internal Trips (18%)	*0*	*0*	*0*
Primary Trips	*0*	*0*	*0*
Pass-By Trips (0%)	*0*	*0*	*0*
PM Peak Hour	**21**	**26**	**47**
Internal Trips (40%)	*8*	*10*	*18*
Primary Trips	*13*	*16*	*29*
Pass-By Trips (0%)	*0*	*0*	*0*
SAT Peak Hour	**23**	**21**	**44**
Internal Trips (53%)	*12*	*11*	*23*
Primary Trips	*11*	*10*	*21*
Pass-By Trips (0%)	*0*	*0*	*0*
LU Code #932 - High-Turnover (Sit-Down) Restaurant - 3,000 SF			
ADT	**191**	**191**	**382**
AM Peak Hour	**18**	**14**	**32**
Internal Trips (18%)	*3*	*3*	*6*
Primary Trips	*10*	*7*	*17*
Pass-By Trips (33%)	*5*	*4*	*9*
PM Peak Hour	**18**	**12**	**30**
Internal Trips (40%)	*7*	*5*	*12*
Primary Trips	*6*	*4*	*10*
Pass-By Trips (43%)	*5*	*3*	*8*
SAT Peak Hour	**22**	**20**	**42**
Internal Trips (53%)	*12*	*11*	*23*
Primary Trips	*7*	*6*	*13*
Pass-By Trips (33%)	*3*	*3*	*6*
Remaining Development Trips			
ADT	**435**	**435**	**870**
AM Peak Hour	**18**	**14**	**32**
Internal Trips	*3*	*3*	*6*
Primary Trips	*10*	*7*	*17*
Pass-By Trips	*5*	*4*	*9*
PM Peak Hour	**39**	**38**	**77**
Internal Trips	*15*	*15*	*30*
Primary Trips	*19*	*20*	*39*
Pass-By Trips	*5*	*3*	*8*
SAT Peak Hour	**45**	**41**	**86**
Internal Trips	*24*	*22*	*46*
Primary Trips	*18*	*16*	*34*
Pass-By Trips	*3*	*3*	*6*

The Piazza Development
Trip Generation Summary

Time Period	Anticipated Trip Generation		
	In	Out	Total
LU Code #932 - *High-Turnover (Sit-Down) Restaurant* - 6,800 SF			
ADT	**365**	**365**	**730**
AM Peak Hour	**36**	**29**	**65**
Primary Trips	*24*	*19*	*43*
Pass-By Trips (33%)	*12*	*10*	*22*
PM Peak Hour	**38**	**24**	**62**
Primary Trips	*22*	*14*	*36*
Pass-By Trips (43%)	*16*	*10*	*26*
SAT Peak Hour	**39**	**37**	**76**
Primary Trips	*26*	*25*	*51*
Pass-By Trips (33%)	*13*	*12*	*25*
LU Code #934 - *Fast-Food Restaurant with Drive-Through Window* - 8,250 SF			
ADT	**1,929**	**1,929**	**250**
AM Peak Hour	**188**	**180**	**368**
Primary Trips	*96*	*92*	*188*
Pass-By Trips (49%)	*92*	*88*	*180*
PM Peak Hour	**141**	**131**	**272**
Primary Trips	*97*	*90*	*187*
Pass-By Trips (31%)	*44*	*41*	*85*
SAT Peak Hour	**233**	**223**	**456**
Primary Trips	*184*	*176*	*360*
Pass-By Trips (21%)	*49*	*47*	*96*
Total Trip Generation			
ADT	**2,294**	**2,294**	**980**
AM Peak Hour	**224**	**209**	**433**
Primary Trips	*120*	*111*	*231*
Pass-By Trips	*104*	*98*	*202*
PM Peak Hour	**179**	**155**	**334**
Primary Trips	*119*	*104*	*223*
Pass-By Trips	*60*	*51*	*111*
SAT Peak Hour	**272**	**260**	**532**
Primary Trips	*210*	*201*	*411*
Pass-By Trips	*62*	*59*	*121*

TRIP GENERATION

David E. Wooster and Associates, LLC
2 East Crafton Avenue
Pittsburgh, PA 15205

County : Allegheny County
Municipality : South Fayette Township
Client Code: #4392

Land Use Code: 821
Description: Shopping Plaza (40-150k) without Supermarket

SF Gross Leasable Area: 45,126
X = 1,000 Square Feet Gross Leasable Area

Page: 212 ADT

Equation: Not Given *Average Rate = 67.52*

T =	3,048 ADT	
50% entering =		1,524
50% exiting =		1,524

Page: 213 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given *Average Rate = 1.73*

T =	78 AM Peak	
62% entering =		48
38% exiting =		30

Table E.9

Primary	Pass-by (30%)*
34	14
21	9

Page: 214 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given *Average Rate = 5.19*

T =	234 PM Peak	
49% entering =		115
51% exiting =		119

Table E.9

Primary	Pass-by (40%)
69	46
71	48

Page: 218 Saturday (Peak hour of generator)

Equation: T = 7.75 (X) - 98.93 R^2 = 0.58

T =	251 SAT Peak	
52% entering =		131
48% exiting =		120

Table E.10

Primary	Pass-by (31%)
90	41
83	37

* PM peak hour minus 10%

TRIP GENERATION

David E. Wooster and Associates, LLC
2 East Crafton Avenue
Pittsburgh, PA 15205

County : Allegheny County
Municipality : South Fayette Township
Client Code: #4392

Land Use Code: 932
Description: High-Turnover (Sit-Down) Restaurant

SF Gross Floor Area: 6,800

PIAZZA BACKGROUND TRIPS

X = 1,000 Square Feet Gross Floor Area

Page: 673 **ADT**

Equation: Not Given *Average Rate = 107.20*

T = 730 ADT

50% entering =	365
50% exiting =	365

Page: 674 **AM Peak Hour (adjacent street between 7 and 9 a.m.)**

Equation: Not Given *Average Rate = 9.57*

T = 65 AM Peak

		Table E.30	
		Primary	Pass-by (33%)*
55% entering =	36	24	12
45% exiting =	29	19	10

Page: 675 **PM Peak Hour (adjacent street between 4 and 6 p.m.)**

Equation: Not Given *Average Rate = 9.05*

T = 62 PM Peak

		Table E.30	
		Primary	Pass-by (43%)
61% entering =	38	22	16
39% exiting =	24	14	10

Page: 682 **Saturday (peak hour of generator)**

Equation: Not Given *Average Rate = 11.19*

T = 76 SAT Peak

		Table E.30	
		Primary	Pass-by (33%)*
51% entering =	39	26	13
49% exiting =	37	25	12

* PM peak hour minus 10%

TRIP GENERATION

David E. Wooster and Associates, LLC
2 East Crafton Avenue
Pittsburgh, PA 15205

County : Allegheny County
Municipality : South Fayette Township
Client Code: #4392

Land Use Code: 934
Description: Fast-Food Restaurant with Drive-Through Window

SF Gross Floor Area: 8,250
X = 1,000 Square Feet Gross Floor Area

PIAZZA BACKGROUND TRIPS

Page: 725 ADT

Equation: Not Given *Average Rate = 467.48*

T =	3,858 ADT	
50% entering =		1,929
50% exiting =		1,929

Page: 726 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given *Average Rate = 44.61*

T =	368 AM Peak	
51% entering =		188
49% exiting =		180

Table E.31

Primary	Pass-by (49%)
96	92
92	88

Page: 727 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given *Average Rate = 33.03*

T =	272 PM Peak	
52% entering =		141
48% exiting =		131

Table E.32

Primary	Pass-by (31%)
97	44
90	41

Page: 731 Saturday (peak hour of generator)

Equation: Not Given *Average Rate = 55.25*

T =	456 SAT Peak	
51% entering =		233
49% exiting =		223

Table E.32

Primary	Pass-by (21%)*
184	49
176	47

* PM peak hour minus 10%

TRIP GENERATION

David E. Wooster and Associates, LLC
2 East Crafton Avenue
Pittsburgh, PA 15205

County : Allegheny County
Municipality : South Fayette Township
Client Code: #4392

Land Use Code: 931
Description: Fine Dining Restaurant

SF Gross Floor Area: 9,377
X = 1,000 Square Feet Gross Floor Area

NEWBURY CIGAR LOUNGE, BAR, AND RESTAURTANT

Page: 646 ADT

Equation: Not Given *Average Rate = 83.84*

T =	788 ADT	
50% entering =		394
50% exiting =		394

Page: 647 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given *Average Rate = 0.73*

T =	7 AM Peak	
**50% entering =		4
**50% exiting =		3

Table E.29

Primary	Pass-by (34%)*
3	1
2	1

Page: 648 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given *Average Rate = 7.80*

T =	73 PM Peak	
67% entering =		49
33% exiting =		24

Table E.29

Primary	Pass-by (44%)
27	22
13	11

Page: 652 Saturday (peak hour of generator)

Equation: Not Given *Average Rate = 10.68*

T =	100 SAT Peak	
59% entering =		59
41% exiting =		41

Table E.29

Primary	Pass-by (34%)*
39	20
21	20

**-Directional Distribution Not Available. Distribution Assumed.

* PM peak hour minus 10%

APPENDIX N

Synchro Printouts – *Opening Year 2024 With Development Condition*

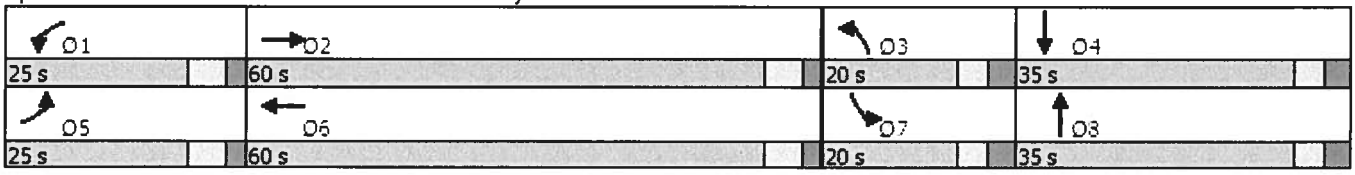
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	617	90	121	381	217	57	28	113	211	40	22
Future Volume (vph)	47	617	90	121	381	217	57	28	113	211	40	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.981				0.850			0.850		0.947	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3297	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3297	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13				238			124		18	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	52	678	99	133	419	238	63	31	124	232	44	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	52	777	0	133	419	238	63	31	124	232	68	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.7	28.8		11.5	35.8	35.8	7.8	6.7	6.7	10.6	8.9	
Actuated g/C Ratio	0.09	0.35		0.14	0.43	0.43	0.09	0.08	0.08	0.13	0.11	
v/c Ratio	0.34	0.67		0.54	0.30	0.29	0.36	0.22	0.52	0.53	0.34	
Control Delay	47.0	25.9		45.3	17.1	3.3	46.3	45.6	17.0	41.6	35.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.0	25.9		45.3	17.1	3.3	46.3	45.6	17.0	41.6	35.8	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		27.2			17.7			29.5			40.3	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	25	167		64	74	0	31	15	0	57	24	
Queue Length 95th (ft)	75	284		148	130	43	86	51	57	121	77	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	394	2260		428	2243	1179	326	625	644	606	634	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.13	0.34		0.31	0.19	0.20	0.19	0.05	0.19	0.38	0.11	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 82.8
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.67
Intersection Signal Delay: 25.8 Intersection LOS: C
Intersection Capacity Utilization 54.3% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



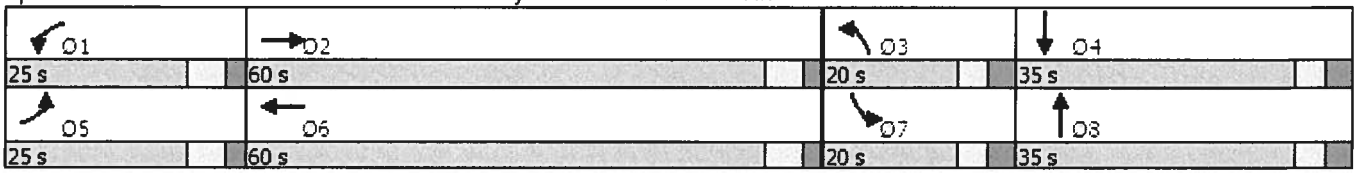
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	81	502	66	98	706	367	58	37	140	351	53	70
Future Volume (vph)	81	502	66	98	706	367	58	37	140	351	53	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.983				0.850			0.850		0.915	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3388	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3388	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12				378			144		42	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	84	518	68	101	728	378	60	38	144	362	55	72
Shared Lane Traffic (%)												
Lane Group Flow (vph)	84	586	0	101	728	378	60	38	144	362	127	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	9.3	33.5		10.1	37.6	37.6	7.8	7.4	7.4	14.6	17.3	
Actuated g/C Ratio	0.10	0.37		0.11	0.41	0.41	0.09	0.08	0.08	0.16	0.19	
v/c Ratio	0.47	0.47		0.51	0.52	0.42	0.38	0.24	0.55	0.66	0.36	
Control Delay	52.1	22.0		51.9	21.8	3.5	51.7	48.3	16.7	46.0	31.9	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	52.1	22.0		51.9	21.8	3.5	51.7	48.3	16.7	46.0	31.9	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		25.8			18.6			30.3			42.4	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	45	121		54	160	0	32	21	0	99	43	
Queue Length 95th (ft)	114	202		131	257	54	89	62	61	#233	125	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	384	2115		389	2121	1166	297	648	630	551	589	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.28		0.26	0.34	0.32	0.20	0.06	0.23	0.66	0.22	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 90.7
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.66
Intersection Signal Delay: 26.0 Intersection LOS: C
Intersection Capacity Utilization 58.2% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



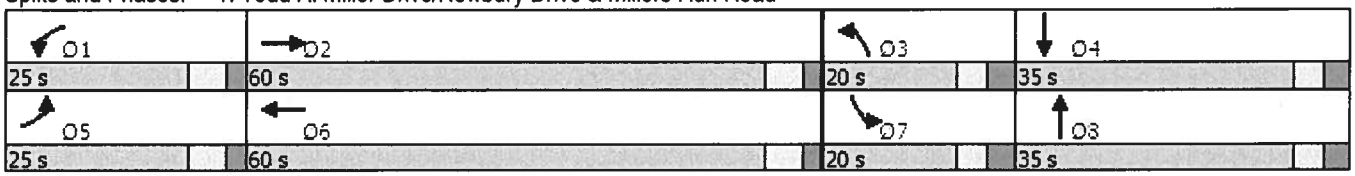
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	96	510	92	99	534	426	74	37	135	394	49	78
Future Volume (vph)	96	510	92	99	534	426	74	37	135	394	49	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.977				0.850			0.850		0.908	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3395	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3395	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				463			147		52	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	104	554	100	108	580	463	80	40	147	428	53	85
Shared Lane Traffic (%)												
Lane Group Flow (vph)	104	654	0	108	580	463	80	40	147	428	138	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	10.2	27.1		10.2	30.3	30.3	8.7	7.8	7.8	14.8	16.9	
Actuated g/C Ratio	0.12	0.32		0.12	0.36	0.36	0.10	0.09	0.09	0.17	0.20	
v/c Ratio	0.50	0.60		0.50	0.48	0.52	0.43	0.23	0.52	0.71	0.36	
Control Delay	48.4	25.5		48.0	23.4	4.3	48.5	44.6	14.9	44.6	28.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	48.4	25.5		48.0	23.4	4.3	48.5	44.6	14.9	44.6	28.6	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.7			18.0			29.4			40.7	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	47	137		48	122	0	36	18	0	98	37	
Queue Length 95th (ft)	136	239		140	213	62	112	63	61	#301	131	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	413	2280		427	2304	1276	320	697	686	599	651	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.25	0.29		0.25	0.25	0.36	0.25	0.06	0.21	0.71	0.21	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 85.3
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.71
Intersection Signal Delay: 26.8 Intersection LOS: C
Intersection Capacity Utilization 57.9% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



APPENDIX O

HCM Printouts – *Opening Year 2024 With Development Condition*

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	47	617	90	121	381	217	57	28	113	211	40	22
Future Volume (veh/h)	47	617	90	121	381	217	57	28	113	211	40	22
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	52	678	99	133	419	0	63	31	0	232	44	9
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	89	925	135	174	1196		113	64		362	121	25
Arrive On Green	0.05	0.30	0.30	0.10	0.34	0.00	0.06	0.03	0.00	0.11	0.08	0.08
Sat Flow, veh/h	1747	3101	452	1818	3485	1656	1959	1847	1654	3445	1466	300
Grp Volume(v), veh/h	52	387	390	133	419	0	63	31	0	232	0	53
Grp Sat Flow(s),veh/h/ln	1747	1771	1783	1818	1743	1656	1959	1847	1654	1722	0	1766
Q Serve(g_s), s	1.5	10.1	10.1	3.7	4.6	0.0	1.6	0.8	0.0	3.3	0.0	1.5
Cycle Q Clear(g_c), s	1.5	10.1	10.1	3.7	4.6	0.0	1.6	0.8	0.0	3.3	0.0	1.5
Prop In Lane	1.00		0.25	1.00		1.00	1.00		1.00	1.00		0.17
Lane Grp Cap(c), veh/h	89	528	532	174	1196		113	64		362	0	145
V/C Ratio(X)	0.58	0.73	0.73	0.76	0.35		0.56	0.48		0.64	0.00	0.36
Avail Cap(c_a), veh/h	644	1857	1869	671	3654		533	1040		936	0	994
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	23.9	16.2	16.2	22.7	12.6	0.0	23.6	24.4	0.0	22.1	0.0	22.4
Incr Delay (d2), s/veh	2.3	0.9	0.9	2.6	0.1	0.0	1.6	2.1	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	3.4	3.5	1.5	1.5	0.0	0.8	0.4	0.0	1.3	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.2	17.1	17.1	25.3	12.7	0.0	25.2	26.5	0.0	22.8	0.0	22.9
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		829			552			94			285	
Approach Delay, s/veh		17.7			15.7			25.6			22.8	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.9	21.4	9.0	10.2	8.6	23.7	11.4	7.8
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.7	12.1	3.6	3.5	3.5	6.6	5.3	2.8
Green Ext Time (p_c), s	0.1	3.2	0.0	0.1	0.0	1.9	0.3	0.1

Intersection Summary	
HCM 6th Ctrl Delay	18.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	81	502	66	98	706	367	58	37	140	351	53	70
Future Volume (veh/h)	81	502	66	98	706	367	58	37	140	351	53	70
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	84	518	65	101	728	0	60	38	0	362	55	39
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	124	924	116	133	1047		109	83		503	132	94
Arrive On Green	0.07	0.28	0.28	0.07	0.29	0.00	0.06	0.04	0.00	0.15	0.13	0.13
Sat Flow, veh/h	1847	3244	406	1818	3599	1683	1959	2057	1705	3445	1007	714
Grp Volume(v), veh/h	84	289	294	101	728	0	60	38	0	362	0	94
Grp Sat Flow(s),veh/h/ln	1847	1814	1836	1818	1800	1683	1959	2057	1705	1722	0	1721
Q Serve(g_s), s	2.3	7.1	7.2	2.9	9.5	0.0	1.6	1.0	0.0	5.3	0.0	2.6
Cycle Q Clear(g_c), s	2.3	7.1	7.2	2.9	9.5	0.0	1.6	1.0	0.0	5.3	0.0	2.6
Prop In Lane	1.00		0.22	1.00		1.00	1.00		1.00	1.00		0.41
Lane Grp Cap(c), veh/h	124	516	523	133	1047		109	83		503	0	226
V/C Ratio(X)	0.68	0.56	0.56	0.76	0.70		0.55	0.46		0.72	0.00	0.42
Avail Cap(c_a), veh/h	666	1859	1882	656	3689		521	1132		915	0	947
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.0	16.0	16.0	24.0	16.6	0.0	24.2	24.7	0.0	21.5	0.0	21.0
Incr Delay (d2), s/veh	2.4	0.4	0.4	3.3	0.4	0.0	1.6	1.4	0.0	0.7	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	2.5	2.6	1.2	3.3	0.0	0.7	0.5	0.0	2.0	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.4	16.5	16.5	27.3	17.0	0.0	25.9	26.2	0.0	22.2	0.0	21.5
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		667			829			98			456	
Approach Delay, s/veh		17.7			18.2			26.0			22.1	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	9.9	21.0	8.9	12.9	9.5	21.3	13.7	8.1
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	4.9	9.2	3.6	4.6	4.3	11.5	7.3	3.0
Green Ext Time (p_c), s	0.1	2.3	0.0	0.3	0.1	3.5	0.4	0.1

Intersection Summary	
HCM 6th Ctrl Delay	19.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	96	510	92	99	534	426	74	37	135	394	49	78
Future Volume (veh/h)	96	510	92	99	534	426	74	37	135	394	49	78
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	104	554	96	108	580	0	80	40	0	428	53	47
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	137	867	150	142	1012		126	86		567	130	115
Arrive On Green	0.07	0.28	0.28	0.08	0.28	0.00	0.06	0.04	0.00	0.16	0.14	0.14
Sat Flow, veh/h	1847	3141	543	1847	3628	1709	1959	2057	1743	3472	926	821
Grp Volume(v), veh/h	104	324	326	108	580	0	80	40	0	428	0	100
Grp Sat Flow(s),veh/h/ln	1847	1842	1842	1847	1814	1709	1959	2057	1743	1736	0	1746
Q Serve(g_s), s	3.0	8.4	8.5	3.1	7.5	0.0	2.2	1.0	0.0	6.4	0.0	2.8
Cycle Q Clear(g_c), s	3.0	8.4	8.5	3.1	7.5	0.0	2.2	1.0	0.0	6.4	0.0	2.8
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.47
Lane Grp Cap(c), veh/h	137	509	508	142	1012		126	86		567	0	245
V/C Ratio(X)	0.76	0.64	0.64	0.76	0.57		0.63	0.47		0.75	0.00	0.41
Avail Cap(c_a), veh/h	646	1831	1831	646	3606		505	1098		895	0	932
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.7	17.3	17.3	24.6	16.8	0.0	24.8	25.4	0.0	21.7	0.0	21.3
Incr Delay (d2), s/veh	3.2	0.6	0.6	3.1	0.2	0.0	1.9	1.5	0.0	0.8	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	3.1	3.1	1.3	2.6	0.0	1.0	0.5	0.0	2.5	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.9	17.9	17.9	27.7	17.0	0.0	26.7	26.9	0.0	22.5	0.0	21.7
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		754			688			120			528	
Approach Delay, s/veh		19.3			18.7			26.8			22.3	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.2	21.0	9.5	13.6	10.0	21.2	14.9	8.3
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.1	10.5	4.2	4.8	5.0	9.5	8.4	3.0
Green Ext Time (p_c), s	0.1	2.6	0.1	0.3	0.1	2.7	0.5	0.1

Intersection Summary	
HCM 6th Ctrl Delay	20.3
HCM 6th LOS	C

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	754	446	5	0	4
Future Vol, veh/h	0	754	446	5	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	820	485	5	0	4

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	245
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	755
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	755
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.8
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	755
HCM Lane V/C Ratio	-	-	-	0.006
HCM Control Delay (s)	-	-	-	9.8
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	649	807	16	0	17
Future Vol, veh/h	0	649	807	16	0	17
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	2	2
Mvmt Flow	0	705	877	17	0	18

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	447
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	559
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	559
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.7
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	559
HCM Lane V/C Ratio	-	-	-	0.033
HCM Control Delay (s)	-	-	-	11.7
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	698	652	18	0	15
Future Vol, veh/h	0	698	652	18	0	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	759	709	20	0	16

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	365
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	632
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	632
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	10.8
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	632
HCM Lane V/C Ratio	-	-	-	0.026
HCM Control Delay (s)	-	-	-	10.8
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	754	448	12	0	3
Future Vol, veh/h	0	754	448	12	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	820	487	13	0	3

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	250
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	750
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	750
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.8
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	750
HCM Lane V/C Ratio	-	-	-	0.004
HCM Control Delay (s)	-	-	-	9.8
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

Intersection						
Int Delay, s/veh	0.2					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	649	801	33	0	22
Future Vol, veh/h	0	649	801	33	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	0	2
Mvmt Flow	0	705	871	36	0	24

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	454
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	553
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	553
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.8
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	553
HCM Lane V/C Ratio	-	-	-	0.043
HCM Control Delay (s)	-	-	-	11.8
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↱
Traffic Vol, veh/h	0	698	652	34	0	18
Future Vol, veh/h	0	698	652	34	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	759	709	37	0	20

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	373
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	624
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	624
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	624
HCM Lane V/C Ratio	-	-	-	0.031
HCM Control Delay (s)	-	-	-	11
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↑↑			↑↑	
Traffic Vol, veh/h	5	0	18	35	0	11	24	209	59	17	213	7
Future Vol, veh/h	5	0	18	35	0	11	24	209	59	17	213	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	3	2	0	2	5	0	0	2	2
Mvmt Flow	6	0	21	41	0	13	28	246	69	20	251	8

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	474	666	130	503	636	158	259	0	0	315	0	0
Stage 1	295	295	-	337	337	-	-	-	-	-	-	-
Stage 2	179	371	-	166	299	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.56	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.56	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.56	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.53	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	474	379	896	449	394	866	1303	-	-	1257	-	-
Stage 1	689	668	-	648	640	-	-	-	-	-	-	-
Stage 2	805	618	-	817	665	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	451	362	896	423	376	866	1303	-	-	1257	-	-
Mov Cap-2 Maneuver	451	362	-	423	376	-	-	-	-	-	-	-
Stage 1	671	655	-	631	623	-	-	-	-	-	-	-
Stage 2	772	602	-	783	652	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	10.1	13.4	0.7	0.7
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1303	-	-	738	482	1257	-	-
HCM Lane V/C Ratio	0.022	-	-	0.037	0.112	0.016	-	-
HCM Control Delay (s)	7.8	0.1	-	10.1	13.4	7.9	0.1	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.4	0	-	-

Intersection	
Int Delay, s/veh	3.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↑↑↔			↑↑↔	
Traffic Vol, veh/h	23	0	57	55	2	28	45	358	62	13	365	19
Future Vol, veh/h	23	0	57	55	2	28	45	358	62	13	365	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	0	2	0	2	2	0	0	2	2
Mvmt Flow	25	0	62	60	2	30	49	389	67	14	397	21

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	730	990	209	748	967	228	418	0	0	456	0	0
Stage 1	436	436	-	521	521	-	-	-	-	-	-	-
Stage 2	294	554	-	227	446	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.5	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.5	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	310	245	797	305	253	781	1138	-	-	1115	-	-
Stage 1	569	578	-	512	530	-	-	-	-	-	-	-
Stage 2	690	512	-	761	572	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	279	227	797	266	235	781	1138	-	-	1115	-	-
Mov Cap-2 Maneuver	279	227	-	266	235	-	-	-	-	-	-	-
Stage 1	536	569	-	482	499	-	-	-	-	-	-	-
Stage 2	622	482	-	691	563	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	13.3	19.6	1	0.4
HCM LOS	B	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1138	-	-	520	338	1115	-	-
HCM Lane V/C Ratio	0.043	-	-	0.167	0.273	0.013	-	-
HCM Control Delay (s)	8.3	0.2	-	13.3	19.6	8.3	0.1	-
HCM Lane LOS	A	A	-	B	C	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.6	1.1	0	-	-

Intersection												
Int Delay, s/veh	8.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕↑			↕↑	
Traffic Vol, veh/h	26	0	61	105	2	25	54	414	106	20	364	23
Future Vol, veh/h	26	0	61	105	2	25	54	414	106	20	364	23
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	1	2	4	2	0	2	0	1	2
Mvmt Flow	31	0	72	124	2	29	64	487	125	24	428	27

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	863	1230	228	940	1181	306	455	0	0	612	0	0
Stage 1	490	490	-	678	678	-	-	-	-	-	-	-
Stage 2	373	740	-	262	503	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.52	6.54	6.98	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.51	4.02	3.34	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	248	176	775	220	189	684	1102	-	-	977	-	-
Stage 1	529	547	-	411	450	-	-	-	-	-	-	-
Stage 2	620	421	-	723	540	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	213	155	775	181	166	684	1102	-	-	977	-	-
Mov Cap-2 Maneuver	213	155	-	181	166	-	-	-	-	-	-	-
Stage 1	481	529	-	374	410	-	-	-	-	-	-	-
Stage 2	537	383	-	634	522	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	15.9	59.1	1	0.5
HCM LOS	C	F		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1102	-	-	433	210	977	-	-
HCM Lane V/C Ratio	0.058	-	-	0.236	0.739	0.024	-	-
HCM Control Delay (s)	8.5	0.3	-	15.9	59.1	8.8	0.1	-
HCM Lane LOS	A	A	-	C	F	A	A	-
HCM 95th %tile Q(veh)	0.2	-	-	0.9	4.9	0.1	-	-

APPENDIX P

Synchro Printouts – *Design Year 2029 With Development Condition*

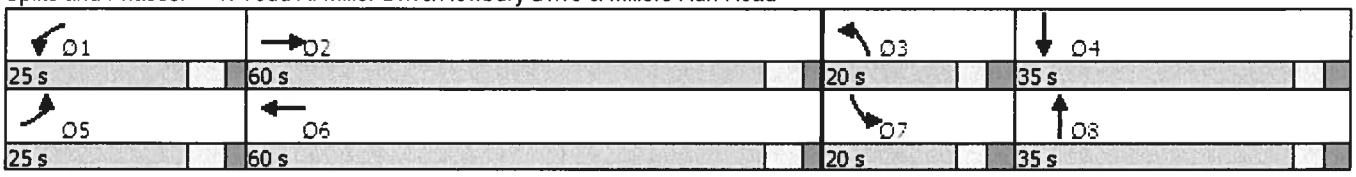
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	49	649	90	121	400	228	57	28	113	221	40	23
Future Volume (vph)	49	649	90	121	400	228	57	28	113	221	40	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.982				0.850			0.850		0.946	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1692	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1692	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13				251			124		18	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	54	713	99	133	440	251	63	31	124	243	44	25
Shared Lane Traffic (%)												
Lane Group Flow (vph)	54	812	0	133	440	251	63	31	124	243	69	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.8	30.8		11.7	37.8	37.8	7.9	6.8	6.8	11.0	9.3	
Actuated g/C Ratio	0.09	0.36		0.14	0.44	0.44	0.09	0.08	0.08	0.13	0.11	
v/c Ratio	0.36	0.68		0.55	0.30	0.29	0.37	0.23	0.53	0.55	0.35	
Control Delay	48.8	26.1		47.0	17.1	3.2	48.1	47.3	17.5	43.2	37.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	48.8	26.1		47.0	17.1	3.2	48.1	47.3	17.5	43.2	37.0	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		27.5			17.7			30.6			41.8	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	27	181		67	80	0	32	16	0	62	25	
Queue Length 95th (ft)	78	300		152	137	43	88	53	57	129	80	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	382	2193		415	2175	1154	316	605	628	587	614	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.14	0.37		0.32	0.20	0.22	0.20	0.05	0.20	0.41	0.11	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 85.5
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 26.2 Intersection LOS: C
Intersection Capacity Utilization 55.5% ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



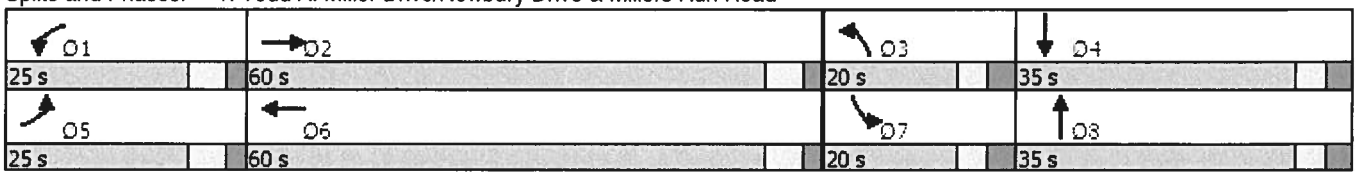
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	84	528	66	98	740	384	58	37	140	366	53	73
Future Volume (vph)	84	528	66	98	740	384	58	37	140	366	53	73
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.983				0.850			0.850		0.913	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1676	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1676	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11				396			144		44	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	87	544	68	101	763	396	60	38	144	377	55	75
Shared Lane Traffic (%)												
Lane Group Flow (vph)	87	612	0	101	763	396	60	38	144	377	130	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	9.5	35.6		10.2	39.6	39.6	7.9	7.5	7.5	14.7	17.5	
Actuated g/C Ratio	0.10	0.38		0.11	0.43	0.43	0.08	0.08	0.08	0.16	0.19	
v/c Ratio	0.49	0.47		0.52	0.53	0.43	0.39	0.24	0.55	0.70	0.37	
Control Delay	53.9	21.9		53.6	21.9	3.5	53.3	49.4	16.9	49.0	32.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	53.9	21.9		53.6	21.9	3.5	53.3	49.4	16.9	49.0	32.7	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		25.9			18.6			31.0			44.8	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	49	130		56	172	0	34	22	0	109	46	
Queue Length 95th (ft)	117	213		131	274	56	89	61	61	#248	127	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	374	2062		380	2069	1154	289	632	619	538	576	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.23	0.30		0.27	0.37	0.34	0.21	0.06	0.23	0.70	0.23	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 93.1
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.70
Intersection Signal Delay: 26.5 Intersection LOS: C
Intersection Capacity Utilization 59.7% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



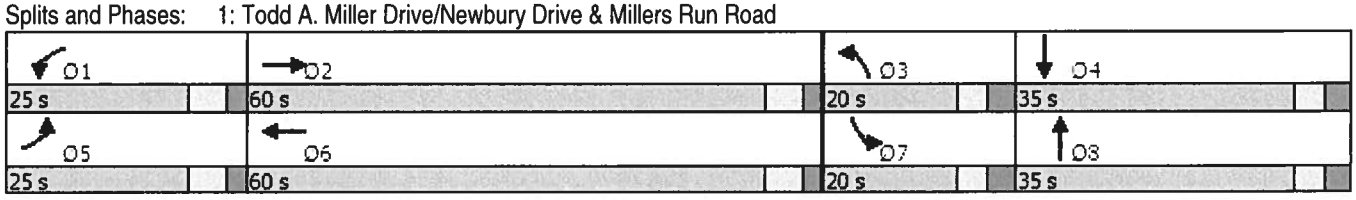
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	99	537	92	99	559	446	74	37	135	411	49	81
Future Volume (vph)	99	537	92	99	559	446	74	37	135	411	49	81
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.978				0.850			0.850		0.906	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3400	0	1814	3438	1677	1841	1938	1647	3450	1713	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3400	0	1814	3438	1677	1841	1938	1647	3450	1713	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		16				485			147		54	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	108	584	100	108	608	485	80	40	147	447	53	88
Shared Lane Traffic (%)												
Lane Group Flow (vph)	108	684	0	108	608	485	80	40	147	447	141	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	10.5	30.4		10.3	30.2	30.2	8.9	8.0	8.0	14.9	17.1	
Actuated g/C Ratio	0.12	0.34		0.12	0.34	0.34	0.10	0.09	0.09	0.17	0.19	
v/c Ratio	0.52	0.58		0.51	0.52	0.55	0.44	0.23	0.52	0.78	0.38	
Control Delay	51.1	24.8		50.9	24.5	4.5	51.3	46.7	15.1	49.8	30.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	51.1	24.8		50.9	24.5	4.5	51.3	46.7	15.1	49.8	30.0	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.4			18.8			30.7			45.1	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	57	147		57	131	0	42	21	0	123	44	
Queue Length 95th (ft)	143	252		143	227	62	114	64	62	#328	134	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	397	2195		410	2212	1252	307	669	665	575	627	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.27	0.31		0.26	0.27	0.39	0.26	0.06	0.22	0.78	0.22	

Intersection Summary

Area Type: Other
Cycle Length: 140
Actuated Cycle Length: 89.1
Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.78
Intersection Signal Delay: 28.0 Intersection LOS: C
Intersection Capacity Utilization 59.2% ICU Level of Service B
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road



APPENDIX Q

HCM Printouts – *Design Year 2029 With Development Condition*

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	49	649	90	121	400	228	57	28	113	221	40	23
Future Volume (veh/h)	49	649	90	121	400	228	57	28	113	221	40	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	54	713	99	133	440	0	63	31	0	243	44	10
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	90	960	133	174	1225		112	64		373	123	28
Arrive On Green	0.05	0.31	0.31	0.10	0.35	0.00	0.06	0.03	0.00	0.11	0.09	0.09
Sat Flow, veh/h	1747	3124	433	1818	3485	1656	1959	1847	1654	3445	1435	326
Grp Volume(v), veh/h	54	404	408	133	440	0	63	31	0	243	0	54
Grp Sat Flow(s),veh/h/ln	1747	1771	1786	1818	1743	1656	1959	1847	1654	1722	0	1761
Q Serve(g_s), s	1.6	10.8	10.8	3.8	5.0	0.0	1.7	0.9	0.0	3.6	0.0	1.5
Cycle Q Clear(g_c), s	1.6	10.8	10.8	3.8	5.0	0.0	1.7	0.9	0.0	3.6	0.0	1.5
Prop In Lane	1.00		0.24	1.00		1.00	1.00		1.00	1.00		0.19
Lane Grp Cap(c), veh/h	90	545	549	174	1225		112	64		373	0	151
V/C Ratio(X)	0.60	0.74	0.74	0.76	0.36		0.56	0.49		0.65	0.00	0.36
Avail Cap(c_a), veh/h	628	1809	1825	654	3561		519	1014		912	0	966
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	16.4	16.4	23.3	12.7	0.0	24.3	25.1	0.0	22.6	0.0	22.8
Incr Delay (d2), s/veh	2.3	0.9	0.9	2.6	0.1	0.0	1.7	2.1	0.0	0.7	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	3.7	3.8	1.5	1.6	0.0	0.8	0.4	0.0	1.4	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.9	17.4	17.4	25.9	12.8	0.0	25.9	27.2	0.0	23.3	0.0	23.3
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		866			573			94			297	
Approach Delay, s/veh		17.9			15.9			26.3			23.3	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	11.1	22.3	9.0	10.5	8.7	24.6	11.7	7.8
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.8	12.8	3.7	3.5	3.6	7.0	5.6	2.9
Green Ext Time (p_c), s	0.1	3.4	0.0	0.2	0.0	2.0	0.3	0.1

Intersection Summary	
HCM 6th Ctrl Delay	18.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	84	528	66	98	740	384	58	37	140	366	53	73
Future Volume (veh/h)	84	528	66	98	740	384	58	37	140	366	53	73
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	87	544	65	101	763	0	60	38	0	377	55	42
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	125	949	113	132	1065		108	83		516	131	100
Arrive On Green	0.07	0.29	0.29	0.07	0.30	0.00	0.06	0.04	0.00	0.15	0.13	0.13
Sat Flow, veh/h	1847	3264	389	1818	3599	1683	1959	2057	1705	3445	973	743
Grp Volume(v), veh/h	87	302	307	101	763	0	60	38	0	377	0	97
Grp Sat Flow(s),veh/h/ln	1847	1814	1839	1818	1800	1683	1959	2057	1705	1722	0	1716
Q Serve(g_s), s	2.5	7.6	7.6	2.9	10.2	0.0	1.6	1.0	0.0	5.6	0.0	2.8
Cycle Q Clear(g_c), s	2.5	7.6	7.6	2.9	10.2	0.0	1.6	1.0	0.0	5.6	0.0	2.8
Prop In Lane	1.00		0.21	1.00		1.00	1.00		1.00	1.00		0.43
Lane Grp Cap(c), veh/h	125	527	535	132	1065		108	83		516	0	232
V/C Ratio(X)	0.70	0.57	0.57	0.76	0.72		0.56	0.46		0.73	0.00	0.42
Avail Cap(c_a), veh/h	653	1823	1848	643	3616		510	1110		897	0	926
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	16.2	16.2	24.5	16.9	0.0	24.8	25.2	0.0	21.8	0.0	21.3
Incr Delay (d2), s/veh	2.6	0.4	0.4	3.4	0.4	0.0	1.7	1.5	0.0	0.8	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	2.7	2.7	1.2	3.5	0.0	0.8	0.5	0.0	2.2	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.1	16.7	16.7	27.9	17.3	0.0	26.4	26.7	0.0	22.6	0.0	21.8
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		696			864			98			474	
Approach Delay, s/veh		18.0			18.6			26.5			22.4	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	9.9	21.6	9.0	13.3	9.6	21.9	14.0	8.2
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	4.9	9.6	3.6	4.8	4.5	12.2	7.6	3.0
Green Ext Time (p_c), s	0.1	2.4	0.0	0.3	0.1	3.7	0.5	0.1

Intersection Summary	
HCM 6th Ctrl Delay	19.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	99	537	92	99	559	446	74	37	135	411	49	81
Future Volume (veh/h)	99	537	92	99	559	446	74	37	135	411	49	81
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	108	584	96	108	608	0	80	40	0	447	53	50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	142	869	142	142	995		126	86		585	131	123
Arrive On Green	0.08	0.27	0.27	0.08	0.27	0.00	0.06	0.04	0.00	0.17	0.15	0.15
Sat Flow, veh/h	1847	3169	520	1847	3628	1709	1959	2057	1743	3472	896	846
Grp Volume(v), veh/h	108	339	341	108	608	0	80	40	0	447	0	103
Grp Sat Flow(s),veh/h/ln	1847	1842	1846	1847	1814	1709	1959	2057	1743	1736	0	1742
Q Serve(g_s), s	3.1	9.0	9.0	3.1	8.0	0.0	2.2	1.0	0.0	6.7	0.0	2.9
Cycle Q Clear(g_c), s	3.1	9.0	9.0	3.1	8.0	0.0	2.2	1.0	0.0	6.7	0.0	2.9
Prop In Lane	1.00		0.28	1.00		1.00	1.00		1.00	1.00		0.49
Lane Grp Cap(c), veh/h	142	505	506	142	995		126	86		585	0	254
V/C Ratio(X)	0.76	0.67	0.67	0.76	0.61		0.64	0.47		0.76	0.00	0.41
Avail Cap(c_a), veh/h	641	1818	1822	641	3580		501	1090		888	0	923
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.7	17.7	17.7	24.7	17.3	0.0	25.0	25.6	0.0	21.7	0.0	21.2
Incr Delay (d2), s/veh	3.1	0.7	0.7	3.1	0.3	0.0	2.0	1.5	0.0	0.9	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	3.3	3.3	1.3	2.8	0.0	1.0	0.5	0.0	2.6	0.0	1.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.8	18.4	18.4	27.8	17.6	0.0	26.9	27.1	0.0	22.6	0.0	21.6
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		788			716			120			550	
Approach Delay, s/veh		19.7			19.1			27.0			22.4	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.2	21.0	9.5	14.0	10.2	21.0	15.2	8.3
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0
Max Q Clear Time (g_c+I1), s	5.1	11.0	4.2	4.9	5.1	10.0	8.7	3.0
Green Ext Time (p_c), s	0.1	2.7	0.1	0.4	0.1	2.8	0.5	0.1

Intersection Summary	
HCM 6th Ctrl Delay	20.6
HCM 6th LOS	C

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	788	466	5	0	4
Future Vol, veh/h	0	788	466	5	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	857	507	5	0	4

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	256
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	743
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	743
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.9
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	743
HCM Lane V/C Ratio	-	-	-	0.006
HCM Control Delay (s)	-	-	-	9.9
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	678	844	16	0	17
Future Vol, veh/h	0	678	844	16	0	17
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	2	2
Mvmt Flow	0	737	917	17	0	18

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	467
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	542
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	542
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.9
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	542
HCM Lane V/C Ratio	-	-	-	0.034
HCM Control Delay (s)	-	-	-	11.9
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↱
Traffic Vol, veh/h	0	728	680	18	0	15
Future Vol, veh/h	0	728	680	18	0	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	791	739	20	0	16

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	380
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	618
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	618
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	618
HCM Lane V/C Ratio	-	-	-	0.026
HCM Control Delay (s)	-	-	-	11
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection						
Int Delay, s/veh	0					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	788	468	12	0	3
Future Vol, veh/h	0	788	468	12	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	857	509	13	0	3

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	261
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	738
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	738
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.9
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	738
HCM Lane V/C Ratio	-	-	-	0.004
HCM Control Delay (s)	-	-	-	9.9
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

Intersection						
Int Delay, s/veh	0.2					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	678	838	33	0	22
Future Vol, veh/h	0	678	838	33	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	0	2
Mvmt Flow	0	737	911	36	0	24

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	474
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	537
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	537
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	12
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	537
HCM Lane V/C Ratio	-	-	-	0.045
HCM Control Delay (s)	-	-	-	12
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.1					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑↱			↱
Traffic Vol, veh/h	0	728	680	34	0	18
Future Vol, veh/h	0	728	680	34	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	791	739	37	0	20

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	-	0	-	0	-	388
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	0	611
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	611
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.1
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	611
HCM Lane V/C Ratio	-	-	-	0.032
HCM Control Delay (s)	-	-	-	11.1
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection	
Int Delay, s/veh	2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	5	0	18	35	0	11	24	219	59	17	223	7
Future Vol, veh/h	5	0	18	35	0	11	24	219	59	17	223	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	3	2	0	2	5	0	0	2	2
Mvmt Flow	6	0	21	41	0	13	28	258	69	20	262	8

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	491	689	135	520	659	164	270	0	0	327	0	0
Stage 1	306	306	-	349	349	-	-	-	-	-	-	-
Stage 2	185	383	-	171	310	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.56	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.56	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.56	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.53	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	461	367	889	437	382	858	1290	-	-	1244	-	-
Stage 1	679	660	-	638	632	-	-	-	-	-	-	-
Stage 2	799	610	-	811	658	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	438	350	889	412	365	858	1290	-	-	1244	-	-
Mov Cap-2 Maneuver	438	350	-	412	365	-	-	-	-	-	-	-
Stage 1	661	647	-	621	615	-	-	-	-	-	-	-
Stage 2	766	594	-	777	645	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	10.2	13.7	0.7	0.6
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1290	-	-	726	470	1244	-	-
HCM Lane V/C Ratio	0.022	-	-	0.037	0.115	0.016	-	-
HCM Control Delay (s)	7.9	0.1	-	10.2	13.7	7.9	0.1	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.4	0	-	-

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	23	0	57	55	2	28	45	375	62	13	382	19
Future Vol, veh/h	23	0	57	55	2	28	45	375	62	13	382	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	0	2	0	2	2	0	0	2	2
Mvmt Flow	25	0	62	60	2	30	49	408	67	14	415	21

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	757	1027	218	776	1004	238	436	0	0	475	0	0
Stage 1	454	454	-	540	540	-	-	-	-	-	-	-
Stage 2	303	573	-	236	464	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.5	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.5	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	297	233	786	291	240	769	1120	-	-	1098	-	-
Stage 1	555	568	-	499	519	-	-	-	-	-	-	-
Stage 2	681	502	-	752	562	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	267	215	786	253	222	769	1120	-	-	1098	-	-
Mov Cap-2 Maneuver	267	215	-	253	222	-	-	-	-	-	-	-
Stage 1	522	558	-	469	488	-	-	-	-	-	-	-
Stage 2	612	472	-	681	552	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	13.6	20.6	0.9	0.4
HCM LOS	B	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1120	-	-	504	323	1098	-	-
HCM Lane V/C Ratio	0.044	-	-	0.173	0.286	0.013	-	-
HCM Control Delay (s)	8.4	0.2	-	13.6	20.6	8.3	0.1	-
HCM Lane LOS	A	A	-	B	C	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.6	1.2	0	-	-

Intersection	
Int Delay, s/veh	9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕↕			↕↕	
Traffic Vol, veh/h	26	0	61	105	2	25	54	432	106	20	380	23
Future Vol, veh/h	26	0	61	105	2	25	54	432	106	20	380	23
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	1	2	4	2	0	2	0	1	2
Mvmt Flow	31	0	72	124	2	29	64	508	125	24	447	27

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	892	1270	237	971	1221	317	474	0	0	633	0	0
Stage 1	509	509	-	699	699	-	-	-	-	-	-	-
Stage 2	383	761	-	272	522	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.52	6.54	6.98	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.51	4.02	3.34	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	237	167	764	209	179	673	1084	-	-	960	-	-
Stage 1	515	536	-	399	440	-	-	-	-	-	-	-
Stage 2	611	412	-	713	529	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	203	146	764	171	157	673	1084	-	-	960	-	-
Mov Cap-2 Maneuver	203	146	-	171	157	-	-	-	-	-	-	-
Stage 1	467	518	-	362	399	-	-	-	-	-	-	-
Stage 2	527	374	-	624	511	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	16.4	67.5	1	0.5
HCM LOS	C	F		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1084	-	-	418	199	960	-	-
HCM Lane V/C Ratio	0.059	-	-	0.245	0.78	0.025	-	-
HCM Control Delay (s)	8.5	0.3	-	16.4	67.5	8.8	0.1	-
HCM Lane LOS	A	A	-	C	F	A	A	-
HCM 95th %tile Q(veh)	0.2	-	-	0.9	5.4	0.1	-	-

APPENDIX R

Radar Speed Study

Weather: Clouds & Sun/ 70's
Study By: J.K. (Spot Radar Speed)
Road: Newbury Dr. NB/SB approaches
Day: Wednesday, May 20, 2023 (3:30 pm)

#	SB app	NB app
1	27	22
2	23	22
3	27	17
4	30	20
5	23	23
6	24	22
7	34	22
8	26	17
9	27	26
10	25	22
11	28	24
12	26	21
13	23	25
14	27	24
15	28	17
16	30	20
17	21	24
18	20	19
19	25	22
20	23	21
21	30	27
22	23	18
23	24	27
24	28	21
25	23	23
26		

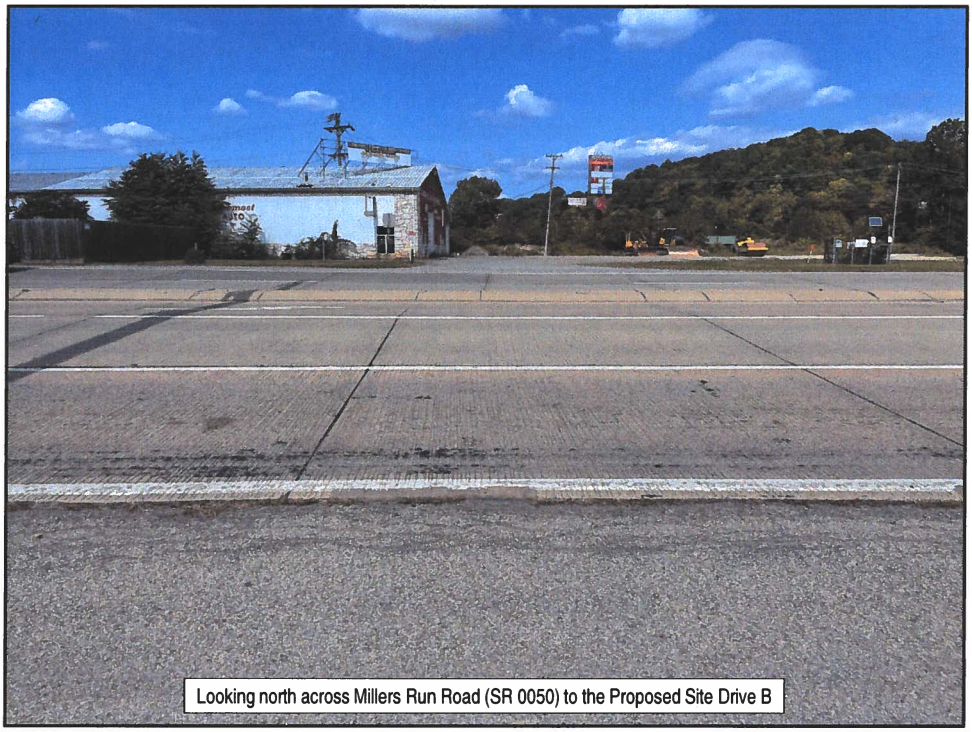
Class	Vehicle Count	Average Speed	85 Percentile
SB app	25	26	28
NB app	25	22	24
Summary	50	24	27

APPENDIX S

Sight Distance Photo Log

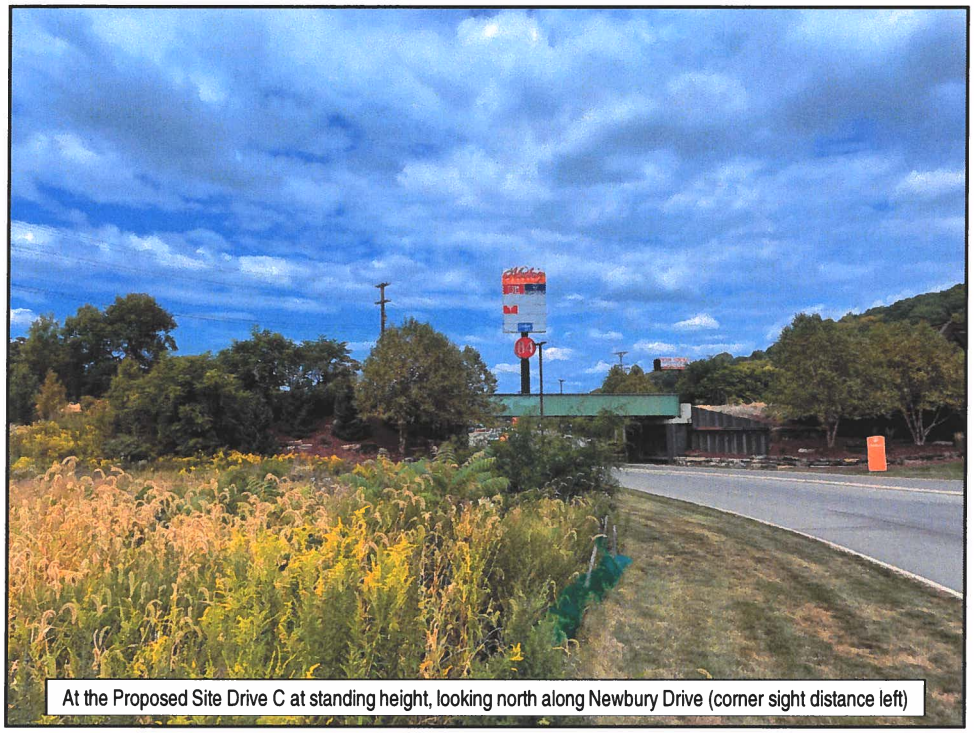
Looking north across Millers Run Road (SR 0050) to the Proposed Site Drive A

At the Proposed Site Drive A, looking east along Millers Run Road (SR 0050)
(corner sight distance left)



Looking north across Millers Run Road (SR 0050) to the Proposed Site Drive B

At the Proposed Site Drive B, looking east along Millers Run Road (SR 0050)
(corner sight distance left)



At the Proposed Site Drive C at standing height, looking north along Newbury Drive (corner sight distance left)

At the Proposed Site Drive C, looking south along Newbury Drive (corner sight distance right)

Forward view of a northbound, left turning vehicle from Newbury Drive into the Proposed Site Drive C
(stopping sight distance ahead)

Rear view of a northbound, left turning vehicle from Newbury Drive into the Proposed Site Drive C
(stopping sight distance behind)

APPENDIX T

SimTraffic Printouts – *Design Year 2029 Without Development Condition*

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	61	207	195	132	141	86	16	97	82	59	146	167
Average Queue (ft)	21	127	96	66	68	33	1	39	26	7	51	86
95th Queue (ft)	50	194	171	113	125	74	11	74	64	37	115	142
Link Distance (ft)		1063	1063		591	591		249	249	249		451
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0										
Queuing Penalty (veh)		0										

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	118
Average Queue (ft)	47
95th Queue (ft)	91
Link Distance (ft)	451
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Newbury Drive & Plaza Access

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	56	4	48
Average Queue (ft)	18	0	6
95th Queue (ft)	39	3	30
Link Distance (ft)	284	451	396
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 0

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	86	187	157	138	234	182	51	106	88	80	171	201
Average Queue (ft)	36	111	67	66	129	80	5	44	31	10	85	119
95th Queue (ft)	73	170	135	121	202	159	32	87	69	48	154	181
Link Distance (ft)		1063	1063		591	591		249	249	249		451
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0			1							
Queuing Penalty (veh)		0			1							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	151
Average Queue (ft)	61
95th Queue (ft)	116
Link Distance (ft)	451
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Newbury Drive & Plaza Access

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	89	66
Average Queue (ft)	28	8
95th Queue (ft)	60	37
Link Distance (ft)	284	396
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 1

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	90	189	173	125	201	156	73	110	104	65	177	208
Average Queue (ft)	41	109	83	57	110	57	11	50	31	5	87	120
95th Queue (ft)	79	169	147	104	178	124	49	91	72	34	155	180
Link Distance (ft)		1063	1063		591	591		249	249	249		451
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)					0							
Queuing Penalty (veh)					0							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	141
Average Queue (ft)	65
95th Queue (ft)	116
Link Distance (ft)	451
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Newbury Drive & Plaza Access

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	115	22	78
Average Queue (ft)	48	1	13
95th Queue (ft)	92	9	51
Link Distance (ft)	284	451	396
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 0

APPENDIX U

SimTraffic Printouts – *Design Year 2029 With Development Condition*

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	108	214	209	151	163	140	49	84	85	38	120	144
Average Queue (ft)	37	127	103	69	72	39	2	37	28	4	50	84
95th Queue (ft)	81	195	172	126	133	95	25	72	69	29	110	134
Link Distance (ft)		399	399		593	593		249	249	249		454
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0		0	0							
Queuing Penalty (veh)		0		0	0							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	100
Average Queue (ft)	43
95th Queue (ft)	81
Link Distance (ft)	454
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Millers Run Road & Site Drive B

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 7: Millers Run Road & Site Drive A

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 10: Newbury Drive & Site Drive C/Plaza Access

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LT	LT
Maximum Queue (ft)	13	54	31	38
Average Queue (ft)	7	23	5	4
95th Queue (ft)	17	49	24	20
Link Distance (ft)	210	298	454	363
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	110	185	171	137	229	195	82	116	77	56	189	206
Average Queue (ft)	53	107	85	62	141	95	11	44	33	9	102	135
95th Queue (ft)	99	163	148	112	217	178	52	86	67	43	171	198
Link Distance (ft)		399	399		593	593		249	249	249		454
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)					1							
Queuing Penalty (veh)					1							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	171
Average Queue (ft)	66
95th Queue (ft)	124
Link Distance (ft)	454
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Millers Run Road & Site Drive B

Movement	SB
Directions Served	R
Maximum Queue (ft)	16
Average Queue (ft)	1
95th Queue (ft)	8
Link Distance (ft)	223
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 7: Millers Run Road & Site Drive A

Movement	SB
Directions Served	R
Maximum Queue (ft)	13
Average Queue (ft)	0
95th Queue (ft)	7
Link Distance (ft)	232
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: Newbury Drive & Site Drive C/Plaza Access

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	LT	TR	LT	TR
Maximum Queue (ft)	51	89	52	4	47	7
Average Queue (ft)	18	35	16	0	4	0
95th Queue (ft)	39	68	44	3	23	3
Link Distance (ft)	210	298	454	454	363	363
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Network Summary

Network wide Queuing Penalty: 1

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	137	206	182	129	206	159	89	103	82	64	197	230
Average Queue (ft)	62	104	80	55	115	76	14	49	32	9	107	134
95th Queue (ft)	109	163	142	103	188	146	58	91	70	44	176	202
Link Distance (ft)		399	399		593	593		249	249	249		454
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0			0							
Queuing Penalty (veh)		0			0							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	143
Average Queue (ft)	62
95th Queue (ft)	118
Link Distance (ft)	454
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Millers Run Road & Site Drive B

Movement	SB
Directions Served	R
Maximum Queue (ft)	17
Average Queue (ft)	1
95th Queue (ft)	9
Link Distance (ft)	223
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 7: Millers Run Road & Site Drive A

Movement	SB
Directions Served	R
Maximum Queue (ft)	7
Average Queue (ft)	0
95th Queue (ft)	5
Link Distance (ft)	232
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: Newbury Drive & Site Drive C/Plaza Access

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	LT	TR	LT	TR
Maximum Queue (ft)	68	204	56	9	65	17
Average Queue (ft)	21	66	20	0	9	1
95th Queue (ft)	47	143	51	5	37	6
Link Distance (ft)	210	298	454	454	363	363
Upstream Blk Time (%)		0				
Queuing Penalty (veh)		0				
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Network Summary

Network wide Queuing Penalty: 0

CRASH DATA APPENDIX

for the proposed

RETAIL DEVELOPMENT

South Fayette Township, Allegheny County, Pennsylvania

October 17, 2023

CRASH DATA APPENDIX

for the proposed

RETAIL DEVELOPMENT

South Fayette Township, Allegheny County, PA

October 17, 2023

Prepared for: **Cozza Enterprises, LLC**
PO Box 453
Carnegie, PA 15106

Prepared by: **David E. Wooster and Associates, LLC**
Two East Crafton Avenue
Pittsburgh, PA 15205

Project Engineer(s): Jesse Nelson & Suleiman A. Swai, P.E.

Supervising Engineer: Joshua A. Haydo, P.E., PTOE

Crash Data Appendix
Proposed Retail Development
South Fayette Township, Allegheny County, Pennsylvania

1.0 PROJECT DESCRIPTION

The proposed project is located on the northwestern corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. The development is proposed to consist of ~45,126-square foot of retail space.

The study area for this project includes two (2) existing intersections:

- Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- Newbury Drive with Plaza Access

2.0 OBTAINING CRASH DATA

Copies of crash data summaries for the five (5) most recent calendar years were obtained from the Pennsylvania Department of Transportation (PennDOT) Pennsylvania Crash Information Tool (PCIT) for the existing study intersections.

As the crash data is property of PennDOT, a summary of the identified crashes and the corresponding crash reports have been provided in this separately-bound appendix.

3.0 CRASH DATA SUMMARY

3.1 Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

At the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive, eight (8) reportable crashes occurred between January 1, 2018 and December 31, 2022. Of the crashes, three (3) occurred in 2020, two (2) occurred in 2021, and three (3) occurred in 2022. Of the crashes four (4) were rear-end collisions, two (2) were opposite direction side-swipe collisions, one (1) involved a motorist striking a curb, and one (1) involved a motorist striking a traffic island/pole. None of the crashes involved fatalities.

3.2 Newbury Drive with Plaza Access

PCIT does not contain crash information for the intersection of Newbury Drive with Plaza Access.

4.0 SUMMARY / CONCLUSIONS

Based on a review of the crash data, five or more crashes of types susceptible to correction by traffic control or geometric improvements were not reported within a twelve-month period at any of the study intersections.

Copies of the crash data provided by PennDOT have been included in the Enclosures section of this appendix.

PENNDOT CRASH DATA

Millers Run Road (SR 0050) with Newbury Drive

Date Range: 01/01/2018 to 12/31/2022

USER ID / QUERY ID:
b-sswai / *0320231006133*

MONTH OF YEAR

	JAN	FEB	APR	MAY	JUN	AUG	NOV	TOTAL
CRASHES	1	1	1	1	1	1	2	8
PCT	13%	13%	13%	13%	13%	13%	25%	100%

DAY OF WEEK

	SUN	MON	TUE	FRI	TOTAL
CRASHES	1	4	2	1	8
PCT	13%	50%	25%	13%	100%

HOUR OF DAY

	05	07	08	13	16	17	18	19	TOTAL
CRASHES	1	1	1	1	1	1	1	1	8
PCT	13%	13%	13%	13%	13%	13%	13%	13%	100%

YEAR

	CRASHES	PCT
2020	3	38%
2021	2	25%
2022	3	38%
TOTAL	8	100%

COLLISION TYPE

	CRASHES	PCT
REAR END	4	50%
HIT FIX OBJ	2	25%
OPP DIR SS	2	25%
TOTAL	8	100%

CRASH SEVERITY LEVEL

	CRASHES	PCT
POSSIBLE INJURY	1	13%
UNK IF INJURED	1	13%
PDO	6	75%
TOTAL	8	100%

SEVERITY COUNT

	PERSONS
FATALITIES	0
SUSPECTED SERIOUS	0
SUSPECTED MINOR	0
POSSIBLE INJURY	1
UNK SEVERITY	0
UNK IF INJURED	2

DRIVER ACTIONS

	ACTIONS	PCT
NO CONTRIBUTING ACTION	7	39%
DRIVER WAS DISTRACTED	5	28%
FAILURE TO RESPOND TCD	1	6%
IMPROPER/CARELESS TURN	1	6%
OTHER IMPROPER DRIVING	1	6%
RUNNING RED LIGHT	1	6%
SUDDEN SLOWING/STOP	1	6%
TAILGATING	1	6%
TOTAL	18	100%

VEHICLE TYPE

	VEHICLES	PCT
AUTOMOBILE	6	38%
SUV	4	25%
SMALL TRUCK	3	19%
VAN	2	13%
BUS	1	6%
TOTAL	16	100%

ROAD CONDITION

	CRASHES	PCT
DRY	7	88%
WET	1	13%
TOTAL	8	100%

ILLUMINATION

	CRASHES	PCT
DAYLIGHT	6	75%
STREET LIGHTS	2	25%
TOTAL	8	100%

WEATHER

	CRASHES	PCT
CLEAR	7	88%
RAIN	1	13%
TOTAL	8	100%

ENVIR/ROADWAY FACTORS

	FACTORS	PCT
NONE	7	88%
OTHER WEATHER COND	1	13%
TOTAL	8	100%

NOTES:

1 Injury Severity Disclaimer

Please note that beginning January 1, 2016, PennDOT adopted the Federal standard for collecting injury severity data. The field descriptions and definitions changed from the state standard that had been in use for decades. This resulted in a substantial shift in severity levels. Therefore, comparison of the "Suspected Serious Injury", "Suspected Minor Injury" and "Possible Injury" categories will not be consistent for crashes taking place before versus after the adoption of the new standard.

REPORT PARAMETERS:

Query ID: *0320231006133*

User ID: b-sswai

Title: Millers Run Road (SR 0050)

Date Range: 01/01/2018 to 12/31/2022

Filter Characteristics:

Selected Shapes : NO NAME RD x MILLERS RUN RD,NO NAME RD x MILLERS RUN RD - Buffer (250 feet)

This report counts the number of crashes.

Pennsylvania Crash Information Tool

Date Range: 01/01/2018 to 12/31/2022*

CRASH SEVERITY LEVEL BY YEAR

	2020 CRASHES	2021 CRASHES	2022 CRASHES	ALL YEARS CRASHES
POSSIBLE INJURY	1	0	0	1
UNKNOWN IF INJURED	0	0	1	1
PROPERTY DMG ONLY	2	2	2	6
TOTAL	3	2	3	8

CRASH DESCRIPTION TYPES BY YEAR

	2020 CRASHES	2021 CRASHES	2022 CRASHES	ALL YEARS CRASHES
HIT FIXED OBJECT	0	1	1	2
OPP DIRECTION SIDESWIPE	2	0	0	2
REAR END	1	1	2	4
TOTAL	3	2	3	8

PERSON INJURY SUMMARY BY YEAR

	2020 PERSONS	2021 PERSONS	2022 PERSONS	ALL YEARS PERSONS
FATALITIES	0	0	0	0
SUSPECTED SERIOUS INJURIES	0	0	0	0
SUSPECTED MINOR INJURIES	0	0	0	0
POSSIBLE INJURIES	1	0	0	1
UNKNOWN SEVERITY	0	0	0	0
UNKNOWN IF INJURED	0	0	2	2

* **PLEASE NOTE: Years which do not appear in the report contain zero crashes for this request.**
* Complete records of reportable crashes are available in PCIT for the following years: 2003 - 2022
* Crash information for 2023 is incomplete at the time of this printing. As such, data for 2023 is not included in this report.

Print Date: 10/06/2023

Print Date: 10/06/2023

PCIT - PUBLIC REQUEST / PRESS INQUIRY REPORT (01-07)

NOTES:

1 Injury Severity Disclaimer

Please note that beginning January 1, 2016, PennDOT adopted the Federal standard for collecting injury severity data. The field descriptions and definitions changed from the state standard that had been in use for decades. This resulted in a substantial shift in severity levels. Therefore, comparison of the "Suspected Serious Injury", "Suspected Minor Injury" and "Possible Injury" categories will not be consistent for crashes taking place before versus after the adoption of the new standard.

REPORT PARAMETERS:

Query ID:	0320231006134
User ID:	b-sswai
Title:	Millers Run Road (SR 0050)
Date Range :	01/01/2018 to 12/31/2022
Selected Shapes :	NO NAME RD x MILLERS RUN RD,NO NAME RD x MILLERS RUN RD - Buffer (250 feet)
Filter Characteristics:	

This report counts the number of crashes.

Millers Run Road (SR 0050) with Newbury Drive

Sorted by Crash Date
Date Range: 01/01/2018 to 12/31/2022

USER ID / QUERY ID:
b-sswai / *0320231006135*

	CRN	CO	DATE	DAY	TIME	LIGHTING	ROAD SURF	WEATHER	FAT	INJ	PED	VEH	MAX SEVERITY
1	2020004800	02	01/13/2020	MON	08:08	DAYLIGHT	DRY	CLEAR	0	0	0	2	PROP DMG ONLY
	ENV RDWY FACTORS: NONE												OPP DIR SIDESW
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 BUS TRAVELING EAST IN RIGHT OF TRAFFICWAY TURNING RIGHT												
	VEH EVENTS: **HIT UNIT 02**												
	DVR ACTIONS: OTHER IMPROPER DRIV ACTIONS												
	VEH: 2 VAN TRAVELING EAST IN RIGHT LANE GOING STRAIGHT												
	VEH EVENTS: **STRUCK BY UNIT 01**												
	DVR ACTIONS: NO CONTRIBUTING ACTION												
2	2020097116	02	11/13/2020	FRI	18:02	STREET LT	DRY	CLEAR	0	0	0	2	PROP DMG ONLY
	ENV RDWY FACTORS: NONE												REAR-END
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 SMALL TRUCK TRAVELING WEST IN LEFT LANE GOING STRAIGHT												
	VEH EVENTS: **HIT UNIT 02**												
	DVR ACTIONS: TAILGATING DRIVER WAS DISTRACTED												
	VEH: 2 SMALL TRUCK TRAVELING WEST IN LEFT LANE GOING STRAIGHT												
	VEH EVENTS: **STRUCK BY UNIT 01**												
	DVR ACTIONS: NO CONTRIBUTING ACTION												
3	2020098335	02	11/17/2020	TUE	07:40	DAYLIGHT	WET	RAIN	0	1	0	2	POSSIBLE INJURY
	ENV RDWY FACTORS: OTHER WEATHER CONDITIONS												OPP DIR SIDESW
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 SUV TRAVELING WEST IN ONCOMING TRAFFIC LANE TURNING LEFT												
	VEH EVENTS: **HIT UNIT 02**												
	DVR ACTIONS: NO CONTRIBUTING ACTION												
	VEH: 2 SUV TRAVELING EAST IN LEFT LANE GOING STRAIGHT												
	VEH EVENTS: **STRUCK BY UNIT 01**												
	DVR ACTIONS: RUNNING RED LIGHT												
4	2021041821	02	05/09/2021	SUN	05:01	STREET LT	DRY	CLEAR	0	0	0	1	PROP DMG ONLY
	ENV RDWY FACTORS: NONE												HIT FIXED OBJ
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 AUTOMOBILE TRAVELING EAST IN RIGHT LANE TURNING RIGHT												
	VEH EVENTS: **HIT CURB**												
	DVR ACTIONS: IMPROPER/CARELESS TURN												
5	2021059685	02	06/28/2021	MON	13:06	DAYLIGHT	DRY	CLEAR	0	0	0	2	PROP DMG ONLY
	ENV RDWY FACTORS: NONE												REAR-END
	T-INT 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 AUTOMOBILE TRAVELING WEST IN RIGHT TURN LANE TURNING RIGHT												
	VEH EVENTS: **HIT UNIT 02**												
	DVR ACTIONS: DRIVER WAS DISTRACTED												
	VEH: 2 AUTOMOBILE TRAVELING WEST IN RIGHT TURN LANE TURNING RIGHT												
	VEH EVENTS: **STRUCK BY UNIT 01**												
	DVR ACTIONS: NO CONTRIBUTING ACTION												

Millers Run Road (SR 0050) with Newbury Drive

Sorted by Crash Date
Date Range: 01/01/2018 to 12/31/2022

USER ID / QUERY ID:
b-sswai / *0320231006135*

	CRN	CO	DATE	DAY	TIME	LIGHTING	ROAD SURF	WEATHER	FAT	INJ	PED	VEH	MAX SEVERITY
6	2022015432	02	02/07/2022	MON	16:15	DAYLIGHT	DRY	CLEAR	0	0	0	1	PROP DMG ONLY
	ENV RDWY FACTORS: NONE												HIT FIXED OBJ
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 SMALL TRUCK TRAVELING WEST IN RIGHT LANE GOING STRAIGHT												
	VEH EVENTS: HIT TRAFFIC ISLAND / CHANNELIZATOTHER POST, POLE, OR SUPPORT												
	DVR ACTIONS: DRIVER WAS DISTRACTED												
7	2022032948	02	04/04/2022	MON	19:20	DAYLIGHT	DRY	CLEAR	0	0	0	2	UNK IF INJURED
	ENV RDWY FACTORS: NONE												REAR-END
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 SUV TRAVELING EAST IN RIGHT LANE GOING STRAIGHT												
	VEH EVENTS: HIT UNIT 02												
	DVR ACTIONS: DRIVER WAS DISTRACTED FAILURE TO RESPOND TO TCD												
	VEH: 2 VAN TRAVELING EAST IN RIGHT LANE SLOWING OR STOPPING IN LANE												
	VEH EVENTS: **STRUCK BY UNIT 01**												
	DVR ACTIONS: SUDDEN SLOWING / STOPPING												
8	2022079619	02	08/30/2022	TUE	17:40	DAYLIGHT	DRY	CLEAR	0	0	0	4	PROP DMG ONLY
	ENV RDWY FACTORS: NONE												REAR-END
	4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR												
	VEH: 1 AUTOMOBILE TRAVELING EAST IN OTHER FWD MOVING LANE GOING STRAIGHT												
	VEH EVENTS: HIT UNIT 02												
	DVR ACTIONS: DRIVER WAS DISTRACTED												
	VEH: 2 AUTOMOBILE TRAVELING EAST IN OTHER FWD MOVING LANE STOPPED IN TRAFFIC LANE												
	VEH EVENTS: **HIT UNIT 03**												
	DVR ACTIONS: NO CONTRIBUTING ACTION												
	VEH: 3 AUTOMOBILE TRAVELING EAST IN OTHER FWD MOVING LANE STOPPED IN TRAFFIC LANE												
	VEH EVENTS: **HIT UNIT 04**												
	DVR ACTIONS: NO CONTRIBUTING ACTION												
	VEH: 4 SUV TRAVELING EAST IN OTHER FWD MOVING LANE STOPPED IN TRAFFIC LANE												
	VEH EVENTS: STRUCK BY UNIT 03												
	DVR ACTIONS: NO CONTRIBUTING ACTION												

Millers Run Road (SR 0050) with Newbury Drive

Sorted by Crash Date

NOTES:

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